Feasibility Report



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Genesee River Basin Study

Volume 2
Supporting
Documentation





US Army Corps of Engineers

Buffalo District

June 1988

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This Feasibility Report is Final.

19. KEY WORDS (Continue on reverse side if necessary and identify by block number) Genesee River Basin Comprehensive Study Mt. Morris Dam and Reservoir Letchworth State Park at Mt. Morris, New York Considered Stannard Dam/Reservoir and Modification to existing Mt. Morris

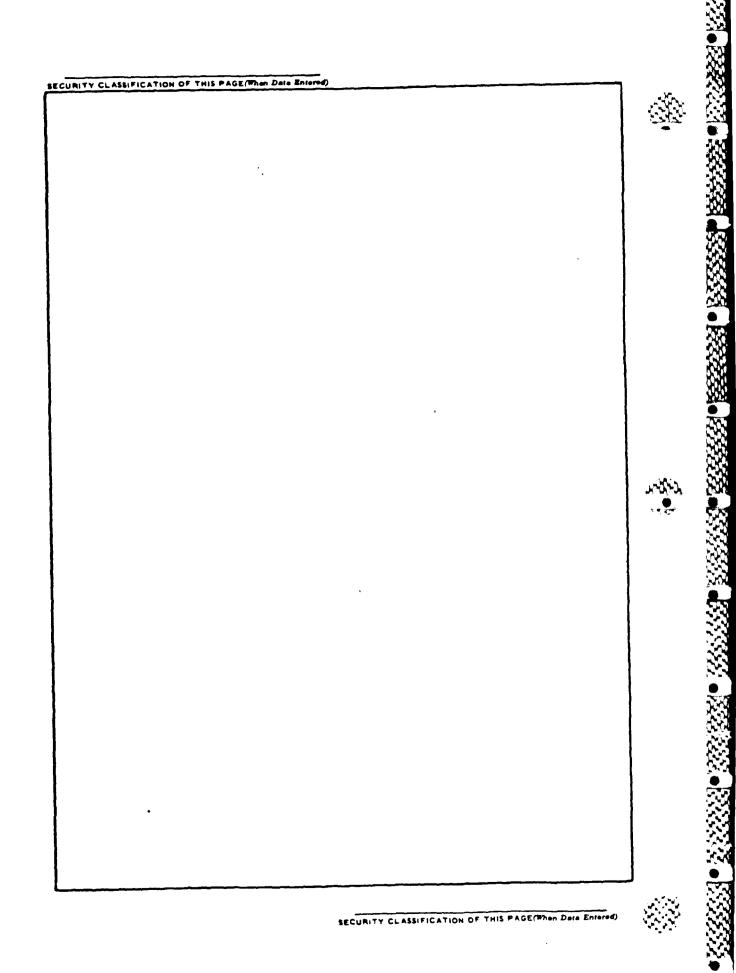
20. ABSTRACT (Continue on reverse side if necessary and identify by block number)

This Feasibility Report discusses plans considered under the Genesee River Basin Study Authority. Two of these plans: a multi-use reservoir at Standards and modification to the existing Mt. Morris Dam and reservoir for multi-use including flood control, recreation, hydropower, and irrigation. These plans were found not economically feasible for single-purpose flood control. The Federal Government therefore recommended no further Federal actions.

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FEASIBILITY REPORT (June 1988)

APPENDIX A

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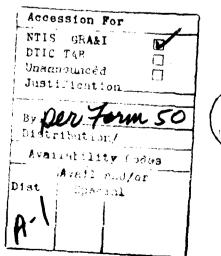




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GENESEE RIVER BASIN STUDY

APPENDIX A

Al BASIN DESCRIPTION

Al.l General.

The Genesee River rises in the Allegheny Mountains in Potter County. Pennsylvania, and flows north for about 157 miles to Rochester, New York, where it empties into Lake Ontario. The watershed is roughly elliptical in shape, with a north south major axis of approximately 100 miles, and a maximum width of 40 miles. The total basin area is 2,480 square miles, and can be found on Plate Al. The largest tributary of the Genesee River is Canaseraga Creek, with a drainage area of 337 square miles. The confluence of Canaseraga Creek with the Genesee River is about 4 miles downstream of the Corps of Engineers dam at Mount Morris. The topography of the southern portion of the basin, upstream of the Mount Morris Dam, is steep and rugged, while the northern portion downstream of Mount Morris is gently rolling plains. The Genesee River drops from about elevation 1,080 feet NGVD to 768 feet NGVD over the three waterfalls in Letchworth Park (just upstream of Mount Morris), flowing through deep gorges cut in the Portage geological formations in Letchworth Park. From Mount Morris downstream, Genesee River flows through alluvial plains in wide flat valleys that can be up to 3 miles in width. At Rochester, the river drops over three falls from elevation 481 feet NGVD to elevation 249 feet NGVD, and then empties into Lake Ontario. A profile of the Genesee River and its major tributaries is shown on Plate A2.

When the slope characteristics of the Genesee River are studied, the slopes contrast from a flashy, steep gradient stream to a sluggish, meandering river. The river from its source in Pennsylvania to the New York State boundary has a slope of approximately 102 feet/mile. For the next 25 miles, the slope is approximately 12 feet/mile, and in the 38 miles before the three waterfalls in Letchworth State Park, the slope is approximately 6 feet/mile. Through the 17 miles of Letchworth State Park, the river drops 317 feet. From Mount Morris to Rochester, the river drops at 0.8 feet/mile. The last 6 miles to Lake Ontario there is no slope.

The largest tributary of the Genesee River is Canaseraga Creek. Canaseraga Creek Watershed drains 337 square miles. Its confluence with the Genesee River is near Jones Bridge, just downstream of Mount Morris Dam. Canaseraga Creek resembles the Genesee River Basin, in that the reaches upstream of Dansville are steep and rugged, while downstream of Dansville, Canaseraga flows through a flat alluvial plain to the Genesee River. Above Dansville, the main stem has a slope of about 40 feet/mile, and below Dansville, Canaseraga Creek has a slope of about 3 feet/mile. The Canaseraga Creek basin is roughly square in shape, about 20 miles to a side. The main stem, which rises at about elevation 1,900 NGVD, has a length of 42 miles. Canaseraga Creek joins the Geneseee River at elevation 548 feet NGVD.

Other tributaries of the Genesee River have a wide range in size and topographic characteristics. For example, Angelica Creek, in the watershed above Mount Morris, has a drainage area of 90 square miles, and is topographically rugged, with a main stream slope of 38 feet/mile. Black Creek, in the lower basin, has a drainage area of 201 square miles. Its basin is relatively flat and marshy, with a main channel slope of 6.5 feet/mile. Drainage area for selected sites on the Genesee River and the major tributaries can be found on Table Al.

Al.2 Climatology.

The climate of the Genesee River Watershed is generally that of the humid or forest climate which prevails over most of the United States east of the Mississippi River. Average annual temperatures range from 45°F to 48°F throughout the basin. Temperatures have reached slightly over 100°F in most parts of the watershed, and it has been as low as 20 to 40 degrees below zero. The average annual precipitation on the basin ranges from 25 to 40 inches with sharp difference between the western rim and the central parts of the valley. Areal distribution of the annual average precipitation is presented on Plate A3.

Al.3 Existing Flood Control Structures.

The existing Corps of Engineers built flood control projects in the basin are:

- a. Mount Morris Dam. This project consists of a dam at Mount Morris on the Genesee River, some 32 miles southwest of Rochester, New York. The concrete gravity dam, rising 210 feet above the river bed, controls a drainage area of 1,080 square miles, and has a flood control pool of 337,400 acre-feet, with very little conservation pool storage. The project was completed in 1953, at a Federal cost of \$23,365,559. Thus far, Mount Morris has prevented flood damages estimated at \$346,670,000 (total to 30 September 1985). For the flood of June 1972, Mount Morris prevented an estimated \$210,000,000.
- b. Local flood control project on Oatka Creek at Warsaw, New York. This project consists of enlargement of the creek channel to increase the capacity of Oatka Creek, a drop structure at the upstream limit of improvement, abutment protection at two bridges, and construction of several short sections of retaining walls. The project was completed in 1968 at a cost of \$698,300. Flood damages prevented to date are estimated at \$415,400.
- c. Wellsville Flood Control Project, Genesee River and Dyke Creek. This project provided for enlargement of the channels of both streams, underpinning of two bridges, riprap bank protection, drop structures and appurtenant works. The project was completed in 1959 at a cost of \$1,230,000. After the June 1972 flood, the project was repaired at a cost of \$374,000. Two separate rectification measures have been undertaken to correct deficiencies in the project at a cost of \$2,418,000 in 1973-1974 and in 1976. Flood damages prevented to date (30 September 1985) have been estimated at \$13,471,000. The project was estimated to have prevented \$8,000,000 in flood damages during the June 1972 event.

Table Al - Drainage Areas, Genesee River Watershed

Stream and Location Farea (sq.:Above mil.) Mouth*			:Drainage	:Miles
1. Genesee River below Genesee, PA			:Area (so	.:Above
1. Genesee River below Genesee, PA 2. Genesee River at Shongo, NY 3. Genesee River at Stannards, NY 4. Genesee River at Stannards, NY 4. Genesee River above Dyke Creek at Wellsville, NY 5. Dyke Creek at mouth at Wellsville, NY 6. Genesee River above Dyke Creek at Wellsville, NY 7. Vandermark Creek at mouth of Scio, NY 8. Genesee River at Scio, NY 7. Vandermark Creek at mouth of Scio, NY 8. Genesee River at Dam at Belmont, NY 9. Phillips Creek at mouth at Belmont, NY 9. Phillips Creek at mouth at Belmont, NY 9. Phillips Creek at mouth of Black Creek at Belfast, NY 10. Genesee River above Angelica Creek near Angelica, NY 11. Angelica Creek at mouth of Black Creek at Belfast, NY 12. Genesee River above mouth of Black Creek at Belfast, NY 13. Black Creek at mouth at Caneadea, NY 14. Genesee River above Caneadea Creek at Caneadea, NY 15. Caneadea Creek at mouth at Caneadea, NY 16. Cold Creek at mouth at Fillmore, NY 17. Rush Creek at mouth of Fillmore, NY 18. Genesee River above mouth of Rush Cr. at Fillmore, NY 19. Genesee River above mouth of Wiscoy Cr. at Rossburg, NY 10. Wiscoy Creek at mouth at Rossburg, NY 11. Genesee River above mouth of Wiscoy Cr. at Rossburg, NY 12. Genesee River above mouth of Rush Cr. at Fillmore, NY 18. Genesee River above mouth of Canaseraga Creek 11. Oald 12. Genesee River at Mouth Morris Dam 12. Genesee River above mouth of Canaseraga Creek 11. Oald 12. Genesee River above mouth of Canaseraga Creek 11. Oald 12. Genesee River above mouth of Canaseraga Creek 11. Oald 12. Genesee River above mouth of Conesus Creek at Avon, NY 12. Genesee River above mouth of Conesus Creek at Avon, NY 13. Genesee River above mouth of Conesus Creek at Avon, NY 14. Genesee River above mouth of Hemlock Lake Outlet 13. Genesee River above mouth of Hemlock Lake Outlet 13. Genesee River above mouth of Hemlock Lake Outlet 13. Genesee River above mouth of Black Creek 14. Oacka creek at mouth at Scottsville, NY 15. Hemlock Lake at Hemlock, NY 16. Honeoye Creek at Roush, NY 17. Hemlock Lake at Hemlock, NY 18. Honeo		Stream and Location	: mi.)	:Mouth*
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9. Phillips Creek at mouth at Belmont, NY : 30.5 : - 10. Genesee River above Angelica Creek near Angelica, NY : 489 : 122.3 11. Angelica Creek at mouth : 90.1 : - 12. Genesee River above mouth of Black Creek at Belfast, NY : 600 : 118.4 13. Black Creek at mouth at Belfast, NY : 31.0 : - 14. Genesee River above Caneadea Creek at Caneadea, NY : 667 : 112.7 15. Caneadea Creek at mouth at Caneadea, NY : 62.8 : - 16. Cold Creek at mouth of Fillmore, NY : 40.9 : - 17. Rush Creek at mouth of Fillmore, NY : 41.2 : - 18. Genesee River above mouth of Rush Cr. at Fillmore, NY : 486 : 102.6 19. Genesee River above mouth of Wiscoy Cr. at Rossburg, NY : 854 : 99.3 20. Wiscoy Creek at mouth at Rossburg, NY : 854 : 99.3 21. Genesee River at Mount Morris Dam : 1,084 : 89.8 22. Silver Lake Outlet at mouth near Mount Morris, NY : 31.4 : -	7.	Vandermark Creek at mouth of Scio, NY	: 22.7	: -
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^{*} Genesee River reaches only.

- d. Snagging and clearing projects were performed at the following areas in the Genesee River Basin:
 - a. Canaseraga Creek, lower reaches (1954)
 - b. Genesee River and Dyke Creek at Wellsville (1951) and
 - c. Keshequa Creek at Nunda (1955)

Al.4 Other Flood Control Projects.

Other flood control projects in the Genesee Basin include flood walls along the Genesee River in the city of Rochester, and levee and pumps built by local interests along Canaseraga Creek and its tributaries (lower reaches).

Al.5 Other Control Works.

In addition to the flood control projects mentioned in the previous two sections, there are other structures on the Genesee River that effect discharges and water levels. These structures are:

- a. Two run-of the-River hydropower plants, developed by the Rochester Gas and Electric Company, on the Genesee River in Rochester.
- b. The Court Street Dam, operated by the New York State Department of Transportation, which is used to maintain a constant water surface elevation in the New York State Barge Canal, which crosses the Genesee upstream of the dam, during the navigation season. This dam is located in the City of Rochester;
- c. Rushford Lake on Caneadea Creek, operated by the Rochester Gas and Electric Corporation;
- d. A dam on Hemlock Lake, in the Honeoye Creek Basin, operated by the City of Rochester, to provide water supply to that city;
- e. A dam on Conesus Lake outlet to maintain adequate lake levels for recreation on the Lake;
- f. A dam on the Genesee River, just below Mount Morris, operated by Rochester Gas and Electric company for power;
- g. A concrete arch type dam on the Wiscoy Creek, 3 miles upstream of the Genesee River; and
 - h. A 14 feet high dam at Belmont, New York on the Genesee River.

Al.6 Summary of Studies.

With this current Genesee River Basin study in the reconnaissance phase, as much data and information as possible was used from earlier studies. The information that was updated for this study included:

- a. Hydropower benefits;
- discharge-frequency curves;
- c. stage-damage curves, and
- d. flood reduction benefits.

The existing information that was used was gathered from file reports and 4 published reports. The published Reports are:

- a. "Genesee River Basin Study," June 1969, Buffalo District
- b. "Stannard Reservoir, New York, Letter Report; Post Flood Report on Effects of Agnes," 1 April 1974, Buffalo District;
- c. "Phase I Report, Canaseraga Creek, New York, Local Protection Project," Sept 1974, prepared by Erdman and Anthony Associates, Consulting Engineers, for the Buffalo District; and
- d. "Reservoir Regulation Manual, Mount Morris Dam and Reservoir," Buffalo District, September 1978.

A2 HISTORICAL FLOODS

Damaging floods on the Genesee Basin have occurred in all months of the year except August. Summer floods are, in general, localized in a part of the watershed and are usually the results of convectively usable air conditions. Winter and Spring floods are usually the result of frontal precipitation on saturated or frozen ground, or on melting snow cover, although floods have occurred from melting snow cover alone. Some of the larger floods are:

- a. Flood of March 1865. The largest known peak discharge at Rochester, estimated at 54,000 cfs, was the result of a heavy snowfall, followed by a sudden thaw accompanied by warm rains. The capacity of the channel in Rochester at that time was less than 40,000 cfs; hence, at the flood crest an overflow in excess of 14,000 cfs flowed into the city, inundating most of the central portion and causing extensive damage. The flats from Rochester to Mount Morris were flooded, and the embankment of the New York Central Railroad near Avon was destroyed.
- b. Flood of March 1875. This flood was caused by the spring break-up and warm rain. During the flood, an ice jam formed at the Clarissa Street bridge in Rochester and backwater inundated large areas of the city, causing extensive damage.
- c. Flood of June 1889. As a result of general rains, all streams in western New York were in flood. Bridges were washed out at Wellsville, Belmont, Mount Morris, and Dansville, and agricultural interests in the Genesee and Canaseraga valley flats were severely damaged. Rochester escaped damage.

- d. Flood of May 1894. Heavy precipitation terminated a long wet spell. The discharge at Mount Norris increased from 5,000 to 42,000 cfs in less than 36 hours. The Canaseraga and Genesee flats were inundated to depths of 4 to 6 feet, and the area covered was stated by local newspapers to be 60 to 80 square miles. The valley storage reduced the discharge at Rochester to approximately 30,000 cfs, and little damage occurred in the city.
- e. Flood of April 1896. This flood was caused by melting snow from the hills of the watershed flowing into swollen streams. The Genesee and Canaseraga flats were inundated but the flood preceded the growing season and little damage resulted. Rochester was not affected.
- f. Flood of March 1902. This flood was caused by a sudden thaw, during which no appreciable precipitation occurred. The Genesee flats were flooded and bridges in the upper basin were washed out. Part of the business section of Rochester was inundated to a depth of about 2 feet.
- g. Flood of July 1902. This flood was the result of a heavy rainfall on ground saturated by prior light rains. Local interests estimated the flow at Mount Morris to have exceeded 40,000 cfs, and the flats in the Canaseraga and Genesee valleys were inundated, resulting in large damages to crops. At Rochester, the discharge did not exceed 20,000 cfs and no damage occurred.
- h. Flood of March 1913. Streams flowing at near-bankfull capacity, as a result of a thaw, were augmented by 5 days of heavy rainfall. During the period 23-27 March, inclusive, the total rainfall for the upper basins was 4.93 inches and for the lower, 3.94 inches. The resulting flood peaked at 37,800 cfs feet at St. Helena, 38,000 cfs at Jones Bridge, and 42,000 cfs at Rochester. The Canaseraga flats were in flood nearly to Dansville and the Genesee flats from Mount Morris to Rochester. Parts of the business section of Rochester were inundated and damages were large.
- i. Flood of March 1916. This flood was caused by rapid melting of a heavy snow cover. The peak discharge of 48,300 cfs at Rochester was the greatest since 1865, but, because of channel improvements through the city, completed after the 1913 flood, little damage occurred.
- j. Flood of May 1916. This flood the second of the year, was caused by excessive precipitation. Discharges of 44,400 and 55,100 cfs were recorded at St. Helena and Jones Bridge, respectively, and are the greatest of record for these stations. Early crops in the flats were affected but the loss was small, and the city of Rochester sustained no damages.
- k. Flood of December 1927. As the result of a long wet spell terminated by 2 days of heavy rainfall, crests of 46,800 cfs at Jones Bridge and 29,600 cfs at Rochester occurred on 1-2 December, respectively. The Genesee and Canaseraga flats were inundated but little damage occurred. Rochester was not affected.

- 1. Flood of July 1935. This flood, caused by an extensive three-day rainstorm concentrated over south central New York, affected only the southeastern portion of the Genesee Basin. The precipitation stations in this portion of the basin, Alfred, Andover, Angelica, and Dansville, recorded totals for the 3-day rain ranging from 5.37 to 6.35 inches. No excessive rains were recorded by stations in other sections of the Genesee Basin. The peak discharges in the Genesee River were only 24,500 cfs at Jones Bridge and 18,600 cfs at Rochester, whereas the station near Dansville on Canaseraga Creek recorded a peak flow of 8,390 cfs. The principal damage areas were the argicultural lands in the Canaseraga valley, and the village of Wellsville on Dyke Creek. Damage in the Genesee flats was small and Rochester was not affected.
- m. Flood of July 1942. Floods, confined principally to western Pennsylvania, were caused by very intense rainfall over a relatively short duration. Records for point rainfall for durations up to 24 hours were broken during this storm. On the Genesee Basin, damage was confined to the upper reaches in the vicinity of Wellsville. The rainfall at Alfred, Andover, and Angelica, for 17-18 July was 3.35, 4.10, and 4.05 inches, respectively. The records from automatic rainfall recorders indicate that most of the precipitation occurred during the evening of the 17th and the early morning of the 18th. Peak discharges of 11,200 cfs, 18,900 cfs, and 15,700 cfs were recorded at Scio, St. Helena, and Jones bridge, respectively.
- n. Floods of March-April 1950. This period covers two peaks a week apart. The first was caused by snowmelt accompanied by light precipitation and produced a crest of 45,400 cfs at Jones Bridge on the 29th of March. The second crest, on 5 April, was the result of moderate rainfall on wet soil and produced a crest at Jones Bridge of 25,200 cfs.
- o. Flood of November 1950. The heavy rain of 25 November caused high water in the upper basin, and Wellsville experienced severe flooding. The south side of the village was inundated and many families were taken from their homes in boats. Several sections of highway near Wellsville and Portageville were under water. In the lower basin, flooding was slight, although some flatlands were flooded and sections of highway near Geneseo were covered by water. Although the construction of Mount Morris Dam was not complete at this time, the dam was operated for flood control.
- p. Flood of March 1956. This flood was of the type common in the Genesee River Basin, a combination of warm rain and snowmelt. This flood occurred after completion of Mount Morris Dam, and gives an example of the operation procedures used during a flood. Releases were reduced to about 300 cfs when the storm began, and then were increased to develop a flow of 12,000 cfs at the Jones Bridge gage, after the danger of downstream flooding had passed. Low-lying farmlands below Avon were flooded from local runoff, and there was some backwater flooding during the reservoir evacuation period. Part of this flooding was due to the fact that because of the protection provided by the dam, there has been some encroachment into the old flood plain. Also some banks had been breached by local farmers in order to drain their lands. The backwater flooding prompted reconnaissance of the lower basin, which established 10,500 cfs as a within-channel capacity in the

vicinity of Avon and set the Avon gage as the primary control point for future evacuation periods. The storm runoff resulted in a peak flow to the reservoir of 46,000 cfs and the operation of the dam controlled the flow at Jones Bridge to not more than 12,000 cfs. The flood discharge at Rochester was held to 24,300 cfs, in contrast to an estimated natural flow of 48,300 cfs. The maximum storage in the reservoir was 183,540 acre/feet, with a corresponding pool elevation of 706.9 feet.

- q. Flood of March-April 1960. This flood was caused primarily by melting of heavy snow cover. Prior to the flood, the average water content of the snow cover was 3.3 inches in the upper basin and 4.1 inches in the lower basin. Therefore, with a sudden rise in temperature, the lower basin had the potential of a serious flood, even with no discharge from the reservoir. Releases from the reservoir were reduced to 300 cfs when a general thaw was forecast, and after the danger of downstream flooding was passed, flows were increased to 10,500 cfs at Avon. Low-lying farmlands again were flooded by local runoff, but no backwater flooding occurred during the evacuation period. The peak inflow to the reservoir was 35,000 cfs and the controlled peaks at Jones Bridge and Avon were 10,000 and 10,500 cfs, respectively.
- r. Flood of April 1961. This flood was of moderate proportions throughout the Genesee Basin, with no significant flooding except in the Canaseraga Creek watershed. It was the fourth highest flood of 50 years of record at Dansville, with a peak of 8,230 cfs. and the highest of 12 years of broken record at Shakers Crossing, near the mouth of the Canaseraga. Flooding of the rich farmland in the Canaseraga valley caused extensive damage.
- s. Flood of June-July 1972. (Hurricane Agnes) Tropical Storm Agnes and associated weather systems produced the most destructive widespread flooding of record over eastern United States. In the Genesee Basin, the predominant portion of the rainfall occurred from 9 p.m. 20 June, to 6 a.m. on 23 June. The maximum total storm rainfall, 13.72 inches, and maximum daily rainfall. 6.57 inches, were recorded at the Wellsville gage. A "bucket survey" of the Genesee Basin made by the Corps personnel indicated a maximum of about 16 inches of rainfall in the upper reaches of Dyke Creek near Andover, New York. The average total basin rainfall for the period 20-25 June was 7.1 inches while the average for the same period on the upper basin (above Mount Morris dam) was 10.20 inches. Regulation during a protion of this flood required the controlled release of dam outflows in excess of downstream channel capacity to prevent overtopping the spillway with debris laden flows. The reservoir pool reached a maximum elevation of 755.8 feet, thus occupying approximately 96 percent of total reservoir storage. This was the highest pool elevation ever attained in the Mount Morris Reservoir. Detailed information on this flood is contained in Buffalo District's "Report of Flood, Tropical Storm Agnes, 21-23 June 1972, Genesee River Basin," dated August 1973.

t. Flood of February-March 1976. During the period 16-23 February, approximately 2.6 inches of rain fell over the upper basin. This rainfall augmented by about 2 inches of snowmelt runoff resulted in a peak reservoir elevation on 23 February of 727.6, or about 71 percent of available storage. During the period 1-6 March, approximately 2.5 inches of rain, including some snowmelt, caused the pool to again rise. On 6 March, the reservoir pool peaked at 744.1 feet, thus utilizing 85 percent of the total storage. Peak inflows to Mount Morris Reservoir during the February and March runoff events reached 32,500 cfs and 28,000 cfs, respectively. Although the peak inflows were not particularly impressive, the volume of water received caused the 6 March pool elevation to be the second highest of record, exceeded only by the flood from Hurricane Agnes.

Since the March 1976 floods, most of the flooding in the Genesee watershed has been limited to the Black and Oatka Creek watersheds (March 1978, March 1979, March 1984, December 1984 and January 1986). Heavy rains have caused flooding on the tributaries to the Genesee River around and south of Wellsville, New York, at various times since 1976.

A3 FLOOD PRONE AREAS

Flooding is experienced throughout the Genesee River watershed. Flooding occurs on Black Creek, Oatka Creek, Honeoye Creek, Conesus Lake, Honeoye Lake, Canaseraga Creek, Genesee River and Dyke Creek. These are the major areas that experience flooding, and there are isolated incidents of flooding in areas other than listed above. The Genesee River was broken up into 14 damage reaches, which are described in Table A2. Table A3 gives the approximate channel capacities for each reach.

A4 GAGING STATIONS

A4.1 Stream and Lake Gages

There are numerous gage sites located throughout the Genesee River Basin. This report has been able to locate 51 active or discontinued gage sites in the Genesee Basin. Table A4 lists the active gages in the Genesee River Basin and Table A5 lists the discontinued gages in the basin. Table A6 has the maximum stage or discharge of record for the active gages. The location of the active recording gages can be found on Plate A4.

A4.2 Precipitation Gages

The aerial distribution of precipitation over the Genesee River Basin is represented by the total precipitation statons at Avon, Warsaw, Hemlock, Portageville, Dansville, Wiscoy, Rushford, Angelica, Wellsville 4 NNW, and Whitesville, and by the recording gages at Rochester Airport, Pavilion, East Bloomfield, Mount Morris Dam, Wellsville, and Raymond. The temporal distribution of rainall is represented by the recording gages. All of the precipitation gages are in New York State except for the gage at Raymond, Pennsylvania. The location of these gages can be found on Plate A5.

Table A2 - Damage Reaches of the Genesee River Basin

	: :		:Initial Damage		7
	: River :		:Stage in Feet		
Reach	: Mile :	Location	: (NGVD Datum)	: Limit of Reach	
	:		:	:	
Lower Basin	: :		:	:	
- .	:		•		
Rochester		e gate at Driving	: 260.8	:From the mouth of	
ı		rk Avenue in	:	:Lake Ontario to	
	: : : : : : : : : : : : : : : : : : : :	chester, mile 6.1		:the NYS Barge Canal	
Chili -	: 11.6 to:The	e confluence of	: 516.0	:Between the NYS	
Kenrietta		ack Creek and the	: 510.0	:Barge Canal and	
2		nesee R., mile 14.1	•	:Brown's Bridge	
•	: :		:	:	
Avon	: 21.1 to:The	e confluence of	: 529.0	:From Brown's Bridge	
3		neoye Ck. and the	:	:to the gage site at	
		nesee R., mile 26.6	:	:Avon Bridge	
	: :	•	:	:	
Geneseo	: 35.3 to:The	e gage at Jones	: 560.0	:Between the Avon	
4	: 65.5 :Br:	idge, mile 61.2	:	:Bridge and the	
	: :		:	:Rochester Gas and	
	: :		:	:Electric Dam	
	: :		:	:	
Upper Basin	: :		:	:	
	:		:	:	
Mt. Morris	: 65.5 to:	(1)	: (1)	:From Rochester Gas	1
5	: 86.0 :		•	:and Electric Dam to	
	:		:	:the Erie Railroad	
	:		:	:Bridge	
Dont croud 11a	: 06 0 to 1	the secondite is	: 1102.0	:Between the Erie	
6		the gage site in rtageville, mile	. 1102.0	:Railroad Bridge and	
U	: :86		•	the mouth of Wiscoy	
	•	,	•	:Creek.	
	•		• :	:	
Fillmore	: 96.0 to:400	O feet downstream	: 1165.0	:From the mouth of	
7		the bridge in	:	:Wiscoy Creek to	
		llmore, mile 99.9	:	:2,200 feet downstream	
	:	•	:	of the bridge at	
	: :		:	:Oramel	
	:		:	:	
Belfast		Belfast, mile	: 1261.0	:From 2,200 feet down-	
8	:120.0 :113	3.5	:	:stream of the bridge	
	:		:	at Oramel to 6,300	
			:	upstream of Transit	
	•				
	:		:	:Bridge, south of	
	: : :		: :	:Bridge, south of :Belfast	

Table A2 - Damage Reaches of the Genesee River Basin (Cont'd)

	: :		:Initial Damage	:
	: River :	Index Point	:Stage in Feet	:
Reach	: Mile :	Location	: (NGVD Datum)	: Limit of Reach
	:		:	:
Belvidere	:120.0 to:A	t the upstream side	: 1320.0	:From 6,300 feet up-
9	:125.1 :0	f New York State	:	stream of Transit:
	: :R	oute 408, bridge	:	:bridge to a section
		ver the Genesee	:	:6,800 feet downstream
	: :R	iver, mile 123.0	:	of NYS Route 244 in
	: :		:	:Belmont
	: :		:	:
Belmont	:123.1 to:4	00 feet upstream or	: 1366.0	:From 6,800 feet down-
10	:131.0 :N	ew York State Route	:	stream of New York
	: :2	44, mile 126.7.	:	:State Route 244 to
	: :		:	:3,300 feet downstream
	: :		:	of the bridge in Scio
	: :		:	:
Scio	:131.0 to:A	t the gage site at	: 1446.5	:From 3,300 feet down-
11	:136.0 :S	cio, mile 132.8	:	stream of the bridge
	: :	·	:	:in Scio to 1,500 feet
	: :		:	:downstream of New York
	: :		:	:State Route 17
	: :		:	:
Wellsville	:136.0 to:	(2)	: (2)	:From 1,500 feet down-
12	:138.8 :		:	stream of New York
	: :		:	:State Route 17 to
	: :		:	:Weidrick Road
	: :		:	:
Stannards	:138.8 to:3	,000 feet upstream	: 1511.7	:From Weidrick Road
Corners		f Weidrick Road,	:	to Hanks Road
13		ile 139.4	:	:
	: :		:	:
Shongo	:140.8 to:1	,600 feet upstream	: 1529.5	:From Hanks Road to
14		f Hanks Road, mile	:	:the New York-
		41.1	:	:Pennsylvania State
	: :		:	:line
	•		•	•

⁽¹⁾ This reach includes the area known as Letchworth State Park and is mainly a deep gorge that contains Mount Morris Lake.

⁽²⁾ This reach lies entirely within the village of Wellsville and has a completed flood control project.

Table A2 - Damage Reaches of the Genesee River Basin (Cont'd)

Reach No.	: Index Point	: Description of Reach
WELLSVILLE AREA		:
Genesee River		:
12, G-1	: 1,400 feet downstream from : Bolivar Road Bridge	: Northerly village limits to : Bolivar Road .
12, G-2	l,500 feet upstream from Bolivar Road Bridge	: From Bolivar Road to a line : joining intersection of Oak : and Main Streets with inter- : section of Seneca and : Chamberlain Streets
12, G-3	: 100 feet downstream from : Pearl Street Bridge	: From above line to Pearl : Street and Stevens Street
12, G-4	: 100 feet downstream from : State Street Bridge	: Pearl Street and Stevens : Street to State Street
12, G-5A	: 1,100 feet upstream from : State Street Bridge	: State Street to West Dyke : Street, left bank only
12, G-5B	: 1,100 feet upstream from : State Street Bridge	: West Dyke Street to southerly : village limit
12, G-6	l,200 feet upstream of existing municipal dam	: Southerly village limit to : southerly property line, : left bank only :

Table A3 - Channel Capacities

	cfs
Reach l	33,000
Reach 2	: 14,000
Reach 3	: : 11,000
Reach 4	: : 12,000
Reach 5	: : -
Reach 6	: : 24,000
Reach 7	: : 14,000
Reach 8	; ; 9,000
Reach 9	: 6,000
Reach 10	: : 11,000
Reach 11	; ; 5,000
Reach 12, G-1	; 4,000
Reach 12, G-2	; ; 9,000
Reach 12, G-3	: : 20,000
Reach 12, G-4	: : 21,000
Reach 12, G-5A	: : 14,000
Reach 12, G-5B	: : 14,000
Reach 12, G-6	: : 14,000
Reach 13	; ; 5,000
Reach 14	3,000
	•

Table A4 - Active Gages

	······································	:			:	:	:		:# of
		:		;	•	:Drainage	: F:	irst	:Years
		:			•	: Area		In-	: of
	Gage	: Gag	ze i	<i>#</i> :	Туре	:(sq. mi.)		_	
		:				:	:		:
1.	Genesee River at	:0422	210	00:	Recording	: 288	: Aug	1955	: 17
	Wellsville, NY	:		:	;	:	:		:
2.	Black Creek at Hyder	:0422	217	69:	Crest-Stage	: 10.6	:	1978	: 7
	Flats Road at Black	:		:	}	:	:		:
	Creek, NY	:		:	•	:	:		:
3.	Wiscoy Creek at Bliss, NY	:0422	2260	00:	Crest-Stage	22.0	:	1962	: 22
4.	Genesee River at	:0422	2300	00:	Recording	: 984	:Aug	1908	: 77
	Portageville, NY	:		:		:	:		:
5.	Mt. Morris Lake near	:0422	2400	00:	Recording	: 1,080	:Jan	1952	: 33
	Mt. Morris, NY	:		:	•	:	:		:
6.	Sugar Creek near Ossian, NY	:0422	2470	00:	Crest-Stage	: 10.1	:	1964	: 22
7.	Canaseraga Creek above	:0422	247	75:	Recording	: 88.9	: Aug	1974	: 11
	Dansville, NY	:		;	•	:	:		:
8.	Stony Brook Trib. at	:0422	2480	07:	Crest-Stage	: 3.3	:	1977	: 8
	South Dansville, NY	:		:	:	:	:		:
9.	Mill Creek at	:0422	2490	00:	Crest-Stage	: 4.2	:	1964	: 22
	Patchville, NY	:		:	•	:	:		:
10.	Canaseraga Creek at	:0422	2700	00:	Recording	: 335	:Jul	1915	: 25
	Shakers Crossing, NY	:		:	•	:	:		:
11.	Genesee River near	:0422	2750	00:	Recording	: 1,424	:May	1903	: 82
	Mt. Morris, NY (Jones	:		:	:	:	:		:
	Bridge)	:		;	•	:	:		:
12.	Conesus Lake near	:0422	279	80:	Recording	: 69.8	:Jul	1963	: 23
	Lakeville, NY	:		:	:	:	:		:
13.	Genesee River at Avon, NY	:0422	285	00:	Recording	: 1,673		1955	
14.	Honeoye Lake near	:0422	288	45:	Recording	: 41.0	:Jul	1963	22
	Honeoye, NY	:		:	:	:	:		:
15.	Honeoye Creek at	:0422	295	00:	Recording	: 196	:Oct	1945	39
	Honeoye Falls, NY	:		:	:	:	:		:
16.	Oatka Creek at Warsaw, NY				Recording	: 39.1			
17.	Oatka Creek at Garbutt, NY					: 200.0			
18.	Genesee River at	:0423	306	50:	Recording	: 2,210	:Oct	1973	: 10
	Ballantyne Bridge near	:		:	•	:	:		:
	Mortimer, NY	:		;		:	:		:
19.	Black Creek at	:0423	310	00:	Recording	: 130	:Oct	1945	40
	Churchville, NY	:		;	;	:	:		:
20.	Hotel Creek at Griffin		310	40:	Crest-Stage	: 4.6	:	1976	: 10
	Road near Churchville, NY			;	:	:	:		:
21.	Genesee River at Driving	:0423	320	00:	Recording	: 2,467	:Apr	1904	: 82
	Park Ave. at	:		:	:	:	:		:
	Rochester, NY	:		;	:	:	:		:
		:		:		:	:		:

^{*} Through 1984

<u></u>

Table A5 - Discontinued Gages

		:			:	# of
		:	: :	Drainage	: First :	Years
		:	;			of
	Gage	: Gage # :	Type :	(sq. mi.)	stalled:	Record
ı.	Quig Hollow Brook near	•04220450	: :Crest-Stage:	4.2	: : 1965:	7
• •	Andover, NY	:	crest stage	4.2	: 1905.	,
2.	Dyke Creek near Andover, NY	:04220470	Recording	38.0	Feb 1964:	4
3.	Dyke Cr. at Wellsville, NY				: 1956:	
4.	Genesee River at Scio, NY		Recording		:Jun 1916:	56
5.	Van Campen Creek at	:04221600:	Recording :		: 1964:	5
	Friendship, NY	:	:	;	: :	
6.	Angelica Cr. at Transit	:04221720:	Recording :	86.7	:Feb 1964:	5
_	Bridge, NY	:	:	:	: :	
7.	Genesee River at Transit	:04221725:	Crest-Stage:	5 79. 0	: 1975:	2
	Road Bridge near	:	:	:	:	
0	Angelica, NY				;	
8.	Genesee R. at Belfast, NY		Recording :		:Feb 1964:	
9.	Canadaa NY	:04222000	Recording	62.0	:Jul 1949:	19
10.	Caneadea, NY	•0422200	; Dogowdii = = = = = = = = = = = = = = = = = =	, /. <u>4</u> =	: . Iom 106/:	c
10.	East Koy Creek at East Koy, NY	. 04222900	Recording	46.5	:Jan 1964:	5
11.	Canaseraga Creek at	.04224650	Recording	58.4	: :Jan 1964:	6
11.	Canaseraga, NY		. Recording	, 50.4	. Jan 1904.	U
12.	Sugar Creek near	• 04224740	Crest-Stage:	16.9	: 1975:	3
	Canaseraga, NY	:	:		: :	
13.	Stony Brook at Stony	:04224848:	Crest-Stage:	21.4	: 1975:	2
	Brook State Park, NY	:			:	
14.	Mill Creek at Dansville, NY	:04224978:	Crest-Stage:	35.9	1977:	1
15.	Canaseraga Creek near		Recording:		Oct 1917:	61
	Dansville, NY	:	:	:	: :	
16.	Canaseraga Creek at	:04225500:	Crest-Stage:	180.00	: 1917:	14
	Groveland, NY	:	:	:	: :	
17.	Bradner Creek near	:04225600:	Crest-Stage:	9.7	: 1976:	1
	Dansville, NY	:	:	;	:	
18.	Keshequa Creek at Nunda, NY		•		: 1975:	
19.	Keshequa Creek at	:04225950:	Crest-Stage:	58.5	: 1976:	2
••	Tuscarora, NY	:			;	
20.	Keshequa Creek at Craig	:04226000:	Recording :	68.3	:Mar 1911:	19
21	Colony, at Sonyea, NY	:		70.0	:	
21.	Conesus Creek near	:04228000:	Recording :	72.0	Dec 1920:	15
2.2	Lakeville, NY	.04228870		7 /	. 1075.	2
22.	Little Conesus Creek near	:04228870:	Crest-Stage:	7.4	: 1975:	2
23.	South Lima, NY Little Conesus Creek near	* U7 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	: :Crest-Stage:	8.0	: : 1975:	2
۷,	East Avon, NY	.07220300	orest_orage:	0.0	, 17/J:	4
24.	Springwater Creek at	:04228900	: :Crest-Stage:	10.1	: 1964:	8
~ ~ •	Springwater, NY	:	ticse stage			
25.	Oatka Cr. at Rock Glen, NY	:04230320	Crest-Stage:	14.5	: 1975:	2
26.	Oatka Cr. at Pearl Cr., NY				1975:	
27.	Pearl Cr. at Pearl Cr., NY		-		1975:	
28.	Oatka Creek near Pavilion		Crest-Stage:		1975:	
-	Center, NY	:		:	:	
29.	Mad Creek near LeRoy, NY	:04230470:	Crest-Stage:	10.2	: 1975:	2
30.	Genesee River below Erie		•	2,457	: 1904:	15
50.						
30.	Canal at Rochester, NY	:	;	:	:	

Table A6 - Maximum Stages and Discharges Through 1984

		Inrou	30 15									
		:	:	Max.			:			:		
		:	:	Dis-			:	Max	-	:		
	Gage	:Gage		charge		Date	:	Stag	e**	:	Date	
		:	:	(cfs)	:		:			:		
		:	:		:		:			:		
		:		15,800								
1.												
2.	Black Creek at HYDER FLATS koad	:0422	1769:	3,500	:22	Mar	78:	1,761	ft	:22	Mar	72
	at Black Creek, NY	:	:	;	:		:			:		
3.	Wiscoy Creek at Bliss, NY	:0422	2600:	1,850	:23	Jun	72:	4.06	ft	:23	Jun	72
4.	Genesee River at	:0422	3000:	90,000	:23	Jun	72:	35.25	ft	:23	Jun	72
	Portageville, NY	:	:	;	:		:			:		
5.	Mt. Morris Lake near Mt.	:0422	4000:	-	:	-	:	755.46	ft	:25	Jun	72
	Morris, NY	:	:		:		:			:		
6.	Sugar Creek near Ossiam, NY	:0422	4700:	1,460	:18	Jun	84:	7.06	ft	:18	Jun	84
7.	Canaseraga Creek above	:0422	4775:	2,870	:20	Sep	77:	5.51	ft	:20	Sep	77
	Dansville, NY	:	:		:	-	:			:	•	
8.	Stony Brook Trib. at	:0422	4807:	7 9 0	: 3	Aug	81:	15.89	ft	: 3	Aug	81
	S. Dansville, NY	:	:	;	:	Ū	:			:		
9.	Mill Creek at Patchville, NY	:0422	4900:	1,860	: 5	Mar	64:	3.79	ft	: 5	Mar	64
10.				5,270					ft	: 4	Mar	76
	Crossing, NY	:		11,200						:	-	
11.	Genesee River near Mt.	:0422		55,100					ft	:	_	
	Morris, NY (Jones Bridge)	:		,	:	•	:			:		
12.	Conesus Lake near Lakeville, NY	:0422	7 9 80:	-	:	_	:	822.50		:24	Jun	72
13.				16,500	:25	Jun	72:	40.67	ft			
14.		:04228		-	:	_		806.91				
15.				4,630	:28	Mar						
	Falls, NY	:		6,600*						:	_	
16.		:04230		4,010					ft	:23	Jun	72
17.				7,050								
18.	· · · · · · · · · · · · · · · · · · ·	:04230		•	:	_		19.33				
•••	Bridge near Mortimer, NY	:	:		•		:	17,033		:		
19.		:0423	1000:	4,880	:31	Mar	60:	9.44	ft	:31	Mar	60
20.		:0423						12.78			Mar	
	near Churchville, NY	• ·			•		•	12.0		•		
21.		•0423	2000 •	48,300	• 30	Mar	16.	_		:	_	
41.	Ave. at Rochester, NY	• • • • • •		54,000*						•		
	Ave. at nothester, wi	•		, , , , , , , , , , , , , , , , , , , ,	0	rica III	•			:		
		•		 	•		<u></u>			<u>·</u>		

^{*} Outside Period of Record.

^{**} All lake elevation in feet NGVD

A5 FUTURE FLOODS

Floods of the same or larger magnitude as those that have previously occurred in the past could also occur in the future. Larger floods have been experienced in the past on streams with characteristics similar to those found in the study area. Combinations of rainfall and runoff similar to those which have occurred in those streams could also occur in the study area.

A6 DAMS AND RESERVOIRS

A6.1 Considered Sites

Various combinations of dams and reservoirs (scenarios) were analyzed in the Reconnaissance stage to develop hydropower and reduce flood damages. The four dams and reservoirs that were analyzed in the Reconnaissance stage were Stannard Dam and Reservoir, Portage Dam and Reservoir, Poag's Hole Dam and Reservoir, and Mount Morris Dam and Reservoir. Stannard, Portage and Poag's Hole were proposed *eservoirs previously analyzed in the June 1969 "Genesee River Basin Study." Poag's Hole and Portage were eliminated from further consideration in the Reconnaissance stage of this study. The physical characteristics and operating policies of Stannard would not vary among the plans, while the physical characteristics and/or operating policies could change for Mount Morris, depending upon the plan. The characteristics of Stannard Dam and Reservoir can be found on Table A7 and for the existing Mount Morris Dam, on Table A8. Changes in the characteristics for Mount Morris for the applicable scenarios can be found on Table A9. A description of the plans appears in Section A9. The considered location of the dams can be found on Plate A6. The plan view of Stannard Dam and Reservoir can be found on Plate A7, and for Mount Morris Dam and Reservoir on Plate A8.

A6.2 Reservoir Plans

In the initial phases of this study, 16 scenarios (or alternatives) were developed. After an initial screening of these 16 scenarioes were added to the no action plan to be evaluated more closely. Five alterntives were selected as plans to be further evaluated in the feasibility stage. They were:

- a. Plan 1: Re-regulation of Mount Morris Dam and Reservoir;
- b. Plan 2: No Federal action
- c. Plan 6: A dam and Reservoir at Stannards (Table A7), a multipurpose reservoir with hydropower development.
- d. Plan 10: Mount Morris Dam would be raised 27 feet. All the increase in storage would be used for downstream flood control. A run of the river hydropower plant would be built at the base of the dam. The description of Mount Morris Dam can be found on Table A8, with changes attributable to this plan on Table A19.
- e. Plan II: A system of reservoirs comprised of Stannards and Mount Morris for flood control, hydropower and irrigation.

	Table A7 - Stannard Dam and Reservoir	
:		:
:	Dam Data	:
:		:
	Elevation in feet NGVD of top of dam in feet NGVD	: 1,630
	Top width in feet	: 20
3:	Height above stream bed in feet	: 90
4:	Length in feet	: 2,300
:	0.411	:
:	Spillway Data	;
5 :	Number of gates	· : 4
	Size of gates in feet	: 47.5 by 27
	Elevation of top of gates in feet NGVD	: 1,620
	Crest of spillway elevation in feety NGVD	: 1,593
	Effective Length of Spillway in feet	: 190
	Maximum design head on crest in feet	; 32
	Design discharge in CFS	: 116,000
:	besign discharge in orb	: 110,000
:		:
:	Outlet Works Data	:
:		:
	Number of pipes	: 5
13:	Size of each pipe in sq. ft.	: 48
:	Denouge In Debe	:
:	Reservoir Data	; •
14:	Spillway design pool elevation in feet NGVD	: 1,625.5
	Maximum topography in feet NGVD	: 1,630.0
	Conservation pool in feet NGVD	: 1,593
	Flood control pool in feet NGVD	: 1,620
	Size of pool at maximum water surface in acres	2,440
	Size of conservation pool in acres	: 1,550
	Size of flood control pool in acres	: 2,330
	Channel elevation at toe of dam in feet NGVD	: 1,531
	Conservation storage in acre-ft.	: 39,500
	Flood Control storage in acre-ft.	: 54,000
	Flood Control storage in inches of runoff	: 5.7
	Dead storage in acre-ft.	: 2,500
:	2001 3001060 111 0010 100	:
:	Hydropower Data	:
26	Mandaum hudmanasam haad da faat	:
	Maximum hydropower head in feet	: 80
	Minimum hydropower head in feet	: 48
	Head used in Hydur in feet	: 64
29:	Minimum flow in CFS	: 35
<u>-</u> :		<u>.</u>

Table A8 - Mount Morris Dam and Reservoir

:	Dom Data	:	
	Dam Data	•	
1 .	Elevation of top of dam in feet NGVD	•	790
	Top width in feet	•	20
	•	•	
	Height above stream bed in feet	:	215
4:	Length in feet	;	1,028
•	Spillway Data	•	
•	Spiliway Data	•	
5:	Number of gates	:	_
	Size of gates in feet	:	_
	Elevation of top of gates in feet NGVD	:	-
	Crest of spillway elevation in feet NGVD	:	760
	Effective Length of Spillway in feet	:	550
	Maximum design head on crest in feet	:	28
	Design discharge in CFS	:	320,000
:		:	,
:		:	
:	Outlet Works Data	:	
:		:	
12:	Number of pipes	:	9
13:	Size of each pipe in sq. ft.	:	35
:	• • •	:	
:	Reservoir Data	:	
:		:	
14:	Spillway design pool elevation in feet NGVD	:	788
15:	Maximum topography in feet NGVD	:	-
16:	Conservation pool in feet NGVD	:	-
17:	Flood control pool in feet NGVD	:	760
18:	Size of pool at maximum water surface in acres	:	3,680
19:	Size of conservation pool in acres	:	· -
20:	Size of flood control pool in acres	:	3,300
21:	Channel elevation at toe of dam in feet NGVD	:	575
22:	Conservation storage in acre-ft.	:	-
	Flood Control storage in acre-ft.	:	301,000
	Flood Control storage in inches of runoff	:	5.24
	Dead storage in acre-ft.	:	610
:	-	:	

Table A9 - Considered Changes to Mount Morris Dam

		
	:	
Plan 10	:	
	:	
Elevation of top of dam in feet NGVD	:	817
Height above stream bed in feet	:	242
Length of dam in feet	:	1,030
Crest of spillway elevation in feet NGVD	:	787
Spillway design pool elevation in feet NGVD	:	815
Flood control pool elevation in feet NGVD	:	787
Size of pool at maximum water surface in acres	:	3,828
Size of flood control pool in acres	:	3,436
Flood Control storage in acre-ft.	:	406,000
Flood Control storage in inches of runoff	:	7.05
Head used in HYDUR in feet	:	36
	:	
Plan 11	:	
	:	
Conservation pool elevation in feet NGVD	:	645
Size of conservation pool in acres	:	1,455
Conservation storage in acre-ft.	:	54,000
Flood Control storage in acre-ft.	:	247,600
Flood Control storage in inches of runoff	:	4.30
Head used in HYDUR	:	50
	<u>;</u>	

A7 FLOOD PROBABILITIES

A7.1 General

In the following description, "natural" conditions refers to conditions without the present Mount Morris project; "existing" conditions refers to the regulated state of flows with the existing project; and "with-project" conditions refers to conditions which would prevail if Mount Morris Dam were modified as a result of the plans considered herein.

Basic data on the Genesee River reaches is given in Table AlO. The discharge-frequency curve for Wellsville-Reach A was used for reaches G-1 through G-4 in Wellsville, and the discharge-frequency for Wellsville-Reach B was used for Reaches G-5A, G-5B, and G-6 in Wellsville.

Because hydroelectric power was dropped as a project purpose prior to completion of the Feasibility Report, and due to the resulting early elimination of Plans 6, 10 and 11 as explained on pages 4-11 and 4-12 of the Main Report, a comprehensive model permitting simulation of short-term variations in flow from multiple reservoirs was not developed.

Table AlO - Genesee River Reaches

	:		:	:	Drainage Are	a:	Drainage Area	:	
Reach	:	Reach Name	: Gage Used	<u>:</u>	at Gage	:	at Site	:	Factor
	:		:	:		:		:	
Reach l		Rochester	: Rochester	:	2467	:	2467	:	1.00
Reach 2	:	Chili-Henrietta	: Rochester	:	2467	:	2411	:	0.980
Reach 3	:	Avon	: Avon	:	1673	:	1978	:	1.163
Reach 4	:	Geneseo	: Jones Bridge	: :	1424	:	1424	:	1.00
Reach 5	:	Mt. Morris	: -	:	-	:	-	:	-
Reach 6	:	Portageville	: Portageville	:	984	:	984	:	1.00
Reach 7	:	Fillmore	: Portageville	:	984	:	726	:	.761
Reach 8	:	Belfast	: Portageville	: :	984	:	641	:	.680
Reach 9	:	Belvidere	: Portageville	:	984	:	483	:	.527
Reach 10	:	Belmont	: Wellsville	:	288	:	418	:	1.398
Reach 11	:	Scio	: Wellsville	:	288	:	309	:	1.065
Reach 12	:	Wellsville	: Weilsville	:	288	:	288	:	1.000
(G-1)	:		:	:		:		:	
Reach 12	:	Wellsville	: Wellsville	:	288	:	288	:	1.000
(G-2)	:		:	:		:		:	
Reach 12	:	Wellsville	: Wellsville	:	288	:	288	:	1.000
(G-3)	:		:	:		:		:	
Reach 12	:	Wellsville	: Wellsville	:	288	:	288	:	1.000
(G-4)	:		:	:		:		:	
Reach 12	:	Wellsville	: Wellsville	:	288	:	216	:	.772
(G-5A)	:		:	:		:		:	
	:	Wellsville	: Wellsville	:	288	:	216	:	.772
(G-5B)	:		:	:		:		:	-
•	:	Wellsville	: Wellsville	:	288	:	216	:	.772
(G-6)	:		:	:		:		:	_
	:	Stannards	: Wellsville	:	288	:	212	:	.759
		Corners	:	:		:		:	
Reach 14			: Wellsville	:	288	:	179	:	.652
	•		:	:		•	- · -	•	

The primary hydrologic investigation effort then involved the development of expected flood damages under existing and with-project conditions for the reaches which would be affected by a combination of Plan 1 and Plan 10 which would include only Mount Morris Dam, that is, Plan 10A.

The general procedure used was to develop the existing discharge-frequency relationships for the affected damage reaches, then to modify them to produce with-project discharge-frequency relationships which account for the considered increased storage capability in Mount Morris Reservoir under with-project conditions.

A7.2 Existing Conditions

The discharge-frequency curves for the stream gages located at Genesee River at Wellsville (04221000), Genesee River at Portageville (04223000), and Canaseraga Creek above Dansville (04224775), were unchanged from the Reconnaissance Report. These curves can be found on Figures Al through A3. The discharge values used for the frequency analyses can be found on Table All. These discharge-frequency curves were used to calculate discharge-frequency relationships for each upstream damage reach on the Genesee River. These curves were calculated using the HEC's microcomputer version of HECWRC (flood flow frequency), dated 14 June 1985.

Bulletin 17B discharge-frequency curves were adapted to the rest of the damage reaches by using the equation $[0_2/Q_1] = (A_2/A_1)$ EXP .9], developed for the Irondequoit Creek Study (July 1981). This equation can be used to estimate discharge frequency curves upstream and downstream of a gage site, as long as the drainage area at the ungaged site is within these limits:

A2 >.5A1 and A2 < 1.5A1. A2 is the drainage area at the ungaged site, and A1 is the drainage area at the gaged site. Q2 is the discharge at the gaged site. Tables A12a and A12b give the discharge frequencies at the upstream index points.

There are four damage reaches downstream of the Mount Morris Dam. Three of these reaches have index points which can be represented by existing discharge-frequency curves at Genesee River stream-gaging stations, since the index points represent drainage areas which are within 5% of the drainage areas at the respective gages. These reaches are described in Table A2 and are designated as follows: Rochester (Index Point #1, U.S.G.S. Gage #04232000); Chili-Henrietta (I.P. #3, Gage #042320000); and Geneseo (I.P. #4, Gage #04227500). Additional information on the gages is given in Table A4. The discharge-frequency relationships for existing conditions for Reach 4 are given in Table A12a and Figure A4. For Reaches 1 and 2, the same gage was used as an index point, and the discharge-frequency relationship for these reaches for existing conditions are given in Table A12a and Figure A6.



For the Avon reach, (Index Point #3), the damage reach was represented by an index point which was not coincident with a stream gaging station. It was therefore necessary to develop an existing-condition discharge-frequency relationship at the index point. The gage record from 1953 through 1984 was used to develop a discharge-frequency curve for existing conditions at the Avon Gage (#04228500). This relationship is shown on Table Al2a and Figure A5.

Because of the existing regulation of the river by Mount Morris Dam, the discharge-frequency relationships for existing conditions could not be analyzed by regional methods. Therefore, a procedure was required which would allow use of discharge frequencies for natural conditions, which can be estimated at ungaged locations. To accomplish this, regional frequency studies were done, to develop discharge frequencies for natural conditions at the Avon gage (Figure A7) and at Index Point #3 (Figure A8).

Differences were developed between the discharges at given frequencies for natural conditions at the Avon gage and at Index Point #3. These differences were then applied to the natural-condition discharges at Index Point #3 to develop discharge frequencies for existing conditions at that point. Two different procedures for weighting these differences were considered, and gave differing results. Since neither procedure was considered clearly more representative than the other, the averages of the values obtained by each were used.

A7.3 Improved Conditions

For the upstream reaches, improved condition discharge frequencies were unchanged from the Reconnaissance Study and are shown on Tables Al3a and Al3b.

For the downstream Genesee River reaches, once the discharge-frequency relationships were established for existing conditions at each of the index points, discharge-frequency relationships for with-project conditions were developed as described below.

At the index points for Reaches 1, 2, and 4, the with-project condition discharge-frequency curves were calculated by using rainfall-frequency curves from TP-40 (Figure A9) combined with values of increased storage for Mount Morris Reservoir for each modified condition. The modifications being considered all involved adding gates to the spillway, thus increasing the flood control storage available. Two conditions were considered: adding 15 ft gates; and adding 30 ft gates atop the existing spillway. Pertinent data for these conditions is given in Table A14, and the discharge-frequency curves are shown on Figures A4 and A6.

The synthesized existing condition discharge-frequency relationship for Avon, described above, was used to develop the with-project discharge-frequency relationship for Index Point #3 (Figure AlO). The procedure has the effect of increasing flow differences by a drainage area factor, but to a greater degree than would be done for peak flows. This is considered reasonable, since lower flows tend to be more directly related to drainage area than peak flows.

Table All - Peak Discharge Values Used in Frequency Analysis

Year	Water	::		:		•	 	:		-		:	
1909			llsville	e:Po	rtagevil	le:	Canaseraga	:Jones	Bridge	e:	Avon	:	Rochester
1910 :		:		:		:		:		:		÷	
1910 :	1909	:	_	:	26,300	:	-	:	_	:	-	:	-
1911 :		:	-	:		:	-	:	-	:	_	:	_
1912 :		:	_	:		:	1.880	:	_	:	-	:	_
1913 :		•	_	:		:		:	_	:	-	:	_
1914			_	:		:	-	:	_	:	_	:	_
1915 :			-	•		•	-	•	_	•	_	•	_
1916			_	•		•	-	•	_		_	•	-
1917 : 5,020 : 20,100 : 3,180 : - : - : - : - 1918 : 13,100 : 29,500 : 3,790 : : - : - : 1919 : 13,900 : 32,800 : 6,540 : - : - : - : 1920 : 8,810 : 39,700 : 3,950 : - : - : - : 1921 : 3,810 : 18,100 : 1,270 : : - : : 1921 : 3,810 : 18,100 : 1,270 : : - : : 1922 : 4,230 : 12,600 : 3,500 : - : - : - : 1923 : 6,010 : 21,900 : 3,570 : - : - : - : 1924 : 8,320 : 17,600 : 1,830 : - : - : - : 1925 : 8,140 : 17,500 : 3,240 : - : - : - : 1926 : 3,530 : 14,800 : 2,100 : - : - : - : 1927 : 6,090 : 15,800 : 2,600 : - : - : - : 1929 : 6,800 : 26,700 : 6,900 : - : - : - : 1929 : 6,800 : 26,700 : 6,900 : - : - : - : 1930 : 4,710 : 20,500 : 1,850 : - : - : - : 1931 : 3,020 : 11,600 : 2,400 : - : - : - : 1932 : 3,970 : 14,200 : 3,350 : - : - : : 1933 : 3,410 : 17,200 : 3,160 : - : - : - : 1934 : 4,710 : 15,200 : 2,360 : - : - : - : 1935 : 8,560 : 17,400 : 8,390 : - : - : : 1938 : 7,120 : 17,200 : 4,420 : - : - : : 1938 : 7,120 : 17,200 : 4,420 : - : - : : 1938 : 7,120 : 17,200 : 4,420 : - : - : : 1938 : 7,120 : 17,200 : 4,420 : - : - : - : 1938 : 7,120 : 17,200 : 4,460 : - : - : - : 1944 : 4,810 : 15,200 : 2,830 : - : - : - : 1942 : 11,200 : 32,300 : 8,400 : - : - : - : 1942 : 11,200 : 32,300 : 8,400 : - : - : - :		-	_	:		:	6 600	•	_	:	_	:	-
1918 : 13,100 : 29,500 : 3,790 : - : - : - : - 1919 : 13,900 : 32,800 : 6,540 : - : - : - : - : - 1920 : 8,810 : 39,700 : 3,950 : - : - : - : - : - 1921 : 3,810 : 18,100 : 1,270 : - : - : - : - : - 1922 : 4,230 : 12,600 : 3,500 : - : - : - : - : - 1924 : 8,320 : 17,600 : 1,830 : - : - : - : - : - 1924 : 8,320 : 17,600 : 1,830 : - : - : - : - : - 1925 : 8,140 : 17,500 : 3,240 : - : - : - : - 1926 : 3,530 : 14,800 : 2,100 : - : - : - : - : - 1927 : 6,090 : 15,800 : 2,600 : - : - : - : - : - 1928 : 7,600 : 42,700 : 6,900 : - : - : - : - 1929 : 6,800 : 26,700 : 6,150 : - : - : - : - 1930 : 4,710 : 20,500 : 11,850 : - : - : - : - : - 1931 : 3,020 : 11,600 : 2,400 : - : - : - : - 1931 : 3,020 : 11,600 : 2,400 : - : - : - : - 1933 : 3,970 : 14,200 : 3,350 : - : - : - : - 1934 : 4,710 : 15,200 : 2,360 : - : - : - : - 1934 : 4,710 : 15,200 : 2,360 : - : - : - : - 1934 : 4,710 : 15,200 : 2,360 : - : - : - : - 1936 : 8,550 : 25,500 : 4,240 : - : - : - : - 1937 : 6,640 : 18,100 : 2,830 : - : - : - : - 1937 : 6,640 : 18,100 : 2,830 : - : - : - : - 1939 : 7,180 : 28,400 : 4,860 : - : - : - : - 1944 : 4,810 : 15,200 : 4,420 : - : - : - : - 1944 : 4,810 : 15,200 : 4,420 : - : - : - : - 1944 : 4,810 : 15,200 : 4,420 : - : - : - : - 1944 : 4,810 : 15,200 : 4,420 : - : - : - : - 1944 : 4,810 : 15,200 : 4,420 : - : - : - : - 1944 : 4,810 : 15,200 : 4,420 : - : - : - : - 1944 : 4,810 : 15,200 : 4,420 : - : - : - : - 1944 : 4,810 : 15,200 : 4,420 : - : - : - : - : - 1944 : 4,810 : 15,200 : 4,420 : - : - : - : - : - 1944 : 4,810 : 15,200 : 4,420 : - : - : - : - : - 1944 : 4,810 : 15,200 : 4,400 : - : - : - : - : - : - 1944 : 4,810 : 15,200 : 4,400 : - : - : - : - : - : - 1944 : 4,810 : 15,200 : 4,400 : - : - : - : - : - : - 1944 : 4,810 : 15,200 : 4,400 : - : - : - : - : - : - 1944 : 4,810 : 15,200 : 4,400 : - : - : - : - : - : - 1944 : 4,810 : 15,200 : 4,400 : - : - : - : - : - : - 1944 : 4,810 : 15,200 : 4,400 : - : - : - : - : - : - 1944 : 4,810 : 15,200 : 4,400 : - : - : - : - : - : - : - 1944 : 1,400 : 28,300 : 4,910 : - : - : -			5 020	:		:		•	_	:	_	:	_
1919 13,900 32,800 6,540 - - - -						•		•	_	:	_	•	-
1920 : 8,810 : 39,700 : 3,950 : - : - : - : - 1921 : 3,810 : 18,100 : 1,270 : - : - : - : - : - 1922 : 4,230 : 12,600 : 3,500 : - : - : - :						:		•	_	:	_	:	=
1921 : 3,810 : 18,100 : 1,270 : - :<				-		•		•	_	:	_	:	_
1922 : 4,230 : 12,600 : 3,500 : - :<						•		•	_	:	_	:	_
1923 : 6,010 : 21,900 : 3,570 : - : - : - :				-				•	_	:	_	:	_
1924 : 8,320 : 17,600 : 1,830 : - : - : - : - 1925 : 8,140 : 17,500 : 3,240 : - : - : - : - 1926 : 3,530 : 14,800 : 2,600 : - : - : - : - 1927 : 6,090 : 15,800 : 2,600 : - : - : - : - 1928 : 7,600 : 42,700 : 6,900 : - : - : - : - 1928 : 7,600 : 42,700 : 6,900 : - : - : - : - 1930 : 4,710 : 20,500 : 1,850 : - : - : - : - 1931 : 3,020 : 11,600 : 2,400 : - : - : - : - 1932 : 3,970 : 14,200 : 3,350 : - : - : - : - 1933 : 3,410 : 17,200 : 3,160 : - : - : - : - 1934 : 4,710 : 15,200 : 2,360 : - : - : - : - 1935 : 8,560 : 17,400 : 8,390 : - : - : - : - 1937 : 6,640 : 18,100 : 2,830 : - : - : - : - 1937 : 6,640 : 18,100 : 2,830 : - : - : - : - 1939 : 7,180 : 28,400 : 4,860 : - : - : - : - 1941 : 7,200 : 29,400 : 5,860 : - : - : - : - 1941 : 7,200 : 29,400 : 5,860 : - : - : - : - 1941 : 7,200 : 29,400 : 5,860 : - : - : - : - 1940 : 8,530 : 26,500 : 9,110 : - : - : - : - 1941 : 7,200 : 29,400 : 5,860 : - : - : - : - 1942 : 11,200 : 32,300 : 8,400 : - : - : - : - 1945 : 8,390 : 24,600 : 6,370 : - : - : - : - 1945 : 8,390 : 24,600 : 6,370 : - : - : - : - 1947 : 10,600 : 28,300 : 4,910 : - : - : - : - 1948 : 13,300 : 35,800 : 7,170 : - : - : - : - 1950 : 13,90				-				•	_	:	_	:	_
1925 : 8,140 : 17,500 : 3,240 : - :<		-						•	_	:	_	:	_
1926 : 3,530 : 14,800 : 2,100 : - :<									_	•	_	•	_
1927 : 6,090 : 15,800 : 2,600 : - :<				-				•	_	•	_	•	_
1928 : 7,600 : 42,700 : 6,900 : - : - : - : - 1929 : 6,800 : 26,700 : 6,150 : - : - : - 1930 : 4,710 : 20,500 : 1,850 : - : - : - 1931 : 3,020 : 11,600 : 2,400 : - : - : - : - 1932 : 3,970 : 14,200 : 3,350 : - : - : - : - 1933 : 3,410 : 17,200 : 3,160 : - : - : - : - 1934 : 4,710 : 15,200 : 2,360 : - : - : - : - 1935 : 8,560 : 17,400 : 8,390 : - : - : - : - 1936 : 8,550 : 25,500 : 4,240 : - : - : - : - 1937 : 6,640 : 18,100 : 2,830 : - : - : - : - 1938 : 7,120 : 17,200 : 4,420 : - : - : - : - 1939 : 7,180 : 28,400 : 4,860 : - : - : - : - 1940 : 8,530 : 26,500 : 9,110 : - : - : - : - 1941 : 7,200 : 29,400 : 5,860 : - : - : - : - 1942 : 11,200 : 32,300 : 8,400 : - : - : - : - 1943 : 8,290 : 27,400 : 7,560 : - : - : - : - 1944 : 4,810 : 15,200 : 4,490 : - : - : - : - 1945 : 8,390 : 24,600 : 6,370 : - : - : - : - 1946 : 17,900 : 25,200 : 4,260 : - : - : - : - 1948 : 13,300 : 24,100 : 6,350 : - : - : - : - 1949 : 2,690 : 9,660 : 1,680 : - : - : - : - 1950 : 13,900 : 35,800 : 7,170 : - : - : - : - 1951 : 23,300 : 35,800 : 6,150 : - : - : - : - : - 1953 : 8,750 : 21,000 : 3,630 : - : 17,100 1954 : 5,360 : 15,400 : 5,240 : 13,800 : - : 17,100 195									_	•	_	•	_
1929 : 6,800 : 26,700 : 6,150 : - : - : - : - 1930 : 4,710 : 20,500 : 1,850 : - : - : - : - 1931 : 3,020 : 11,600 : 2,400 : - : - : - : - 1932 : 3,970 : 14,200 : 3,350 : - : - : - : - 1933 : 3,410 : 17,200 : 3,160 : - : - : - : - 1934 : 4,710 : 15,200 : 2,360 : - : - : - : - 1935 : 8,560 : 17,400 : 8,390 : - : - : - : - 1936 : 8,550 : 25,500 : 4,240 : - : - : - : - 1937 : 6,640 : 18,100 : 2,830 : - : - : - : - 1938 : 7,120 : 17,200 : 4,420 : - : - : - : - 1939 : 7,180 : 28,400 : 4,860 : - : - : - : - 1941 : 7,200 : 29,400 : 5,860 : - : - : - : - 1942 : 11,200 : 32,300 : 8,400 : - : - : - : - 1944 : 4,810 : 15,200 : 4,490 : - : - : - : - 1944 : 4,810 : 15,200 : 4,490 : - : - : - : - 1945 : 8,390 : 24,600 : 6,370 : - : - : - : - 1946 : 17,900 : 25,200 : 4,260 : - : - : - : - 1947 : 10,600 : 28,300 : 4,910 : - : - : - : - 1948 : 13,300 : 24,100 : 6,350 : - : - : - : - 1950 : 13,900 : 35,800 : 7,170 : - : - : - : - 1951 : 23,300 : 35,800 : 7,170 : - : - : - : - 1953 : 8,750 : 21,000 : 3,790 : 10,800 : - : 17,500								•		•	_	•	-
1930 : 4,710 : 20,500 : 1,850 : - : - : - 1931 : 3,020 : 11,600 : 2,400 : - : - : - 1932 : 3,970 : 14,200 : 3,350 : - : - : - 1933 : 3,410 : 17,200 : 3,160 : - : - : - 1934 : 4,710 : 15,200 : 2,360 : - : - : - 1935 : 8,560 : 17,400 : 8,390 : - : - : - 1936 : 8,550 : 25,500 : 4,240 : - : - : - 1937 : 6,640 : 18,100 : 2,830 : - : - : - 1938 : 7,120 : 17,200 : 4,420 : - : - : - 1939 : 7,180 : 28,400 : 4,860 : - : - : - 1940 : 8,530 : 26,500 : 9,110 : - : - : - 1941 : 7,200 : 29,400 : 5,860 : - : - : - 1942 : 11,200 : 32,300 : 8,400 : - : - : - 1943 : 8,290 : 27,400 : 7,560 : - : - : - 1944 : 4,810 : 15,200 : 4,490 : - : - : - 1945 : 8,390 : 24,600 : 6,370 : - : - : - 1946 : 17,900 : 25,200 : 4,260 : - : - : - 1948 : 13,300 : 24,100 : 6,350 : - : - : - 1949 : 2,690 : 9,660 : 1,680 : - : - : - 1950 : 13,900 : 35,800 : 7,170 : : - : - 1951 : 23,300 : 35,800 : 7,170 : : - : - 1953 : 8,750 : 21,000 : 3,790 : 10,800 : - : 17,500				-				•	-	•	•	•	-
1931: 3,020: 11,600: 2,400: - :								:	-	•	-	:	-
1932 : 3,970 : 14,200 : 3,350 : - : - : - : - 1933 : 3,410 : 17,200 : 3,160 : - : - : - 1934 : 4,710 : 15,200 : 2,360 : - : - : - 1935 : 8,560 : 17,400 : 8,390 : - : - : - 1936 : 8,550 : 25,500 : 4,240 : - : - : - 1937 : 6,640 : 18,100 : 2,830 : - : - : - 1938 : 7,120 : 17,200 : 4,420 : - : - : - 1939 : 7,180 : 28,400 : 4,860 : - : - : - : - 1941 : 7,200 : 29,400 : 5,860 : - : - : - : - 1941 : 7,200 : 29,400 : 5,860 : - : - : - : - 1942 : 11,200 : 32,300 : 8,400 : - : - : - : - 1944 : 4,810 : 15,200 : 4,490 : - : - : - : - 1944 : 4,810 : 15,200 : 4,490 : - : - : - : - 1945 : 8,390 : 24,600 : 6,370 : - : - : - : - 1947 : 10,600 : 28,300 : 4,910 : - : - : - : - 1948 : 13,300 : 24,100 : 6,350 : - : - : - : - 1949 : 2,690 : 9,660 : 1,680 : - : - : - : - 1950 : 13,900 : 35,800 : 6,150 : - : - : - : - 1951 : 23,300 : 35,800 : 6,150 : - : - : - : - 1953 : 8,750 : 21,000 : 3,790 : 10,800 : - : 17,100 1954 : 5,360 : 15,400 : 5,240 : 13,800 : - : 17,500				-				:	-	•	-	•	-
1933 : 3,410 : 17,200 : 3,160 : - :<								:	-	:	-	:	-
1934: 4,710: 15,200: 2,360: -::				-		:		:	-	:	-	:	-
1935 : 8,560 : 17,400 : 8,390 : - : - : - : - 1936 : 8,550 : 25,500 : 4,240 : - : - : - 1937 : 6,640 : 18,100 : 2,830 : - : - : - 1938 : 7,120 : 17,200 : 4,420 : - : - : - 1939 : 7,180 : 28,400 : 4,860 : - : - : - : - 1940 : 8,530 : 26,500 : 9,110 : - : - : - : - 1941 : 7,200 : 29,400 : 5,860 : - : - : - : - 1942 : 11,200 : 32,300 : 8,400 : - : - : - : - 1943 : 8,290 : 27,400 : 7,560 : - : - : - : - 1944 : 4,810 : 15,200 : 4,490 : - : - : - : - 1945 : 8,390 : 24,600 : 6,370 : - : - : - : - 1946 : 17,900 : 25,200 : 4,260 : - : - : - : - 1947 : 10,600 : 28,300 : 4,910 : - : - : - : - 1948 : 13,300 : 24,100 : 6,350 : - : - : - : - 1949 : 2,690 : 9,660 : 1,680 : - : - : - : - 1950 : 13,900 : 35,800 : 7,170 : - : - : - : - 1951 : 23,300 : 35,800 : 6,150 : - : - : - : - 1953 : 8,750 : 21,000 : 3,790 : 10,800 : - : 17,100 1954 : 5,360 : 15,400 : 5,240 : 13,800 : - : 17,500						:		:	-	:	_	:	-
1936: 8,550 : 25,500 : 4,240 : - <td></td> <td></td> <td></td> <td></td> <td></td> <td>:</td> <td></td> <td>:</td> <td>-</td> <td>:</td> <td>•=</td> <td>:</td> <td>-</td>						:		:	-	:	•=	:	-
1937: 6,640 : 18,100 : 2,830 : - <td></td> <td></td> <td></td> <td></td> <td></td> <td>:</td> <td>8,390</td> <td>:</td> <td>-</td> <td>:</td> <td>-</td> <td>:</td> <td>-</td>						:	8,390	:	-	:	-	:	-
1938: 7,120: 17,200: 4,420: - :				-		:	4,240	:	-	:	-	:	-
1939: 7,180 : 28,400 : 4,860 : - <td></td> <td>:</td> <td></td> <td>:</td> <td></td> <td>:</td> <td>2,830</td> <td>:</td> <td>-</td> <td>:</td> <td>-</td> <td>:</td> <td>-</td>		:		:		:	2,830	:	-	:	-	:	-
1940 : 8,530 : 26,500 : 9,110 : - :<				:		:		:	-	:	-	:	-
1941 : 7,200 : 29,400 : 5,860 : - - : - : - : - 1942 : 11,200 : 32,300 : 8,400 : - : - : - : - : - 1943 : 8,290 : 27,400 : 7,560 : - : - : - : - : - 1944 : 4,810 : 15,200 : 4,490 : - : - : - : - 1945 : 8,390 : 24,600 : 6,370 : - : - : - : - 1946 : 17,900 : 25,200 : 4,260 : - : - : - : - 1947 : 10,600 : 28,300 : 4,910 : - : - : - : - 1948 : 13,300 : 24,100 : 6,350 : - : - : - : - 1949 : 2,690 : 9,660 : 1,680 : - : - : - : - 1950 : 13,900 : 35,800 : 7,170 : - : - : - : - 1951 : 23,300 : 35,800 : 6,150 : - : - : - : - 1952 : 7,500 : 23,200 : 3,630 : - : - : - : - 1953 : 8,750 : 21,000 : 3,790 : 10,800 : - : 17,100 1954 : 5,360 : 15,400 : 5,240 : 13,800 : - : 17,500		:		:		:		:	-	:	-	:	-
1942 : 11,200 : 32,300 : 8,400 : - : - : - 1943 : 8,290 : 27,400 : 7,560 : - : - : - 1944 : 4,810 : 15,200 : 4,490 : - : - : - 1945 : 8,390 : 24,600 : 6,370 : - : - : - 1946 : 17,900 : 25,200 : 4,260 : - : - : - 1947 : 10,600 : 28,300 : 4,910 : - : - : - 1948 : 13,300 : 24,100 : 6,350 : - : - : - : - 1949 : 2,690 : 9,660 : 1,680 : - : - : - : - 1950 : 13,900 : 35,800 : 7,170 : - : - : - : - 1951 : 23,300 : 35,800 : 6,150 : - : - : - : - 1953 : 8,750 : 21,000 : 3,790 : 10,800 : - : 17,100 1954 : 5,360 : 15,400 : 5,240 : 13,800 : - : 17,500		:		:		:		:	-	:	-	:	-
1943 : 8,290 : 27,400 : 7,560 : - : - : - 1944 : 4,810 : 15,200 : 4,490 : - : - : - 1945 : 8,390 : 24,600 : 6,370 : - : - : - 1946 : 17,900 : 25,200 : 4,260 : - : - : - 1947 : 10,600 : 28,300 : 4,910 : - : - : - 1948 : 13,300 : 24,100 : 6,350 : - : - : - : - 1949 : 2,690 : 9,660 : 1,680 : - : - : - : - 1950 : 13,900 : 35,800 : 7,170 : - : - : - 1951 : 23,300 : 35,800 : 6,150 : - : - : - : - 1953 : 8,750 : 21,000 : 3,790 : 10,800 : - : 17,100 1954 : 5,360 : 15,400 : 5,240 : 13,800 : - : 17,500				:		:		:	-	:	-	:	-
1944 : 4,810 : 15,200 : 4,490 : - : - : - 1945 : 8,390 : 24,600 : 6,370 : - : - : - 1946 : 17,900 : 25,200 : 4,260 : - : - : - 1947 : 10,600 : 28,300 : 4,910 : - : - : - 1948 : 13,300 : 24,100 : 6,350 : - : - : - : - 1949 : 2,690 : 9,660 : 1,680 : - : - : - : - 1950 : 13,900 : 35,800 : 7,170 : - : - : - 1951 : 23,300 : 35,800 : 6,150 : - : - : - 1952 : 7,500 : 23,200 : 3,630 : - : - : - : - 1953 : 8,750 : 21,000 : 3,790 : 10,800 : - : 17,100 1954 : 5,360 : 15,400 : 5,240 : 13,800 : - : 17,500				:		:		:	-	:	-	:	-
1945 : 8,390 : 24,600 : 6,370 : - : - : - 1946 : 17,900 : 25,200 : 4,260 : - : - : - 1947 : 10,600 : 28,300 : 4,910 : - : - : - 1948 : 13,300 : 24,100 : 6,350 : - : - : - : - 1949 : 2,690 : 9,660 : 1,680 : - : - : - : - 1950 : 13,900 : 35,800 : 7,170 : - : - : - 1951 : 23,300 : 35,800 : 6,150 : - : - : - 1952 : 7,500 : 23,200 : 3,630 : - : - : - : - 1953 : 8,750 : 21,000 : 3,790 : 10,800 : - : 17,100 1954 : 5,360 : 15,400 : 5,240 : 13,800 : - : 17,500				:		:		:	-	:	-	:	-
1946: 17,900: 25,200: 4,260: - : <td></td> <td></td> <td></td> <td>:</td> <td></td> <td>:</td> <td></td> <td>:</td> <td>-</td> <td>:</td> <td>-</td> <td>:</td> <td>-</td>				:		:		:	-	:	-	:	-
1947: 10,600 : 28,300 : 4,910 : - : - : - 1948: 13,300 : 24,100 : 6,350 : - : - : - 1949: 2,690 : 9,660 : 1,680 : - : - : - 1950: 13,900 : 35,800 : 7,170 : - : - : - 1951: 23,300 : 35,800 : 6,150 : - : - : - 1952: 7,500 : 23,200 : 3,630 : - : - : - 1953: 8,750 : 21,000 : 3,790 : 10,800 : - : 17,100 1954: 5,360 : 15,400 : 5,240 : 13,800 : - : 17,500	1945	:	8,390	:	24,600	:	6,370	:	-	:	-	:	-
1948 : 13,300 : 24,100 : 6,350 : - : - : - 1949 : 2,690 : 9,660 : 1,680 : - : - : - 1950 : 13,900 : 35,800 : 7,170 : - : - : - 1951 : 23,300 : 35,800 : 6,150 : - : - : - 1952 : 7,500 : 23,200 : 3,630 : - : - : - 1953 : 8,750 : 21,000 : 3,790 : 10,800 : - : 17,100 1954 : 5,360 : 15,400 : 5,240 : 13,800 : - : 17,500	1946	: .	17,900	:	25,200	:	4,260	:	-	:	-	:	-
1949: 2,690: 9,660: 1,680: - : 17,100 : 1954:: : 5,360:: : 15,400:: : 5,240:: : 13,800:: - : 17,500	1947	:	10,600	:	28,300	:	4,910	:	-	:	-	:	-
1950 : 13,900 : 35,800 : 7,170 : - : - : - 1951 : 23,300 : 35,800 : 6,150 : - : - : - 1952 : 7,500 : 23,200 : 3,630 : - : - : - 1953 : 8,750 : 21,000 : 3,790 : 10,800 : - : 17,100 1954 : 5,360 : 15,400 : 5,240 : 13,800 : - : 17,500	1948	:	13,300	:	24,100	:	6,350	:	-	:	-	:	-
1950 : 13,900 : 35,800 : 7,170 : - : - : - 1951 : 23,300 : 35,800 : 6,150 : - : - : - 1952 : 7,500 : 23,200 : 3,630 : - : - : - 1953 : 8,750 : 21,000 : 3,790 : 10,800 : - : 17,100 1954 : 5,360 : 15,400 : 5,240 : 13,800 : - : 17,500	1949			:	9,660	:		:	_	:	-	:	-
1951: 23,300 : 35,800 : 6,150 : - : - : - 1952: 7,500 : 23,200 : 3,630 : - : - : - 1953: 8,750 : 21,000 : 3,790 : 10,800 : - : 17,100 1954: 5,360 : 15,400 : 5,240 : 13,800 : - : 17,500	1950	:		:		:		:	-	:	-	:	_
1952 : 7,500 : 23,200 : 3,630 : - : - : - : - 1953 : 8,750 : 21,000 : 3,790 : 10,800 : - : 17,100 1954 : 5,360 : 15,400 : 5,240 : 13,800 : - : 17,500	1951			:		:		:	-	:	-	:	-
1953: 8,750: 21,000: 3,790: 10,800: -: 17,100 1954: 5,360: 15,400: 5,240: 13,800: -: 17,500				:		:		:	-	:	-	:	-
1954: 5,360: 15,400: 5,240: 13,800: -: 17,500				:	•	:		: 10	,800	:	_	:	17,100
		:		:		:	•		-	:	-	:	
		:		:	-	:		:	-	:		:	·

Table All - Peak Discharge Values Used in Frequency Analysis (Cont'd)

Water	::		:		-:-		:		-:		$\overline{\cdot}$	
Year	:W	ellsvill	e:P	ortagevil.	le:Ca	naseraga	:J	ones Bridg	e:	Avon	:	Rochester
	:		:				:		:		:	
1955	:	6,730	:	20,700	:	3,990	:	12,800	:	-	:	19,100
1956	:	16,900	:	43,300	:	4,500	:	11,900	:	15,600	:	24,300
1957	:	8,240	:	19,700	:	2,970	:	11,600	:	12,400	:	17,000
1958	:	7,950	:	19,300	:	2,910	:	10,700	:	10,800	:	14,900
1959	:	19,500	:	37,600	:	6,000	:	12,100	:	9,720	:	17,700
1960	:	12,800	:	27,800	:	5,170	:	10,400	:	9,820	:	25,800
1961	:	14,400	:	30,200	:	8,230	:	9,220	:	9,620	:	15,400
1962	:	3,590	:	12,000	:	1,570	:	9,800	:	8,130	:	11,900
1963	:	6,990	:	24,500	:	2,770	:	10,500	:	10,200	:	21,500
1964	:	19,200	:	39,400	:	4,370	:	11,000	:	12,400	:	16,600
1965	:	3,280	:	11,500	:	1,440	:	8,540	:	8,060	:	19,300
1966	:	5,930	:	14,900	:	2,950	:	8,360	:	8,0 9 0	:	13,900
1967	:	7,180	:	47,300	:	4,510	:	7,310	:	8,200	:	11,200
1968	:	6,160	:	17,900	:	1,750	:	8,600	:	7,710	:	12,500
1969	:	4,360	:	13,600	:	-	:	8,900	:	8,140	:	16,600
1970	:	5,820	:	17,800	:	-	:	7,490	:	6,980	:	13,400
1971	:	7,840	:	18,600	:	2,920	:	8,380	:	9,440	:	17,800
1972	:	41,000	:	90,000	:	9,600	:	17,800	:	16,500	:	29,600
1973	:	9,200	:	35,900	:	3,370	:	6 ,9 20	:	11,500	:	18,000
1974	:	5,210	:	15,700	:	2,460	:	8,040	:	8,200	:	15,300
1975	:	7,360	:	25,300	:	2,390	:	7,900	:	9,260		18,000
1976	:	8,100	:	28,600	:	3,800	:	9,980	:	10,200		22,400
1977	:	8,020	:	25,100	:	-	:	10,400	:	11,500		17,500
1978	:	6,600	:	24,600	:	-	:	10,300	:	10,400		17,100
1979	:	7,320	:	23,700	:	-	:	9,500	:	11,100		21,700
1980	:	5,540	:	14,900	:	-	:	8,620	:	8,930		24,300
1981	:	5,920	:	22,300	:	-	:	9,500	:	9,200		20,300
1982	:	15,800	:	24,000	:	-	:	10,300	:	10,200		23,200
1983	:	3,220	:	10,700	:	_	:	9,240	:	8,880		13,800
1984	:	9,680	:	38,700	:	-	:	10,500	:	10,700	:	28,200
	:		:		:		:		:		:	

NOTE: Discharges are in cfs.

Table Al2a- Discharge-Frequency Curves

				Discharges in CFS	in CFS			
Expected	••	••	••	••		••	••	
Probability In %	: : Rochester	: : Chili-Henrietta	: : Avon	: Geneseo :	: :Geneseo : Portageville :	: : Filmore :	: : Belfast :	Belvidere
0.2	: 40,200	39,400	:24,400	20,600	90,300	68,700	61,400	47,600
0.5	36,400	35,700	:22,000	18,600	76,000	57,800	51,700	40,100
1.0	33,600	32,900	:20,200	17,200	66,200	20,400	45,000	34,900
2.0	31,000	30,400	:18,600	15,800	57,300	43,600	39,000	30,200
4.0	28,400	27,800	:17,100	14,500	49,100	37,400	33,400	25,900
10.0	24,900	24,400	:14,900	12,900	39,100	29,800	26,600	20,600
20.0	22,200	21,700	:13,600	11,600	31,900	24,300	21,700	16,800
50.0	17,900	17,500	:11,300	9,720	22,300	17,000	15,200	11,800
80.0	14,600	14,300	009,6	8,290	16,000	12,200	10,900	8,400
0.06	13,100	12,800	8,900	7,670	13,600	10,400	9,200	7,200
95.0	12,000	11,800	8,400	7,220	11,900	9,100	8,100	6,300
0.66	10,000	9,800	7,500	6,470	6,500	7,200	6,400	2,000
	•							

Table Al2b - Discharge-Frequency Curves

Discharges in CFS Expected Probability: :Wellsville :Wellsville : : Scio :Stannards: Shongo In % : Belmont :(Reach A) :(Reach B) 47,100 36,400 : 30,700 0.2 65,800 : 50,200 : 35,700 : 24,200 28,200 0.5 51,900 : 39,500 : 37,100 28,600 1.0 42,900 32,700: 30,700 23,700 23,300 : 20,000 2.0 35,200 26,800: 25,200 19,500 19,100 : 16,400 21,700: 4.0 28,500 20,400 15,700 15,500 : 13,300 16,000: 10.0 21,000 15,000 11,600 11,400 9,800 20.0 12,200: 8,900 8,700 16,100 11,500 7,500 4,700 50.0 10,100 7,700: 7,200 5,600 5,500 80.0 6,700 5,100: 4,800 3,700 3,600 3,100 90.0 5,500 4,200: 3,900 3,020 2,970 2,500 95.0 2,200 4,700 3,600: 3,300 2,580 2,540

2,500

1,960

1,930

1,700

99.0

3,600

2,7 ::

Table Al3a - Discharge-Frequency Curves (Improved Conditions)

				Discharg	ges in CFS		
Expected Probability In %	:	Belmont	: : : Scio	: :Wellsville :(Reach A)	: :Wellsville :(Reach B)	: : : : : : : : : : : : : : : : : : :	Shongo
0.2	:	7,000	: : 5,300	: : 5,000	3,900	: 3,800 :	30,700
0.5	:	5,900	: 4,500	: 4,200	: 3,300	3,200	24,200
1.0	:	5,600	: 4,300	4,000	3,100	3,000	20,000
2.0	:	5,600	: 4,300	: 4,000	: 3,100	3,000	16,400
4.0	:	5,600	: 4,300	: 4,000	: 3,100	3,000	13,000
10.0	:	5,600	: 4,300	4,000	: : 3,100	3,000	9,800
20.0	:	5,600	: 4,300	: 4,000	: 3,100	3,000	7,500
50.0	:	5,600	: 4,300	: 4,000	: 3,100	: 3,000	4,700
80.0	:	5,600	: 4,300	: 4,000	: 3,100	3,000	3,100
90.0	:	5,500	: 4,200	: : 3,900	: 3,020	3,000	2,500
95.0	:	4,700	: : 3,600	: : 3,300	: 2,580	: 2,540 :	2,200
99. 0	:	3,600	: 2,705	: : 2,500	: 1,960	: 1,930 :	1,700

Table Al3b- Discharge-Frequency Curves (Improved Conditions)

Discharges in CFS

Expected	:		ges in crs		:
Probability In %	:	Portageville	: : Filmore :	: Belfast	: : Belvidere
111 /6	:	rortageville .	· FIIMOTE	Dellast	· belvidere
0.2	:	62,000	47,100	42,200	32,700
0.5	:	49,000	37,200	33,320	25,800
1.0	:	41,000	31,100	27,880	: : 21,600
2.0	:	33,000	25,100	22,400	: : 17,400
4.0	:	26,500	20,100	18,000	: : 14,000
10.0	:	19,000	14,400	: 12 ,9 00	: : 10,000
20.0	:	15,000	11,400	: : 10,200	: : 7,900
50.0	:	10,000	7,600	: 6,800	: : 5,270
80.0	:	10,000	7,600	6,800	: 5,270
90.0	:	10,000	7,600	6,800	: : 5,270
95.0	:	10,000	7,600	: 6,800	: : 5,270
99.0	:	9,500	7,200	6,400	: : 5,000

Table Al4 Pertinent Data for Considered Modifications to Mount Morris Dam

Spillway: 760 ft NGVD Height: 215 ft

Top of Dam: 790 ft NGVD

EXISTING FLOOD CONTROL STORAGE: 302000 acre - ft (5.24 inches of runoff)

STORAGE WITH 15 ft GATES: 346000 acre - ft

INCREASE STORAGE: 44000 acre - ft - .76 inches of runoff

STORAGE WITH 30 ft GATES: 406000 acre - ft

INCREASE STORAGE: 104000 acre ft - 1.81 inches of runoff
Drainage Area at Mount Morris 1080 Sq. Mi.
Drainage Area at Index point 1 2467 Sq. Mi.
Drainage Area at Index point 3 1978 Sq. Mi.
Drainage Area at Index point 4 1424 Sq. Mi.

In order to provide a range of economic values, two general "with project" conditions were analyzed: addition of 15-ft gates atop the Mount Morris Dam spillway, and addition of 30-ft gates. Frequency curves were developed for each reach for each case for use in determining damages under "with project" conditions, and applied as described in Section A8.

The method used to determine the with-project condition curves proportioned the increased storage at the reservoir to the index points by drainage area. For example, the .76 inches of increased storage attributable to the addition of the 15 ft gates would be the equivalent of .58 inches of storage at index point #4 (.76 X 1,080 divided by 1424). The equivalent additional storage at each index point was used to determine the discharge-frequency curves under with-project conditions. The rainfall-frequency for a 96 hour storm duration was used to determine the associated rainfall for a selected exceedence frequency. The equivalent storage was subtracted from this value and the exceedence frequency for the modified rainfall then read from the rainfall-frequency curve. The modified exceedence frequency was used to determine the discharge from the discharge-frequency curve. This discharge was then plotted at the original exceedence frequency. This was then repeated for a number of exceedence frequencies.

Example: The 100-year rainfall amount (96 hours) is 4.80 inches. The equivalent additional storage (for the 15 ft gates) at Index Point 4 is .58 inches (4.80 - .58 = 4.22 inches). For a rainfall of 4.22 inches, the exceedence frequency is 3% (33 years). From the discharge frequency curve, the 3% exceedence frequency has a discharge of 15,000 CFS, which is plotted at the 1% exceedence frequency for with-project conditions.

A8 NON-AGRICULTURAL FLOOD DAMAGES

Stage-discharge (rating) curves were developed using July 1987 survey data at each of the index points and normal depth calculations for all reaches except 2, 4, and 13. For those reaches, the rating curves were taken from the August 1973 "Genesee River Basin - Report of Flood, Tropical Storm Agnes, 21-23 June 1972." These curves are shown in Figures All through A28.

Expected annual non-agricultural flood damages were calculated for each of the reaches described in Table A2. The stage-damage curves, presented in Figures A29 through A46, were taken from the above mentioned Report of Flood, and updated to October 1987 by price level. The curves for the Wellsville reaches were adjusted to account for levee work performed since the original curves were developed.

Discharge-frequency curves for the Upper Genesee Basin were taken from the Genesee River Basin Reconnaissance Report. The discharge-frequency curves for the Lower Genesee River Basin were developed as described in Section A7.

The combination of these three types of curves, through integration within the Hydrologic Engineering Center's Expected Annual Damage (EAD) Program, yields expected annual flood damages for each reach of the Genesee River. The results of the expected annual flood damage computations for each reach can be found in Table Al5. The 'with-project' expected annual flood damages were computed using the improved conditions discharge-frequency curves presented in Section A6. The results of the damage computations as well as the resulting benefits of the plans are presented in Table Al6. These numbers will differ slightly from those presented in the Economic Appendix because of the effects of affluence. Also included in the Economic Appendix is the analysis of agricultural flood damages.

A9 NON-OVERFLOW SECTION

The increased height of the non-overflow section of the Mt. Morris dam had to be determined, based on the reduced weir length caused by the tainter gates on the spillway. The PMF analysis resulted in a discharge of 442,000 CFS. Using the weir equation with a coefficient of 3.9 and a weir length of 420 feet, the required height is 41.8 feet. Adding 1.7 feet for runup above the freeboard reference level, the height of the non-overflow section is 43.8 feet, resulting in an elevation of 803.5 feet USCAGS. The pre-nt non-overflow section is at 790.0 feet, resulting in a required increase of approximately 13 feet.

Table Al5 - Damage Summary of the Genesee River Basin
(Non-Agricultural)

	: י זאודרו	AL DAMAGE	•	:	AVERAGE			
KEACH	: STAGE (ft.)			(Yr):	ANNUAL DAMAGES			
Lower Basin	: (2)	:	TO THE STATE OF TH	:				
ROCHESTER 1	; ; ;		erage annual					
CHILI-HENRIETTA	: : 516.0	:	2	:	\$481,000			
AVON 3	: : 529.0	:	2	:	401,000			
GERESEO 4	: : 560.0	:	20	:	122,000			
Upper Basin	:	: :	es the area	:				
MT. MORRIS	: Inis read :Letchworth Sta :contains Hount	te Park	and is main					
Portageville	: : 1102.0 :		2	: :	13,000			
Fillmore 7	: : 1165.0	65.0 : 2			: : 50,000			
Belfast 8	: : : 1261.0	:	2	:	87,000			



DODGESS CARAGOOD BULLINGS PRESSED OF THE

Table Al5 - Damage Summary of the Genesee River Basin (Cont'd)
____(Non-Agricultural)____

REACH	: INITIAL	AVERAGE			
KEACH	: STAGE (ft.)	: APPROX. FREQ. (Yr):	ANNUAL DAMAGES		
BELVIDERE 9	: 1320.0	· : 2 :	9,000		
BELMONT 10	: : 1366.0	: : 2 : :	44,000		
SCIO II	: : 1446.5 :	: : 2 : : :	83,000		
WELLSVILLE	: : <u>:</u>	: For Wellsville damage : below :	e summary, see		
STANNARDS 13	: : 1511.7 :	: : 2 : : :	32,000		
SHONGO 14	: : 1529.5	: : 2 : :	14,000		
WELLSVILLE AREA	: : (2)	: : :			
GENESEE RIVER	:	:			
12, G-1	: 1479.5	: : 5 :	33,000		
12, G-2	: 1495.0	<u> </u>	0		
12, G-3	1493.0	100	24,000		
12, G-4	1496.0	100	34,000		
12, G-5A	: : 1495.0	: : 50	14,000		
12, G-5B	: : : 1495.3	: : : 100	2,000		
12, G-6	: : : 1497.5	: : : 50	44,000		

GRAND TOTAL \$1,487,000

⁽¹⁾ Average Annual Damages shown are at October 1987 price level.

⁽²⁾ Stages are referenced to National Geodetic Vertical Datum of 1929.

TABLE Al6 - Damage Summary of the Genesee River Basin (Non-agricultural)

Improved Conditions - Plan 10A - 15 FOOT GATE SCENARIO

REACH	:		AVERAGE A	ANNUAL	DAMAGES (\$)	
	:		Existing :		Improved	
	- :			<u>- , </u>		
ROCHESTER	:		: NEGLI	[GIBLE:		
1	:		:	}		
	<u>:</u> _					
CHILI-	:		•			
HENIETTA	:					
2	:	481,000	:	;	304,000	
	-			<u> </u>		
AVON	:		•	'		
3	:	401,000	:	:	239,000	
	_ <u>:</u> _					
GENESEO	:		•			
4	:	122,000		·	12,000	
TOTAL	:	1,004,000	:		555,000	
	Impi	roved Conditio	ons - Plan 10A	- 30	FOOT GATE SCENARIO	
22.4						
REACH	:		AVERAGE A Existing:		DAMAGES (\$) Improved	
	:		intoering :	' 		
	:					
ROCHESTER 1	:		NEGLI	GIBLE		
	:		•	<u>'</u>		
	:		:			
CHILI-	:		:			
HENIETTA 2	:	481,000	•		127,000	
-	:	101,000			127,000	
	:		:			
AVON 3	:	401,000			113,000	
J	:	401,000	•	<u>'</u>	113,000	
	:		:			
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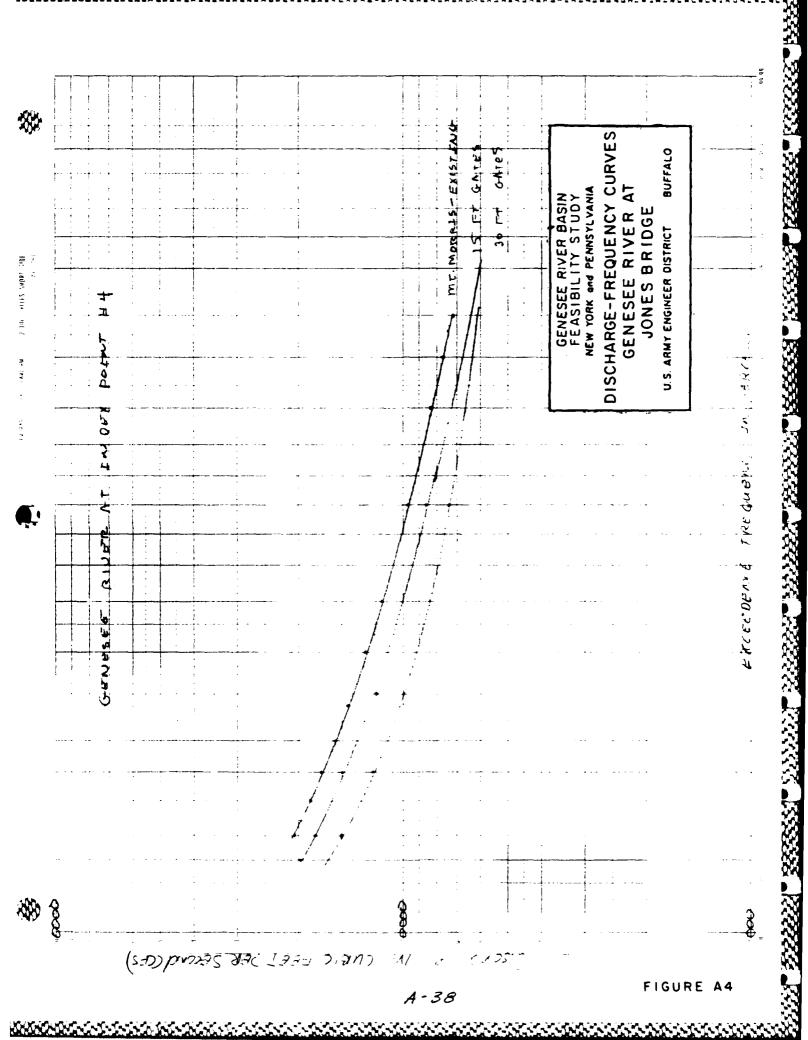
A-35

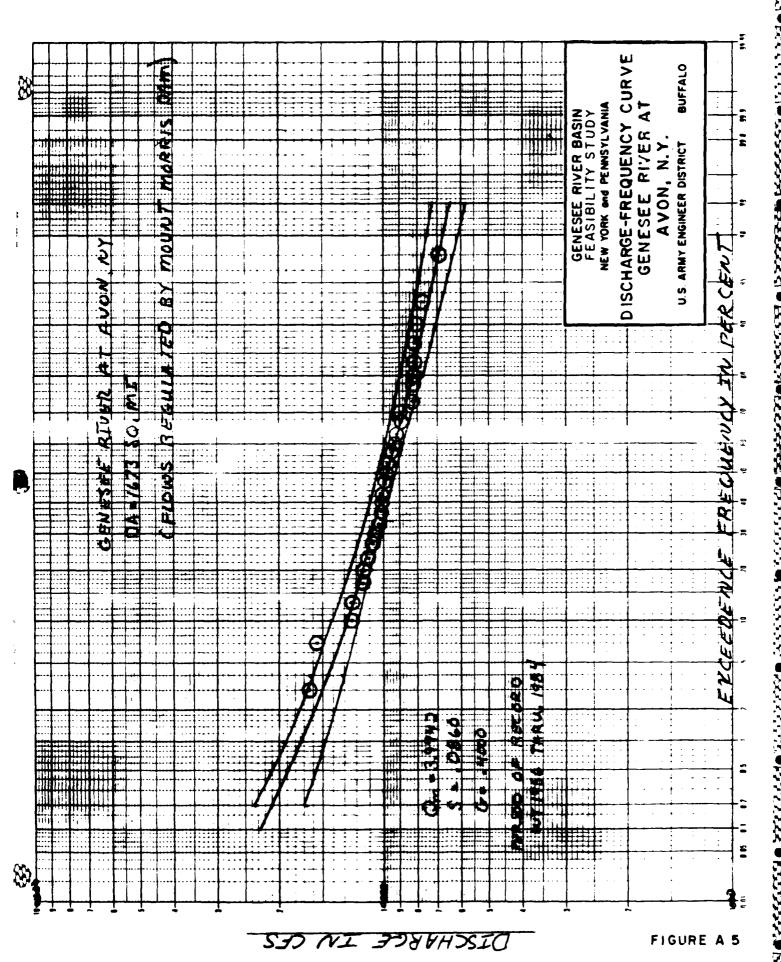
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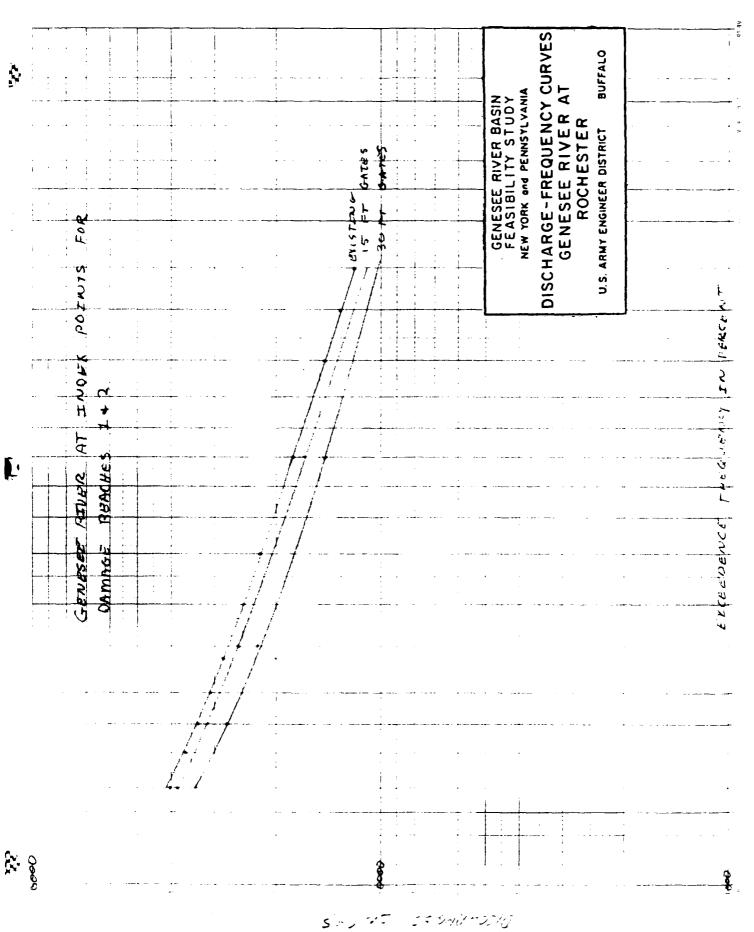
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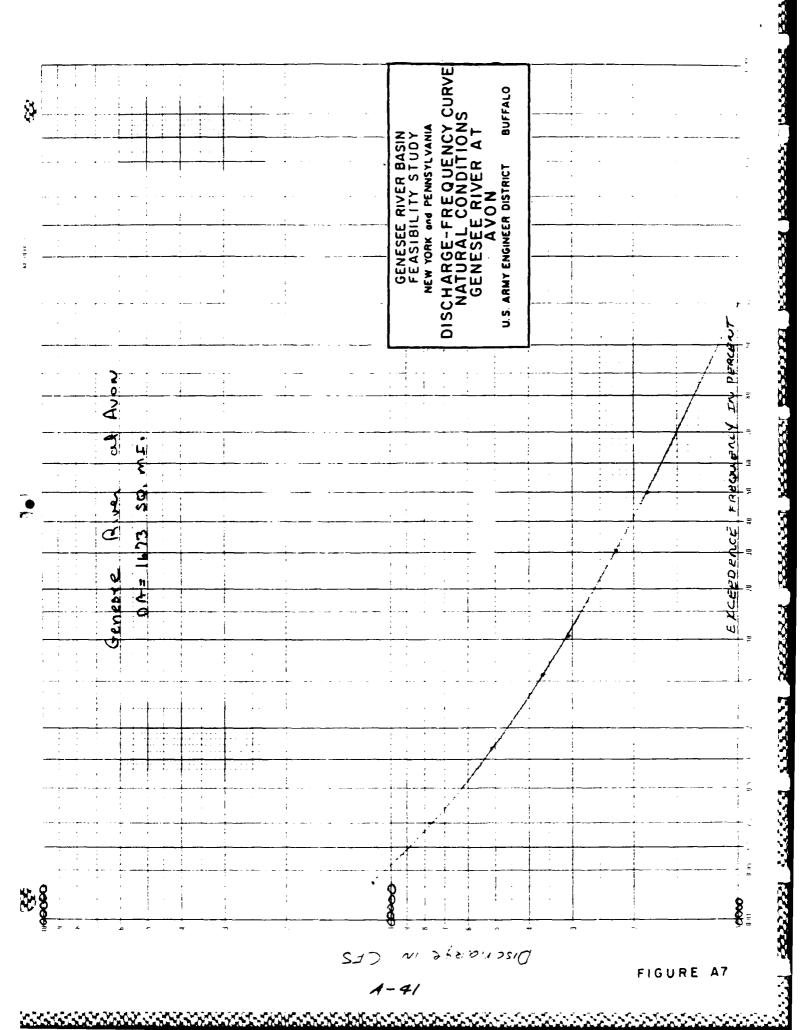






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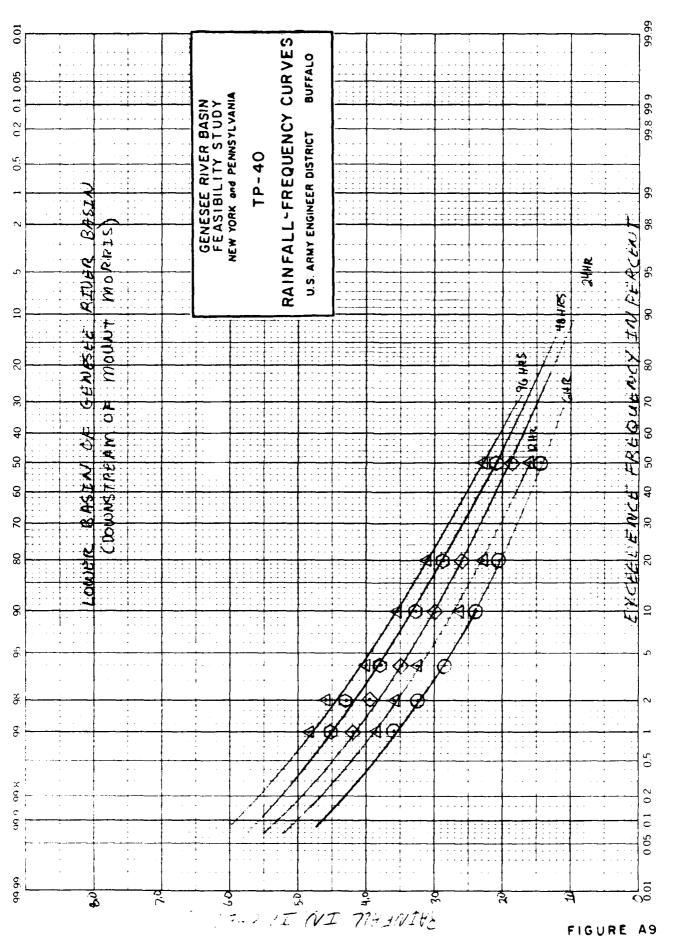
FIGURE A6



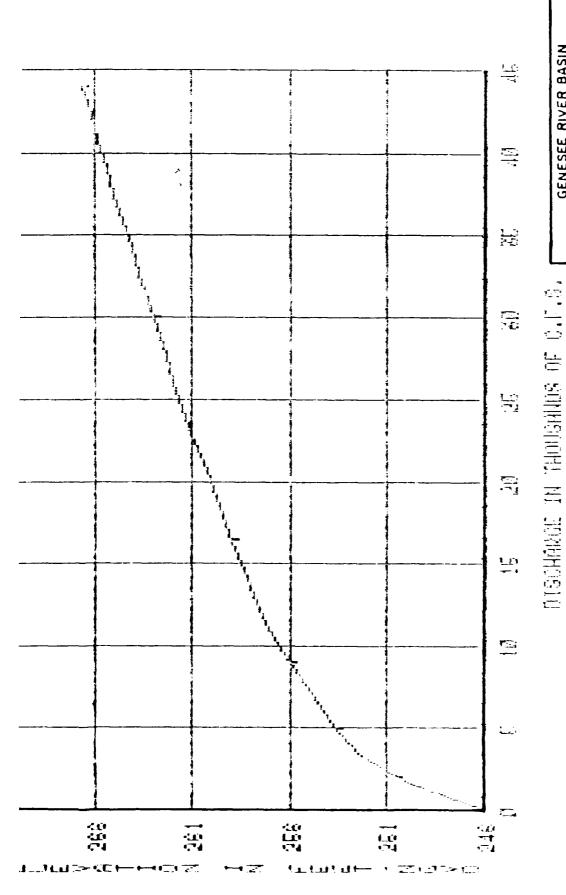
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FIGURE A8

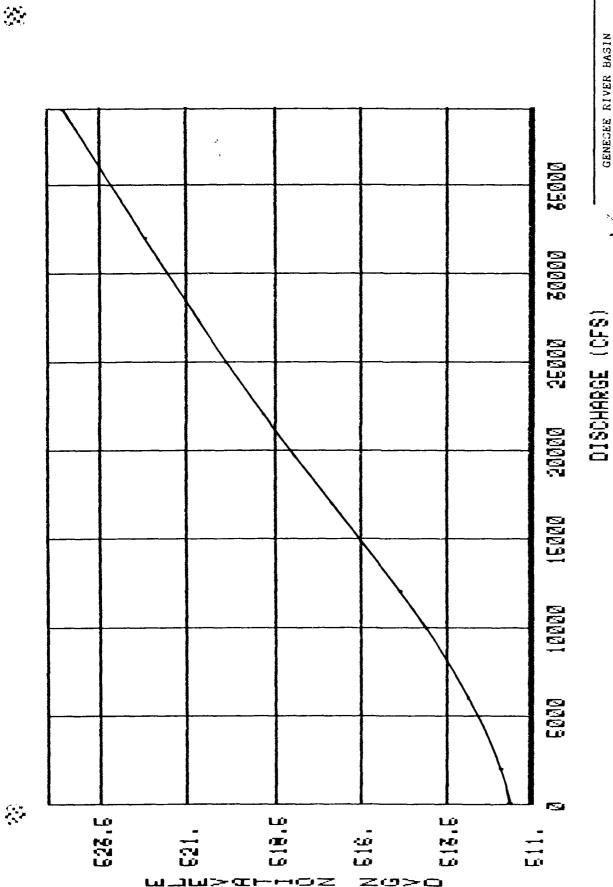


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GENESEE RIVER BASIN FEASIBILITY STUDY NEW YORK ON PENNSYLVANIA RATING CURVE

BUFFALO REACH I-ROCHESTER U.S. ARMY ENGINEER DISTRICT



GENESEE RIVER BASIN FEASIBILITY STUDY New York and Pennsylvania

W York and Pennsylvar RATING CURVE

REACH 2 U.S. Army Engineer District Buffalo

NOTE:

INDEX POINT AT RIVER MILE 14.1, CHILI, NY.

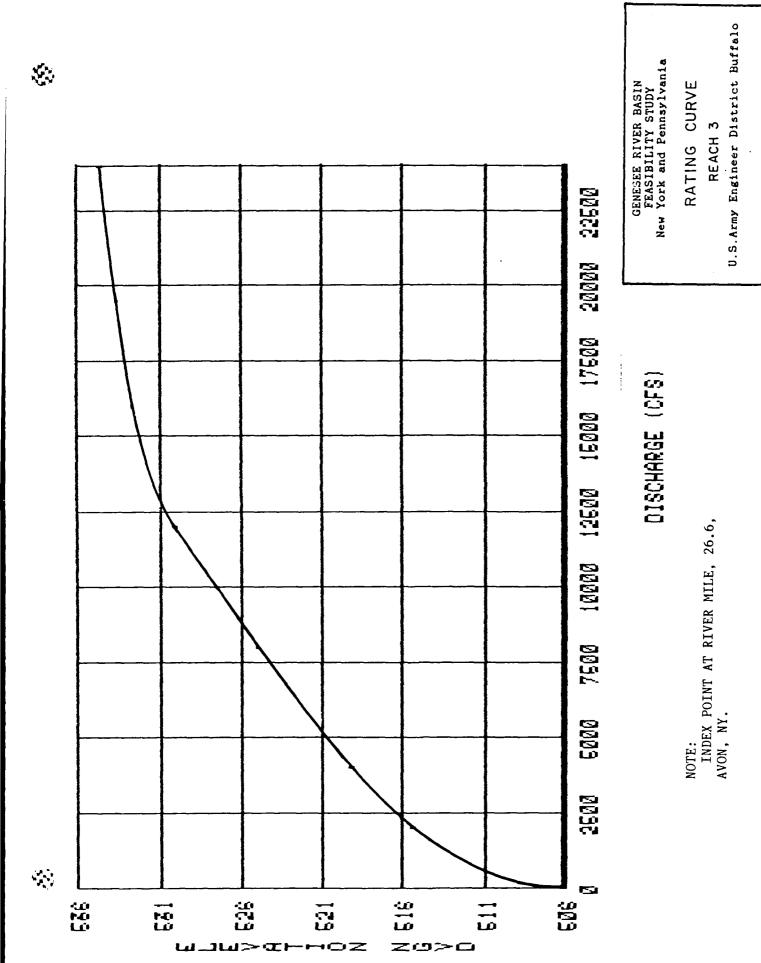
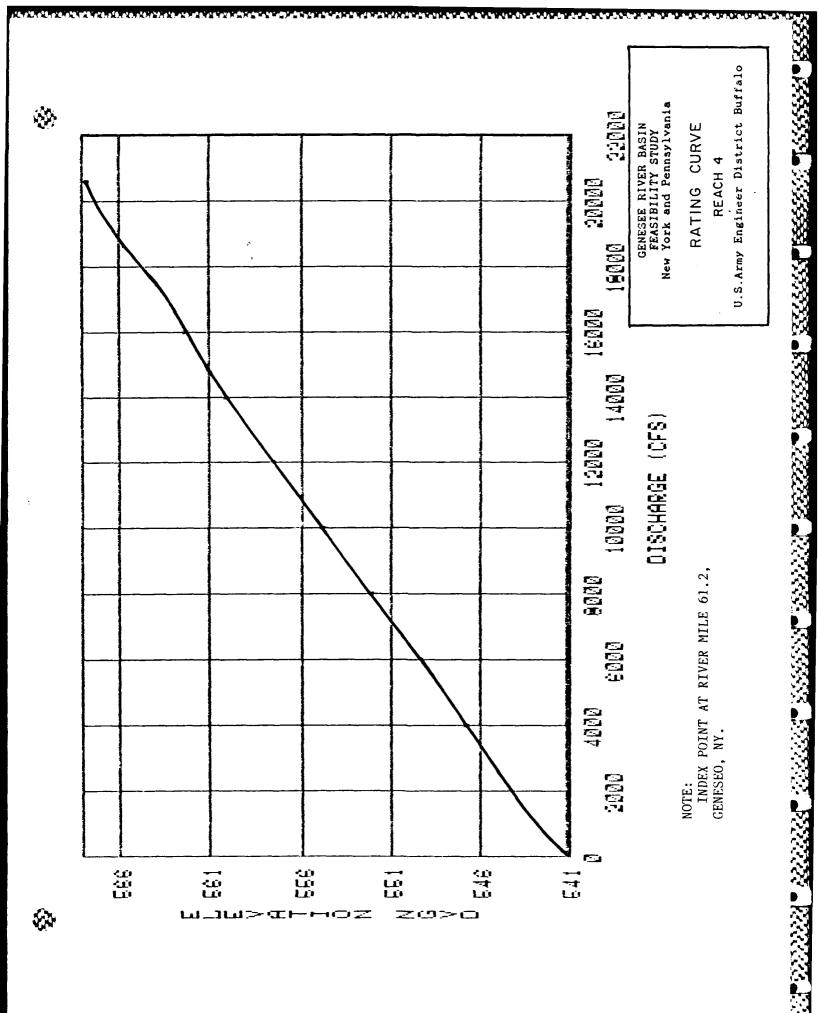
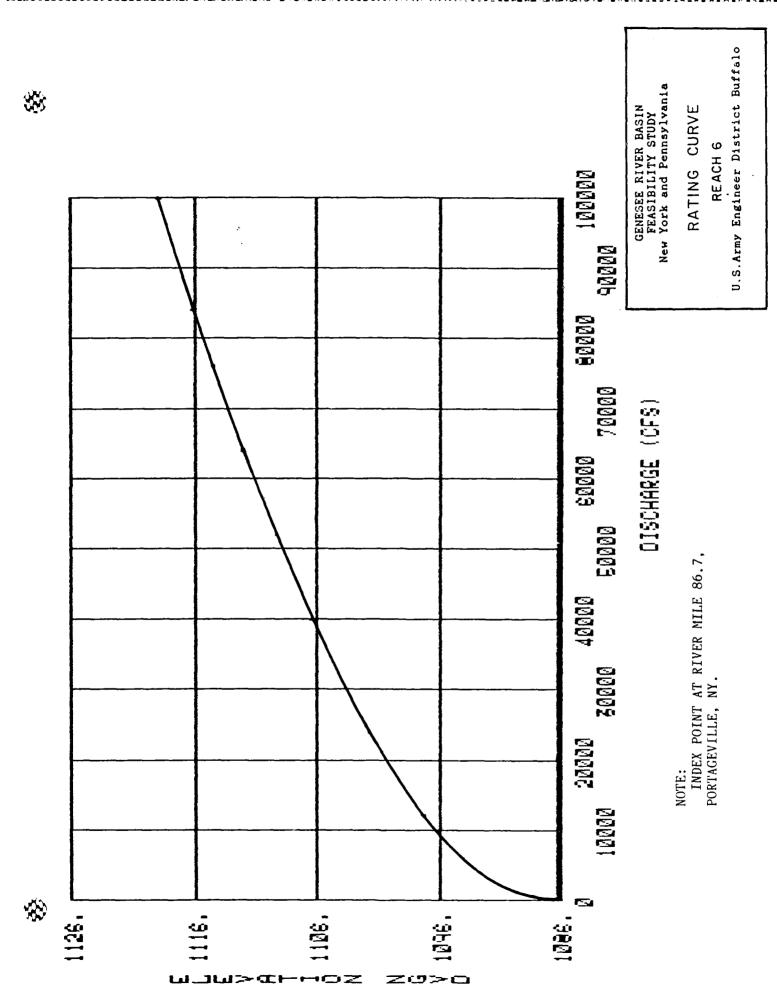
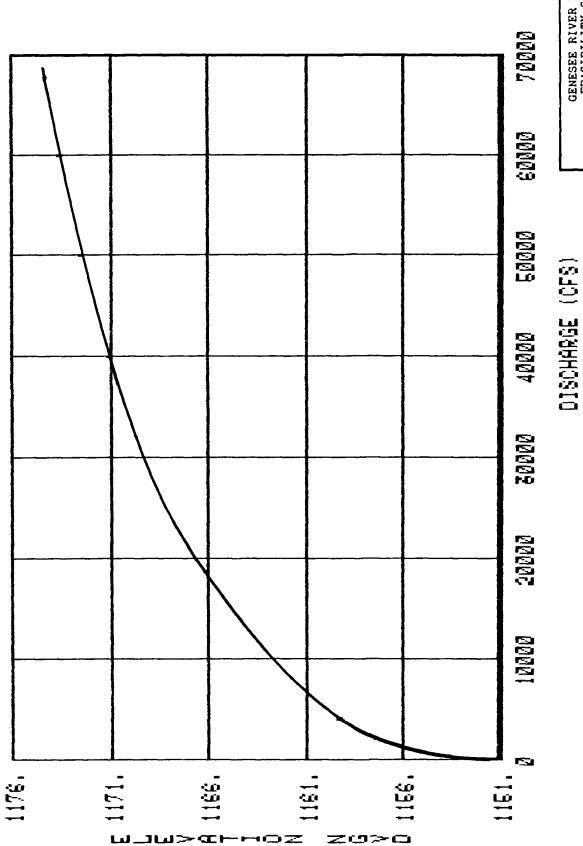


FIGURE A13









GENESEE RIVER BASIN FEASIBILITY STUDY New York and Pennsylvania

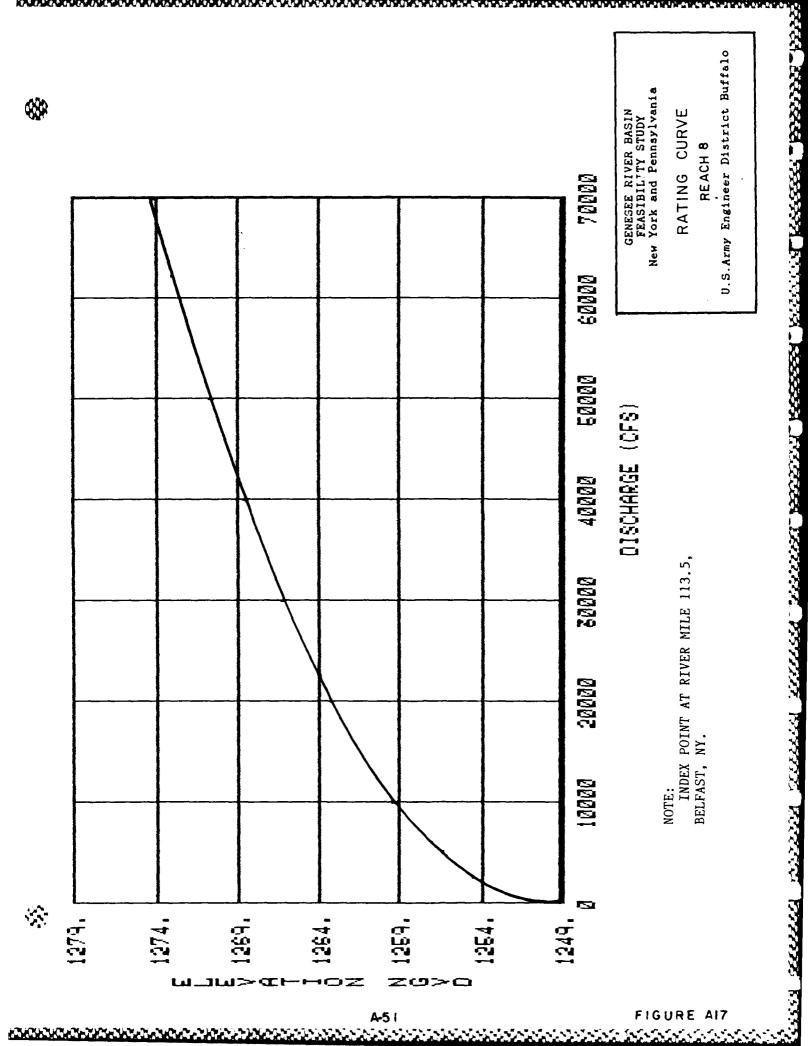
RATING CURVE

REACH 7

U.S. Army Engineer District Buffalo

INDEX POINT AT RIVER MILE 99.9, FILLMORE, NY.

NOTE:



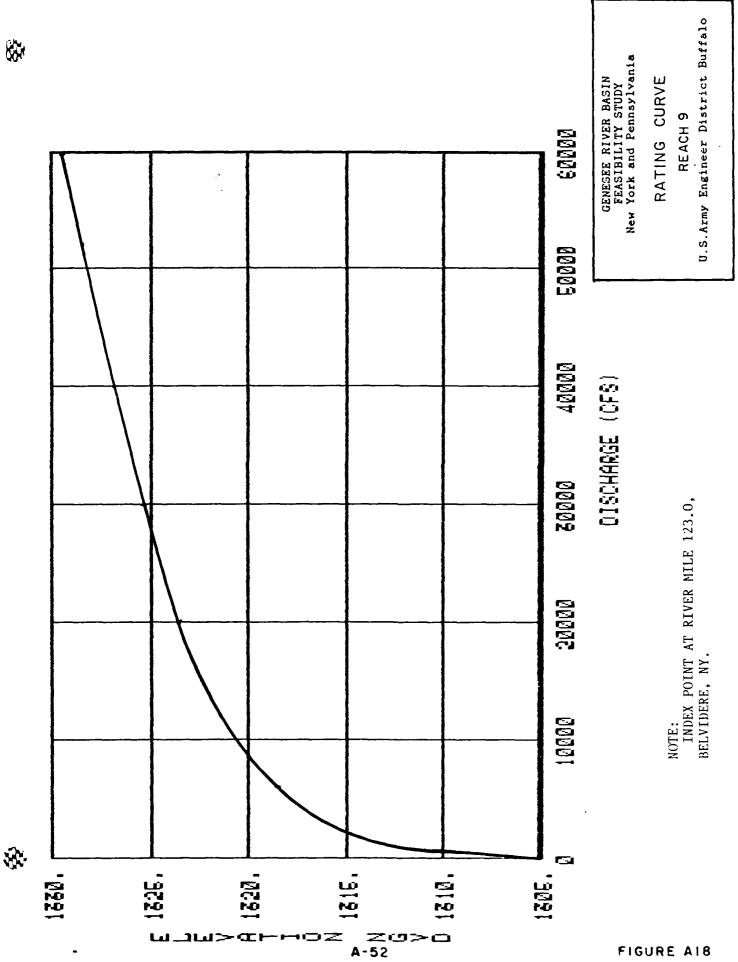
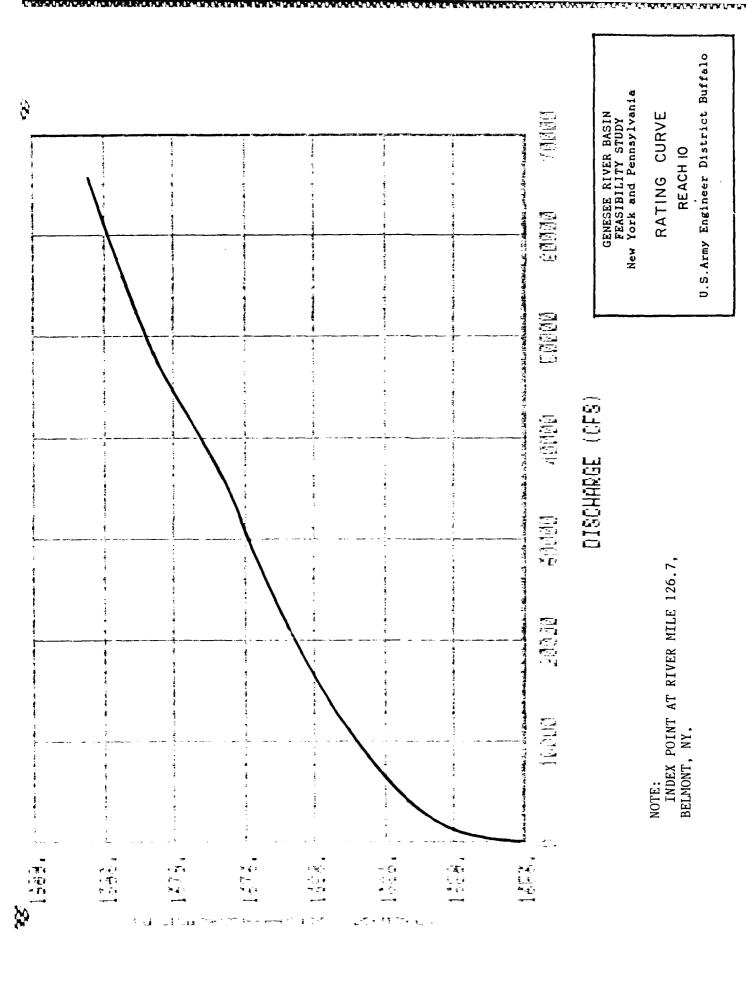
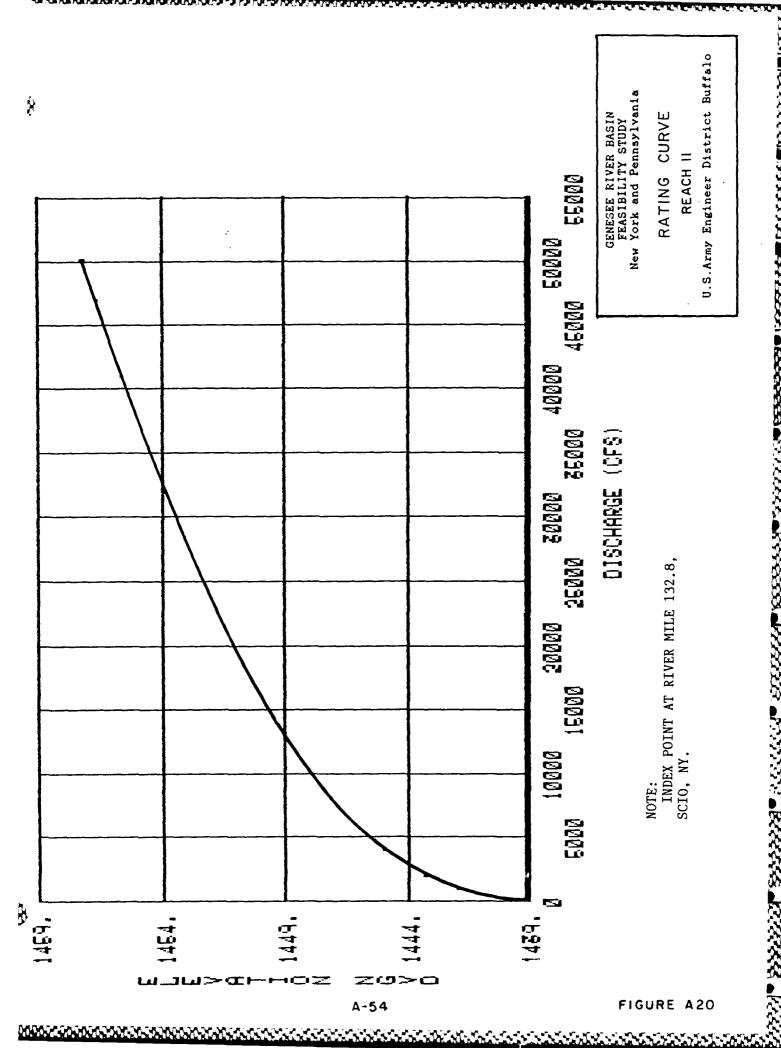
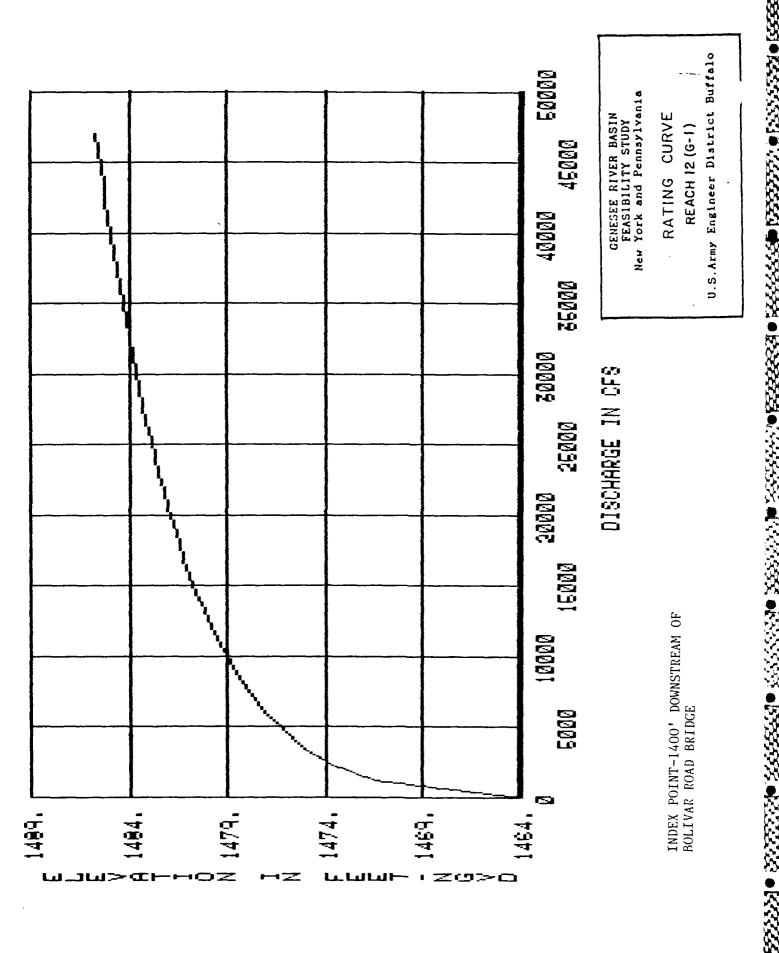
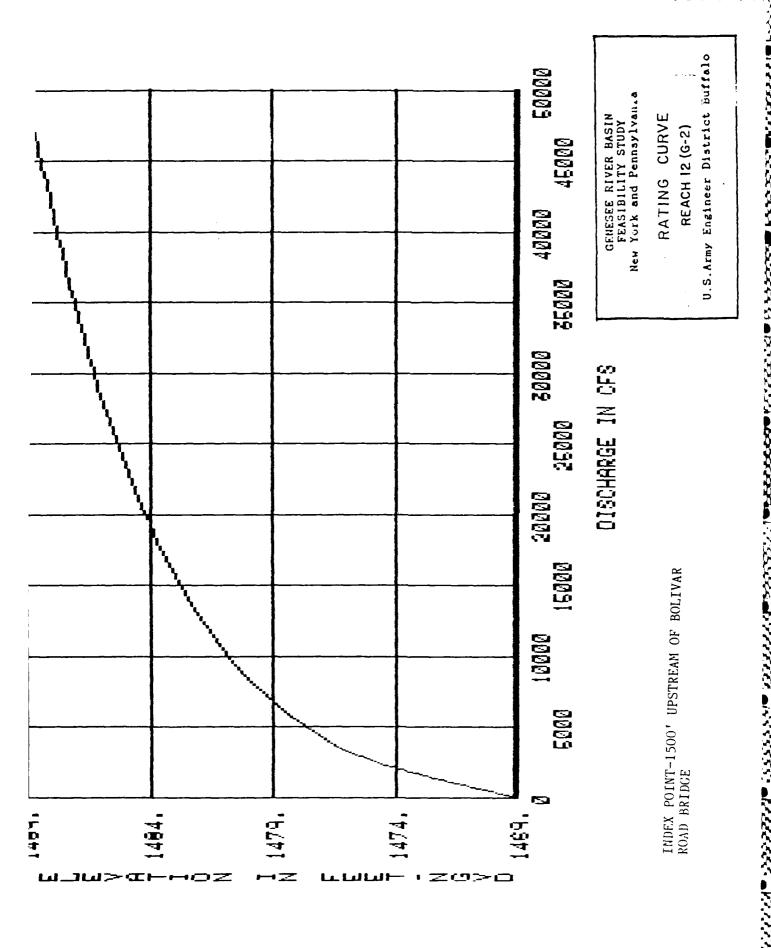


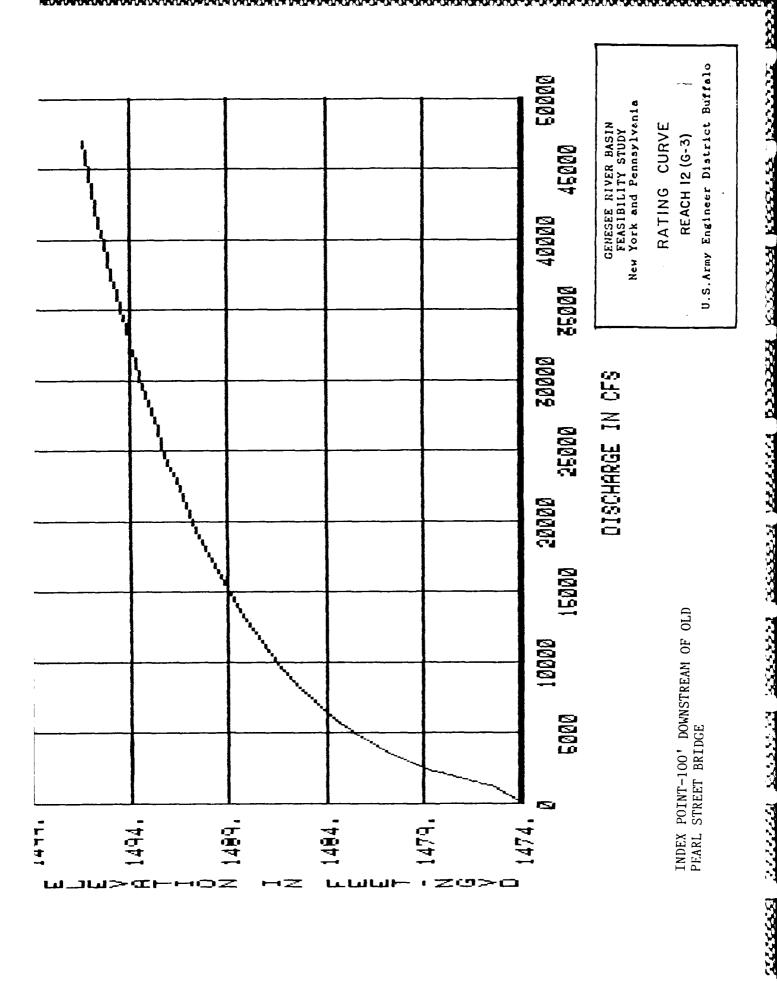
FIGURE A18

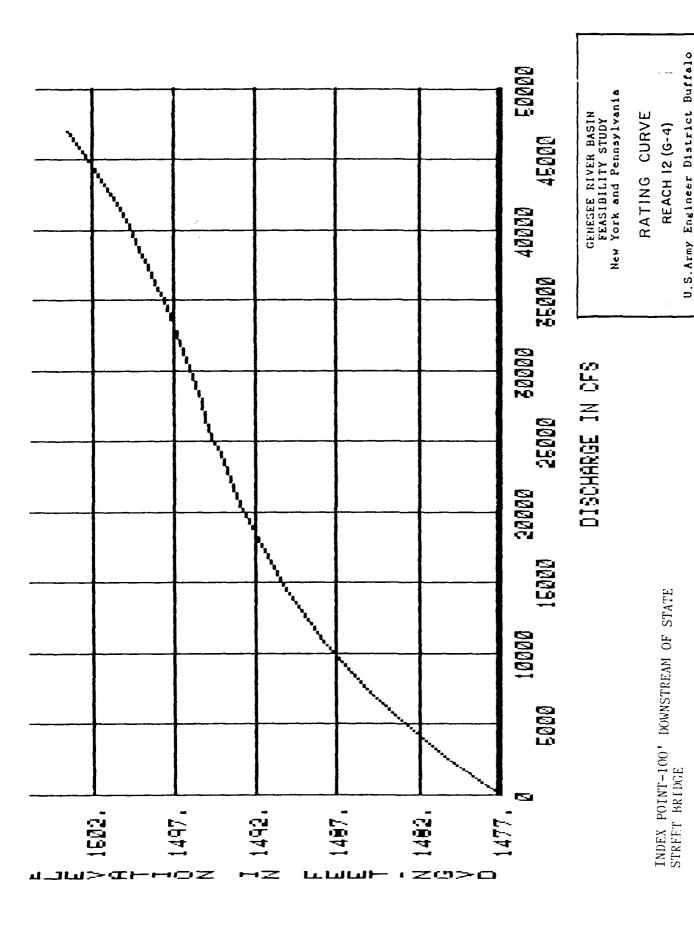


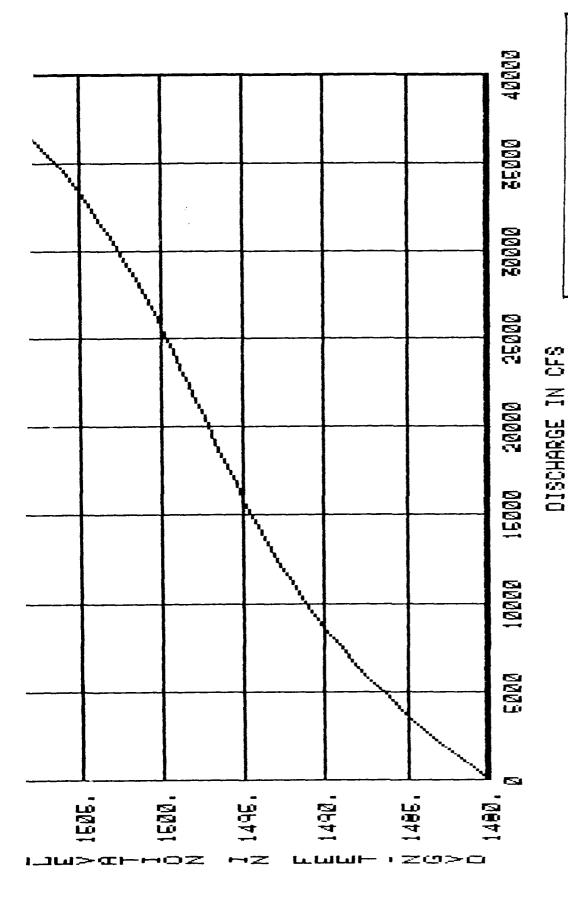








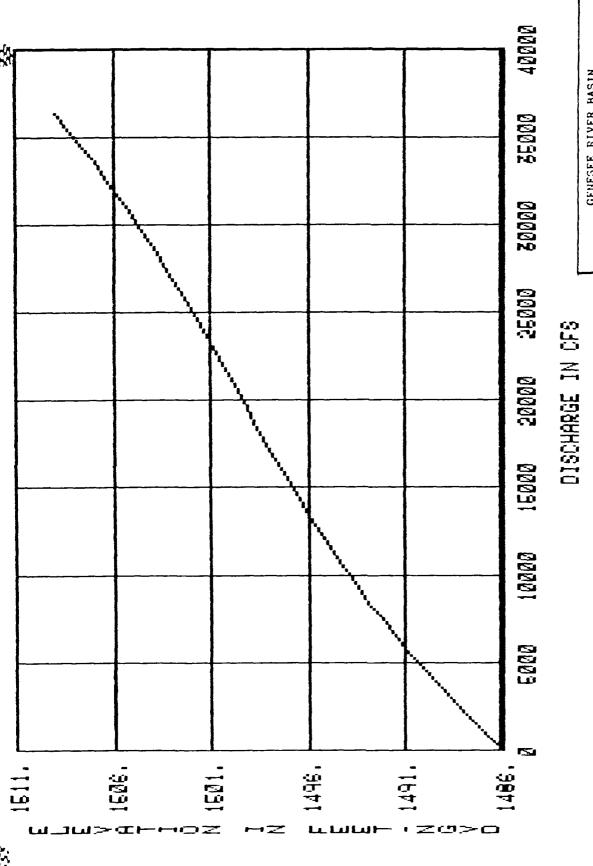




GENESEE RIVER BASIN FEASIBILITY STUDY New York and Pennsylvania

REACH 12 (6-50) & 12 (6-5b) U.S. Army Engineer District Buffalo RATING CURVE

INDEX POINT-1100' UPSTREAM OF STATE STREET BRIDGE

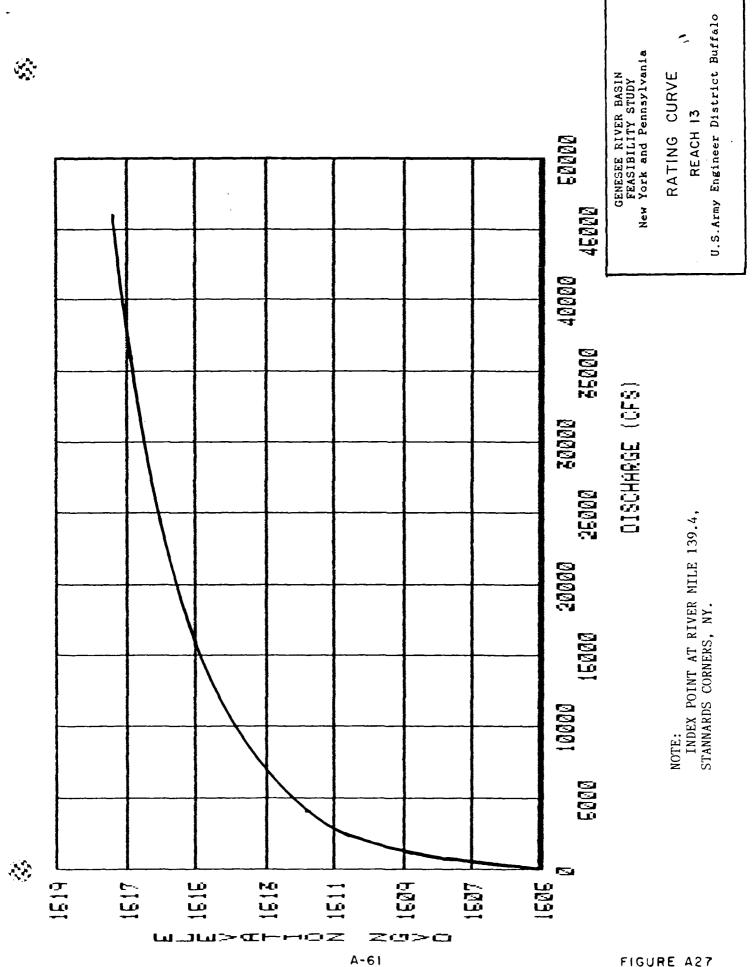


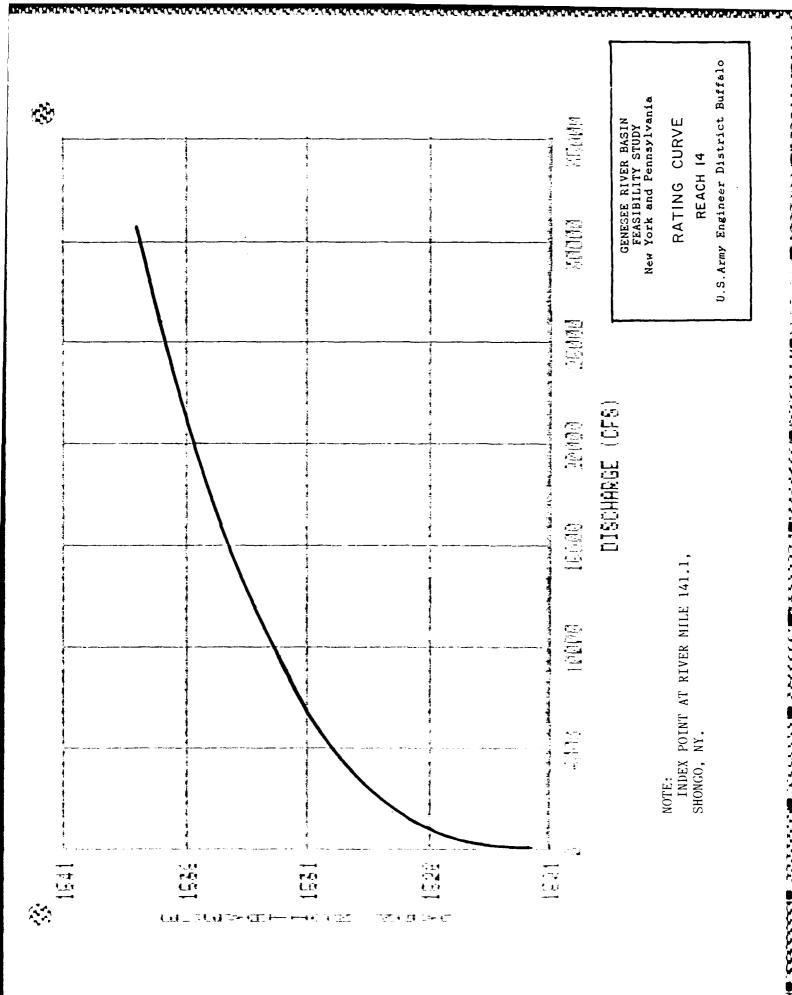
GENESEE RIVER BASIN FEASIBILITY STUDY New York and Pennsylvania RATING CURVE

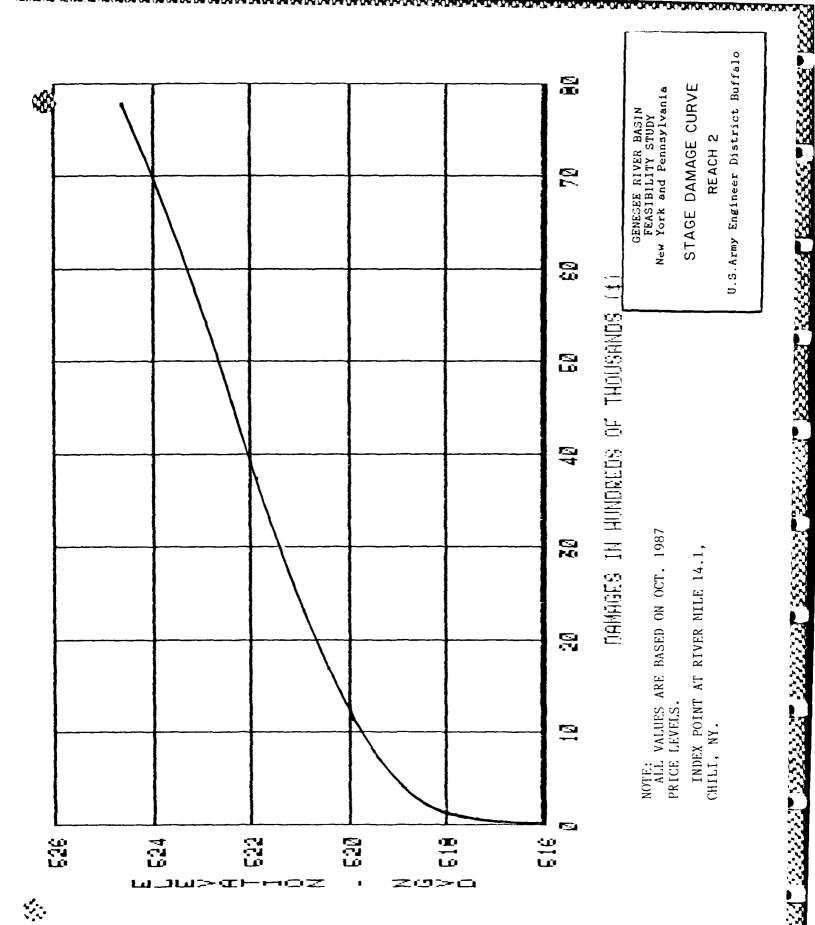
REACH 12 (6-6)
U.S. Army Engineer District Buffalo

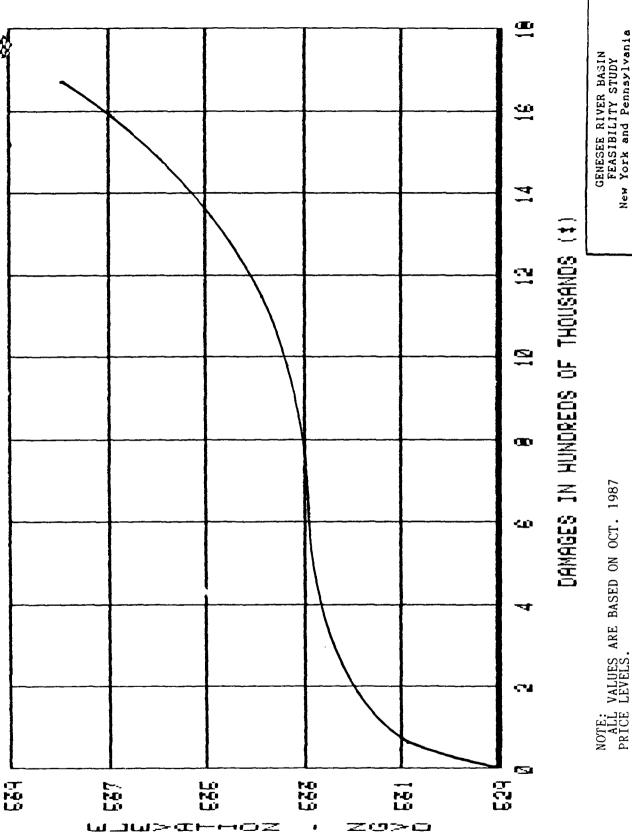
A-60

INDEX POINT-1200' UPSTREAM OF OLD MUNICIPAL DAM









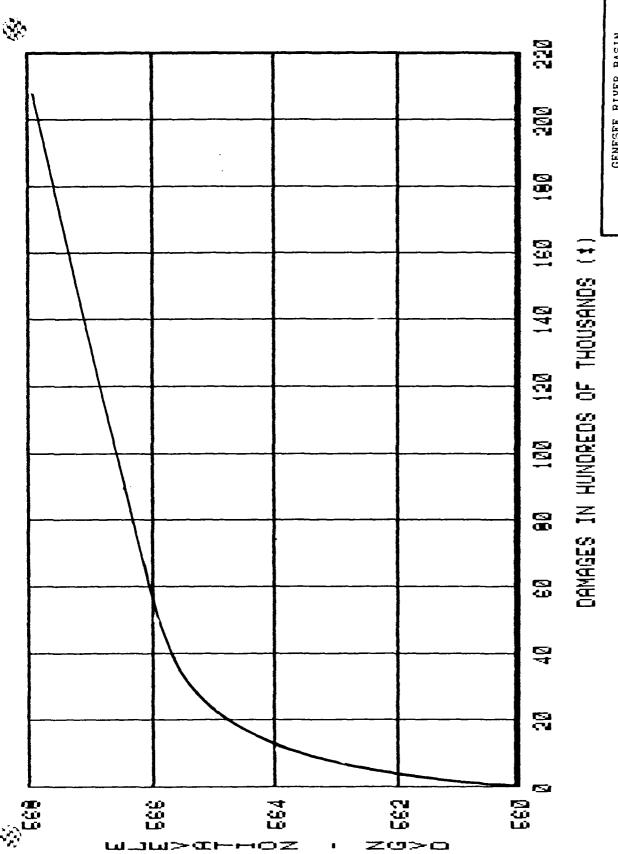
STAGE DAMAGE CURVE

REACH 3

U.S.Army Engineer District Buffalo

INDEX POINT AT RIVER MILE 26.6

AVON, NY.



STAGE DAMAGE CURVE

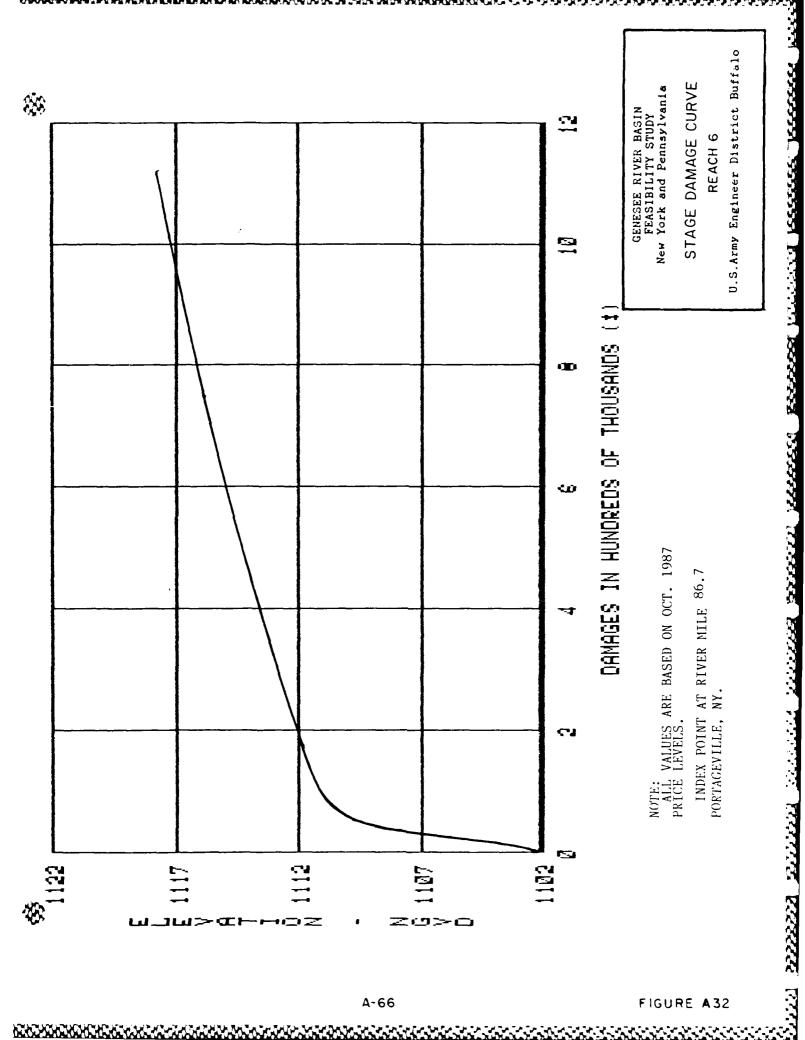
REACH 4

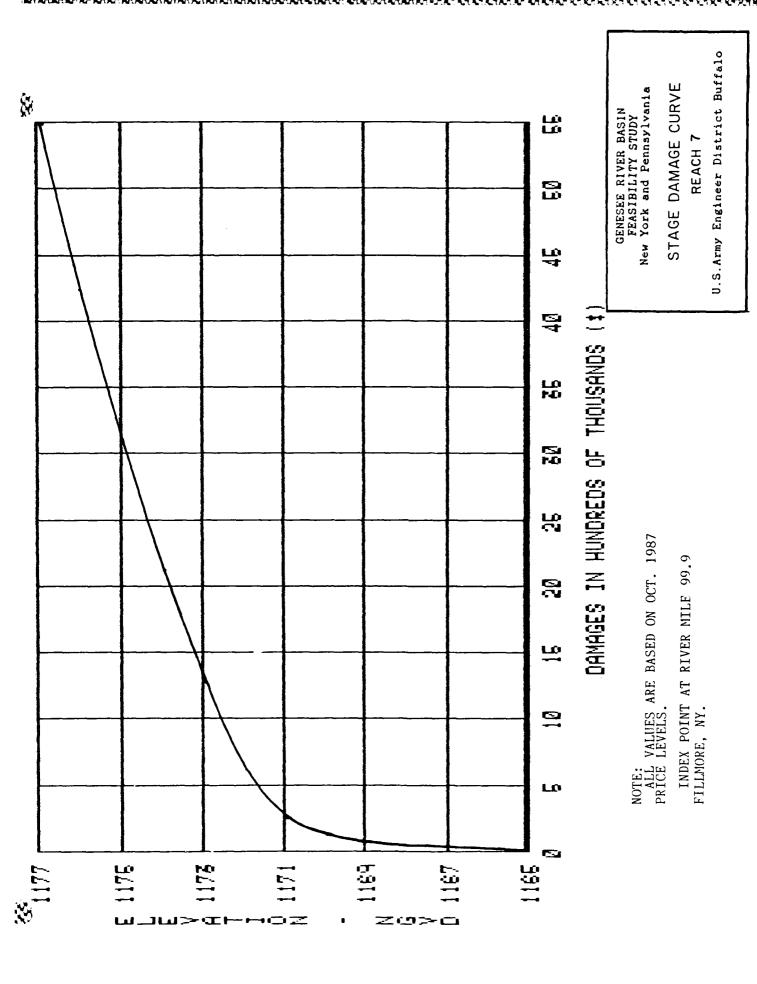
U.S. Army Engineer District Buffalo

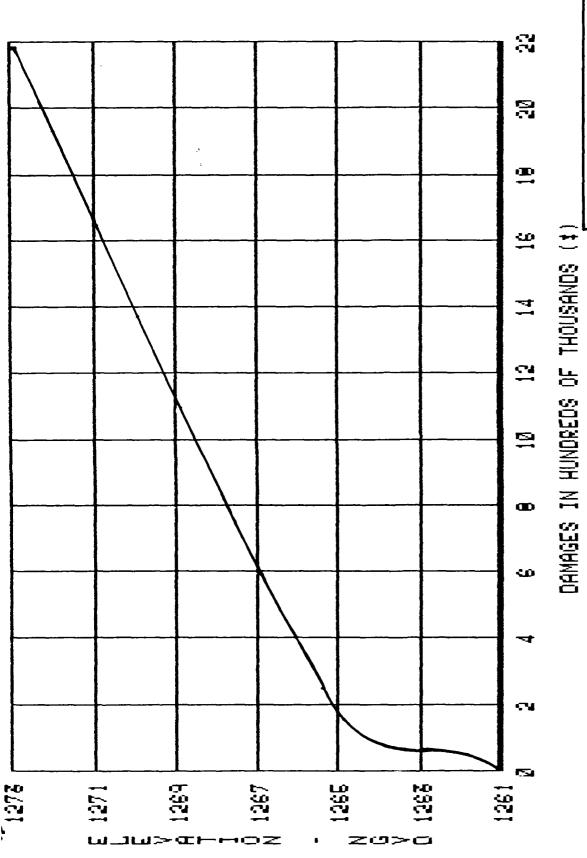
NOTE: ALL VALUES ARE BASED ON OCT. 1987 PRICE LEVELS.

INDEX POINT AT RIVER MILE 61.2

GENESEO, NY.





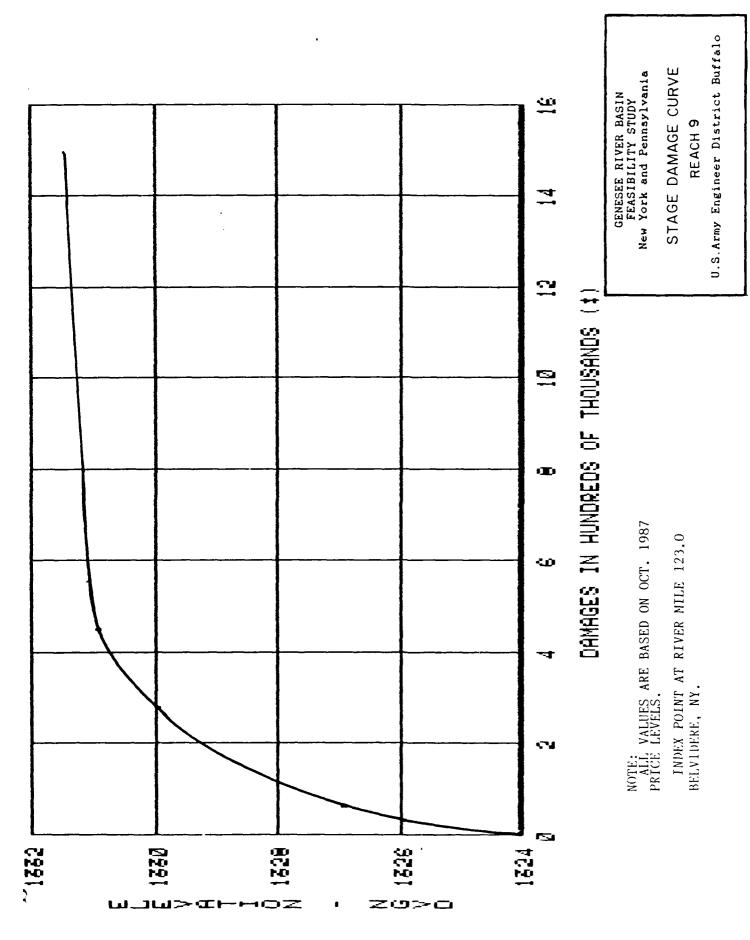


STAGE DAMAGE CURVE REACH 8

U.S. Army Engineer District Buffalo

NOTE: ALL VALUES ARE BASED ON OCT. 1987 PRICE LEVELS.

INDEX POINT AT RIVER MILE 113.5 BELFAST, NY.



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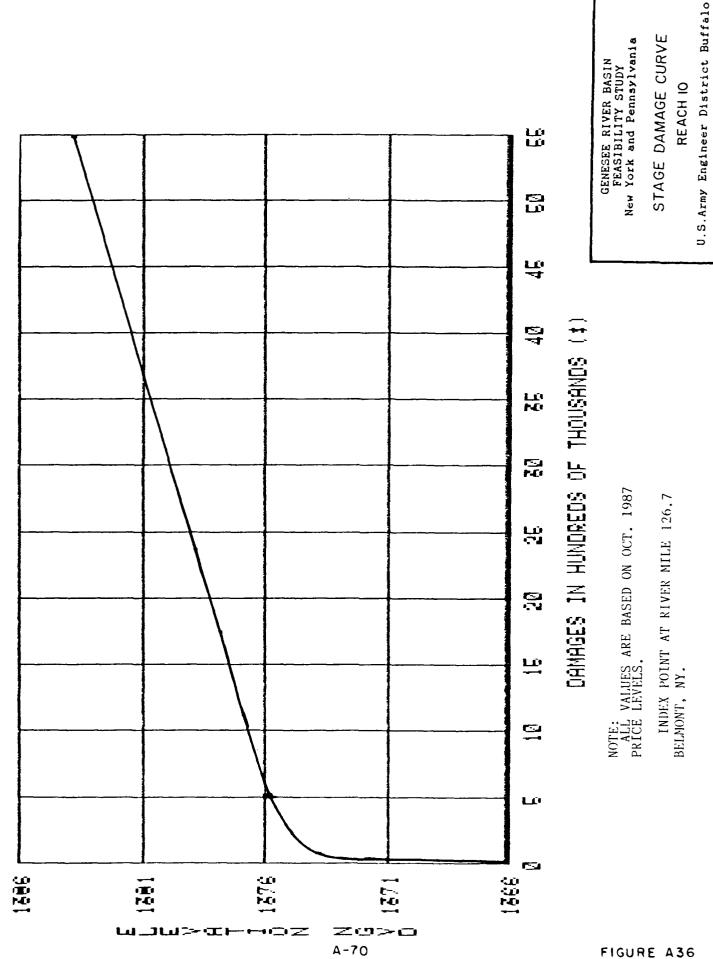
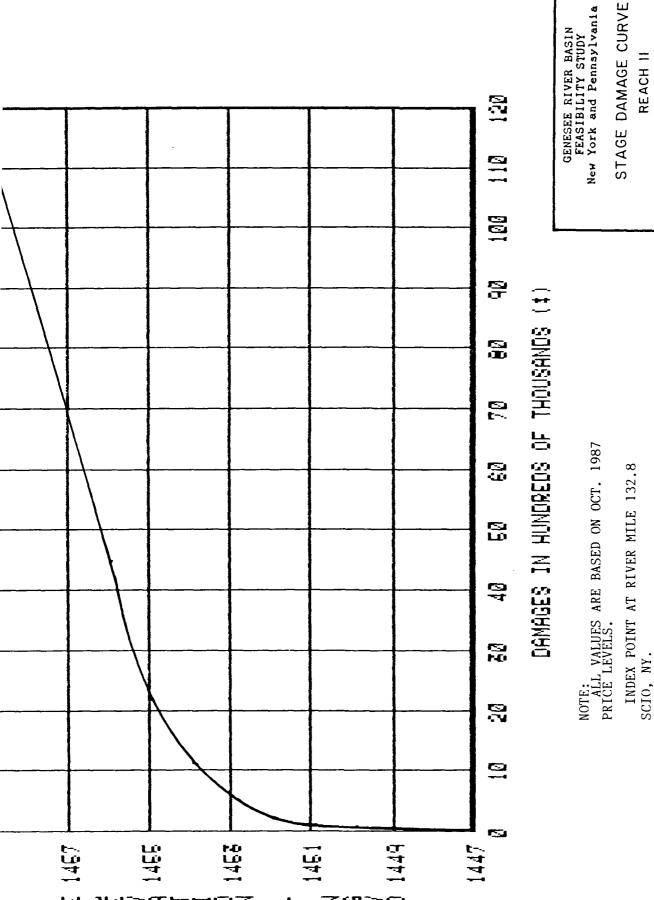


FIGURE A36

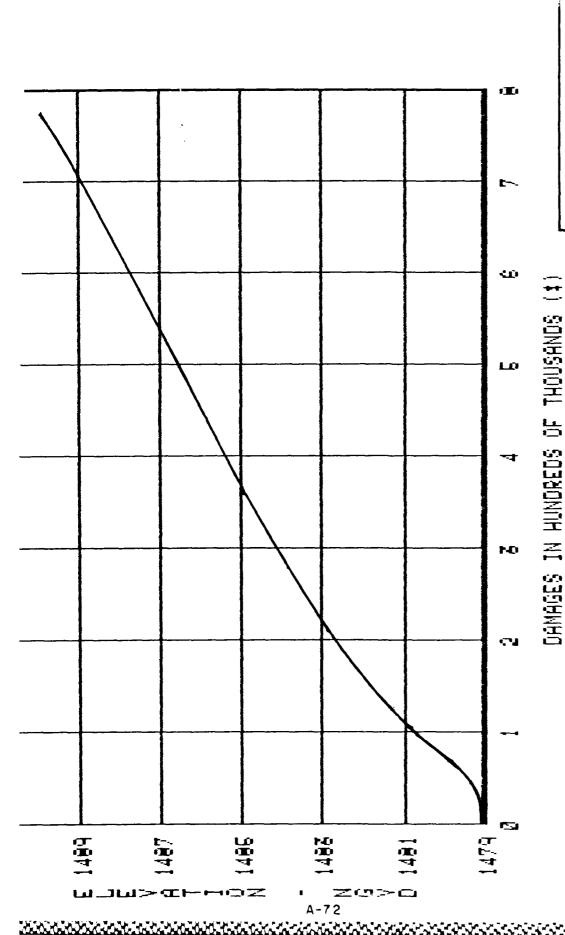
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REACH 11

U.S.Army Engineer District Buffalo

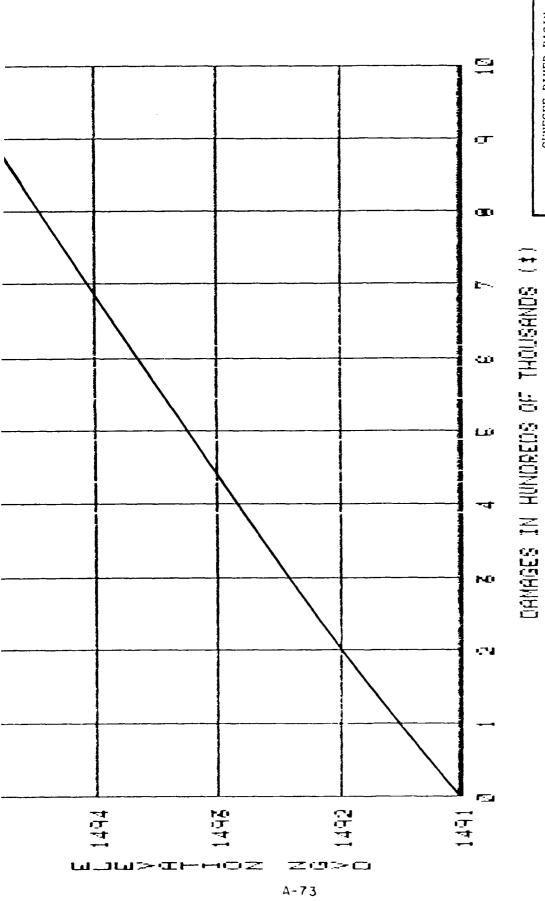


U.S. Army Engineer District Buffalo STAGE DAMAGE CURVE REACH 12 (G-1)

NOTE:

ALL VALUES ARE BASED ON OCT. 1987 PRICE LEVELS.

INDEX POINT-1400' DOWNSTREAM OF BOLIVAR ROAD BRIDGE



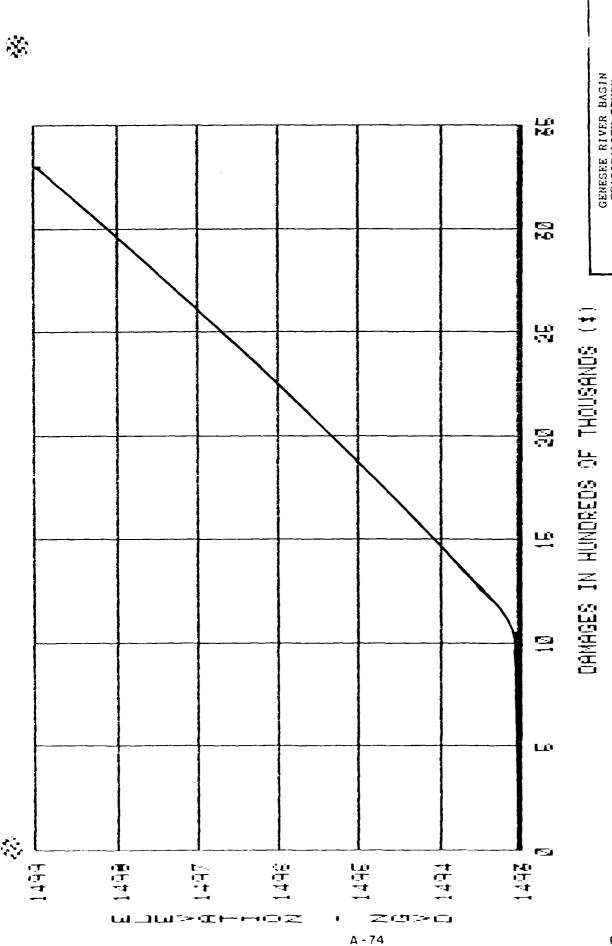
STAGE DAMAGE CURVE REACH 12 (6-2)

U.S.Army Engineer District Buffalo

ALL VALUES ARE BASED ON OUT, 1987 PRICE LEVELS.

NOTE:

INDEX POINT-1500' UPSTREAM OF BOLIVAR ROAD BRIDGE



STAGE DAMAGE CURVE

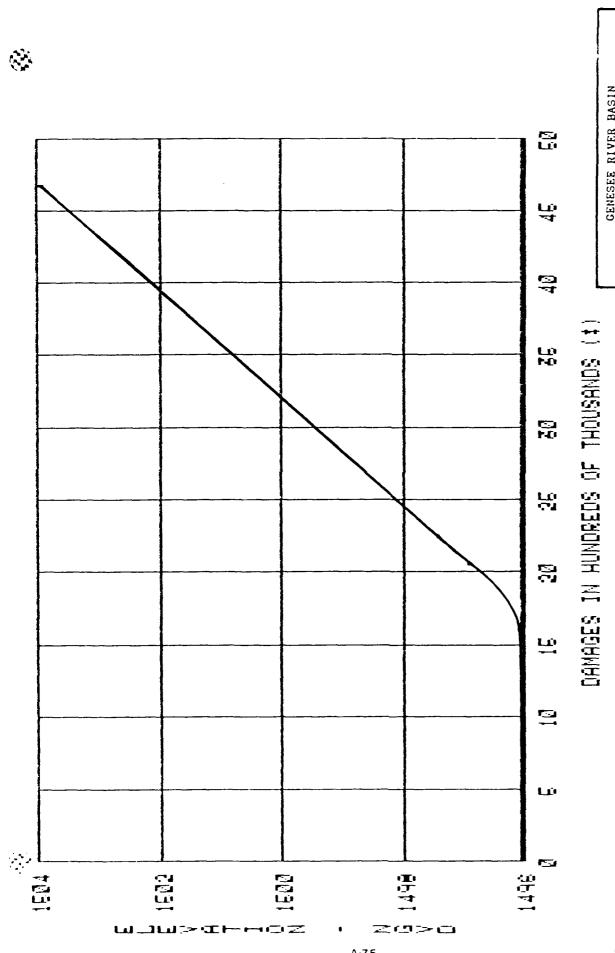
U.S. Army Engineer District Buffalo REACH 12 (G-3)

NOTE:

INDEX POINT-100' DOWNSTREAN OF OLD

PEARL STREET BRIDGE

ALL VALUES ARE BASED ON OCT. 1987 PRICE LEVELS.



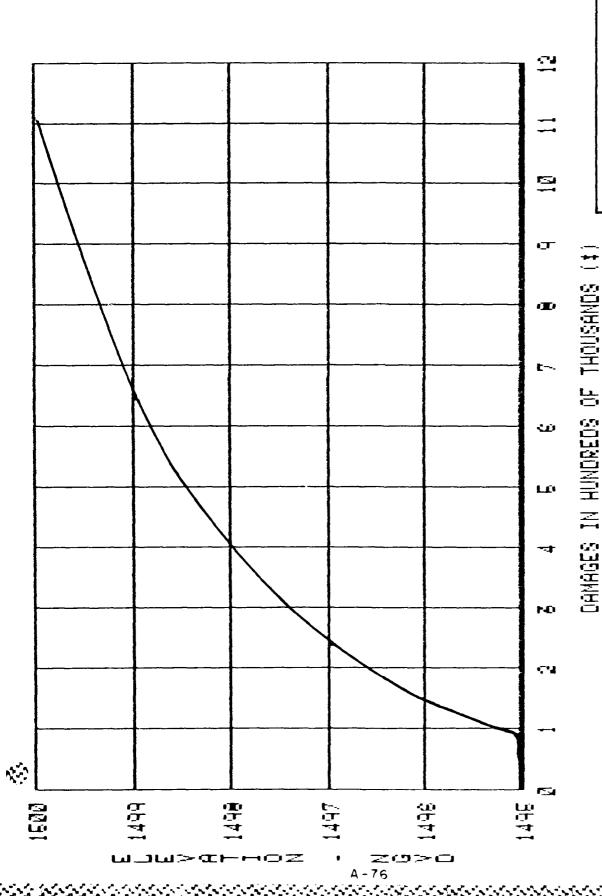
STAGE DAMAGE CURVE REACH 12 (G-4)

U.S.Army Engineer District Buffalo

NOTE:

ALL VALUES ARE BASED ON OCT, 1987 PRICE LEVILS.

INDEX POINT-100' DOWNSTREAM OF STATE STREET BRIDGE



GENESEE RIVER BASIN FEASIBILITY STUDY New York and Pennsylvania

STAGE DAMAGE CURVE

REACH 12 (G-5a) U.S.Army Engineer District Buffalo NOTE:

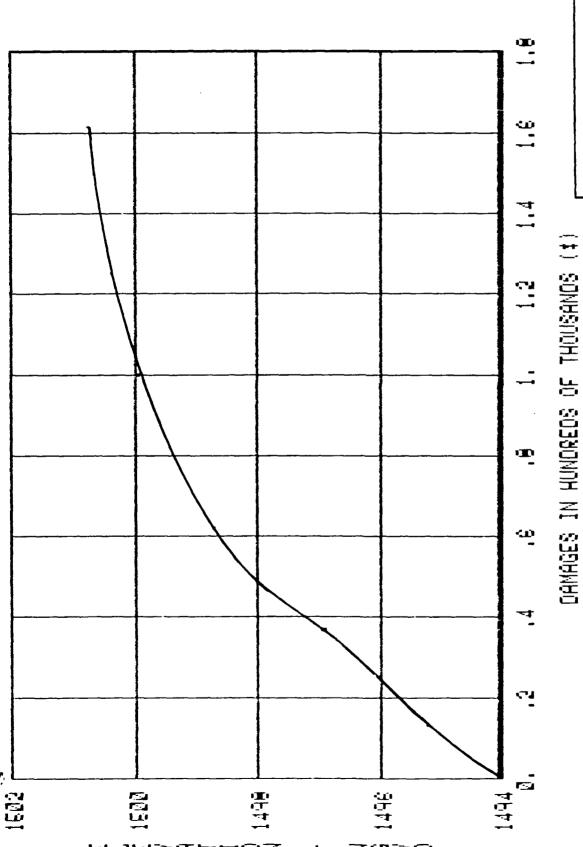
ALL VALUES ARE BASED ON OCT, 1987

PRICE LEVELS.

INDEX POINT-1100' UPSTREAM OF

STATE STREET BRIDGE



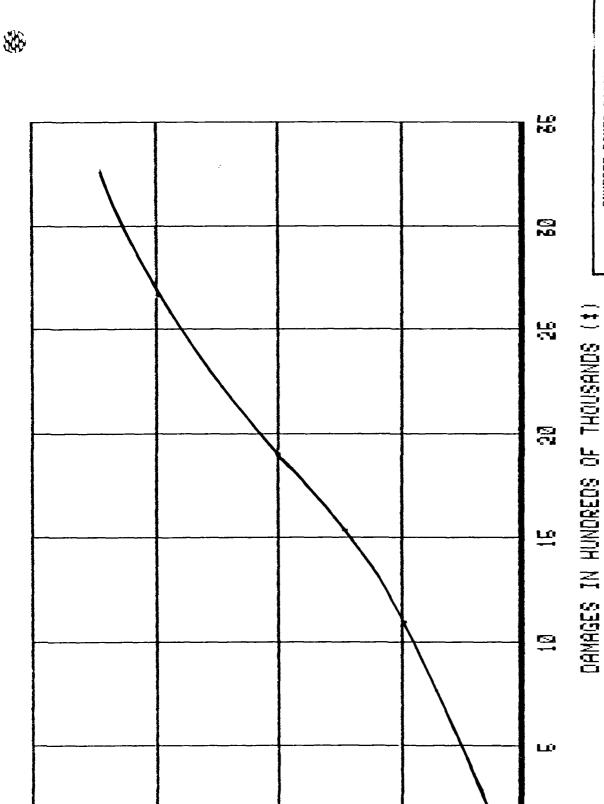


New York and Fennsylvania

STAGE DAMAGE CURVE REACH 12 (G-5b) U.S.Army Engineer District Buffalo

ALL VALUES ARE BASED ON OCT. 1987 PRICE LEVELS.

INDEX POINT-1100' UPSTREAM OF STATE STREET BRIDGE



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GENESEE RIVER BASIN FEASIBILITY STUDY New York and Pennsylvania

STAGE DAMAGE CURVE REACH 12 (G-6)

U.S.Army Engineer District Buffalo

FIGURE A44

NOTE:

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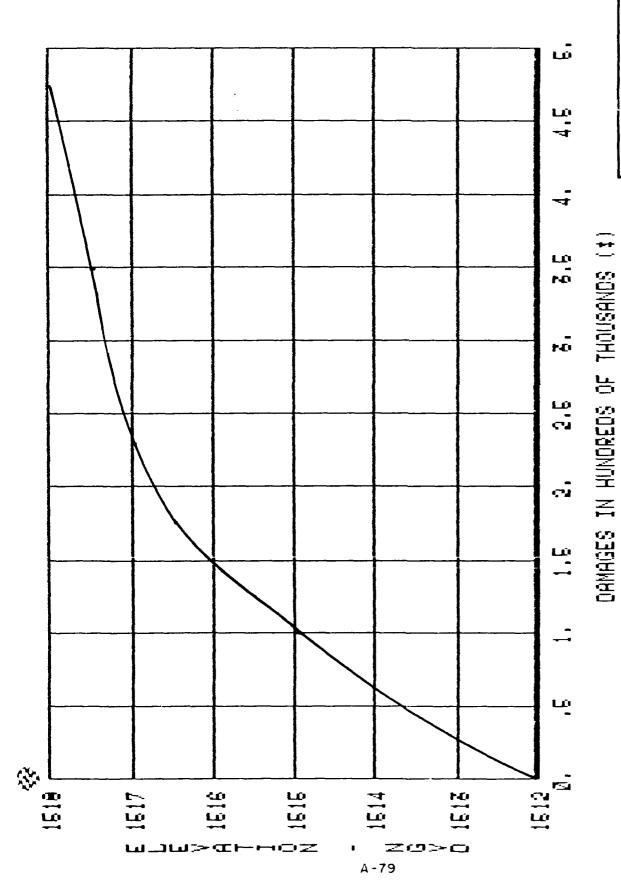
A-78

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ALL VALUES ARE BASED ON OCT. 1987 PRICE LEVELS.

INDEX POINT-1200' UPSTREAM OF OLD MUNICIPAL DAM





STAGE DAMAGE CURVE

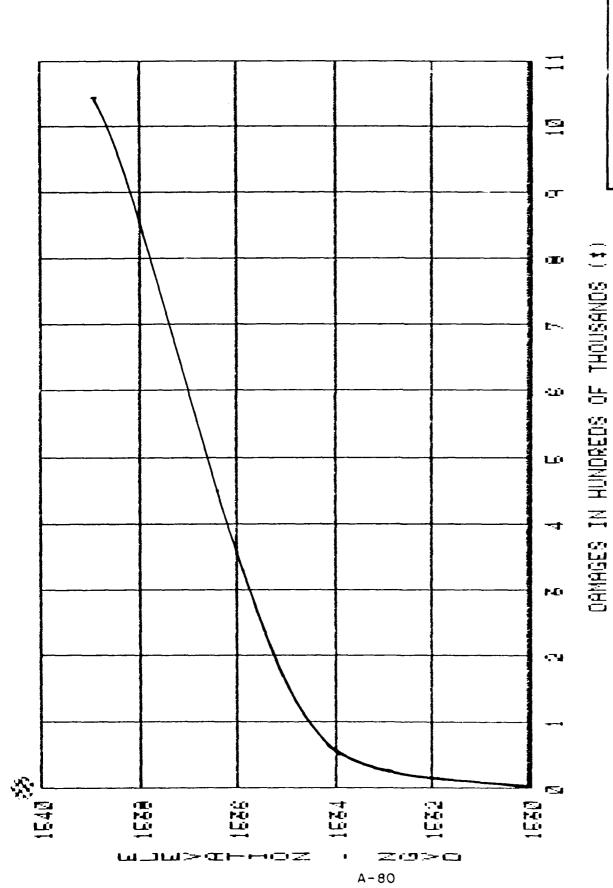
REACH 13

U.S. Army Engineer District Buffalo

NOTE: ALL VALUES ARE BASED ON OCT 1987 PRICE LEVELS.

INDEX POINT AT RIVER MILE 139.4

STANNARDS CORNERS, NY.



GENESEE RIVER BASIN FEASIBILITY STUDY New York and Pennsylvania STAGE DAMAGE CURVE

U.S.Army Engineer District Buffalo

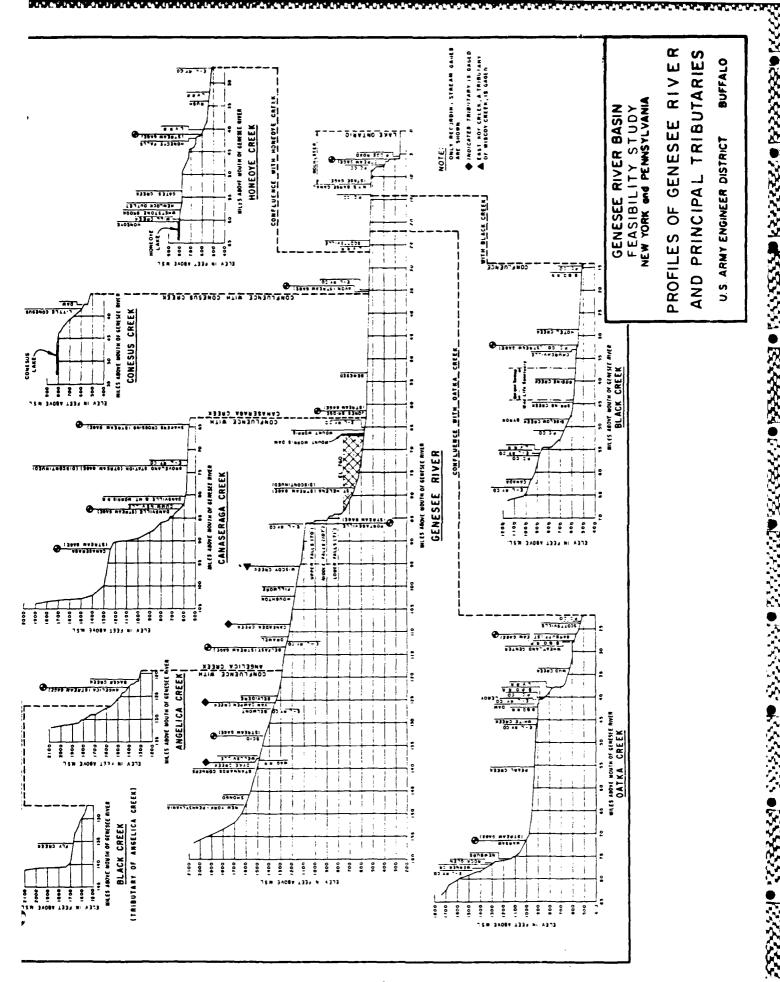
REACH 14

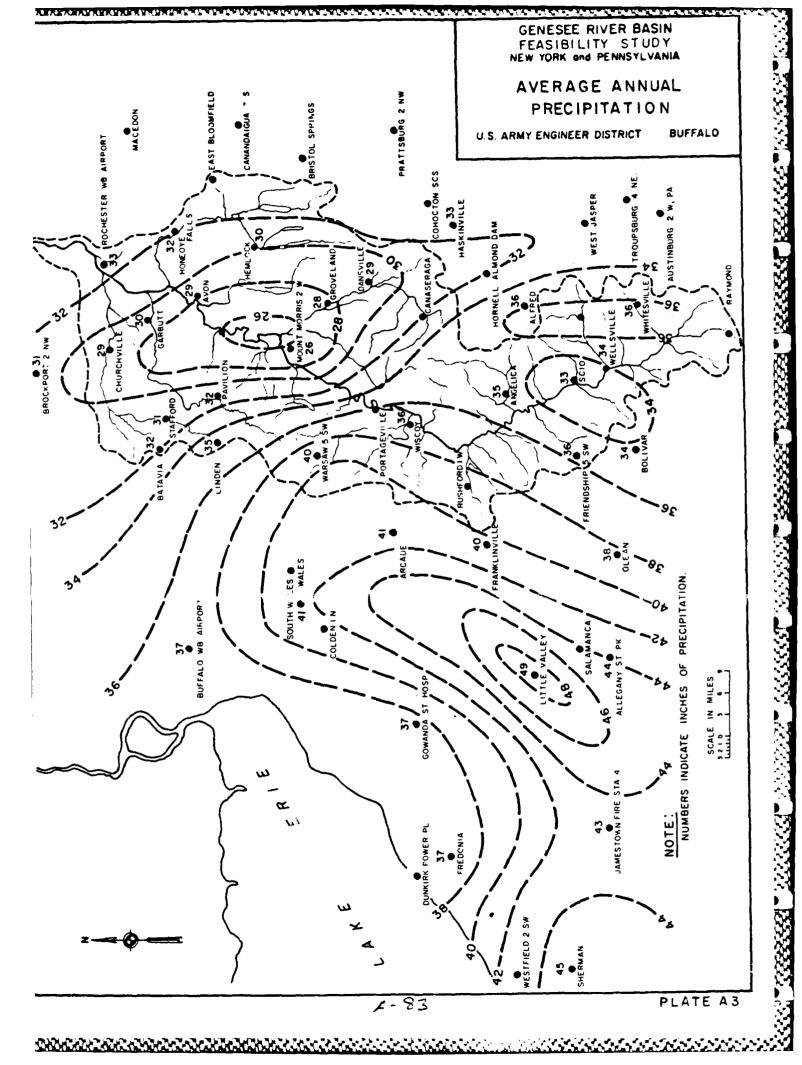
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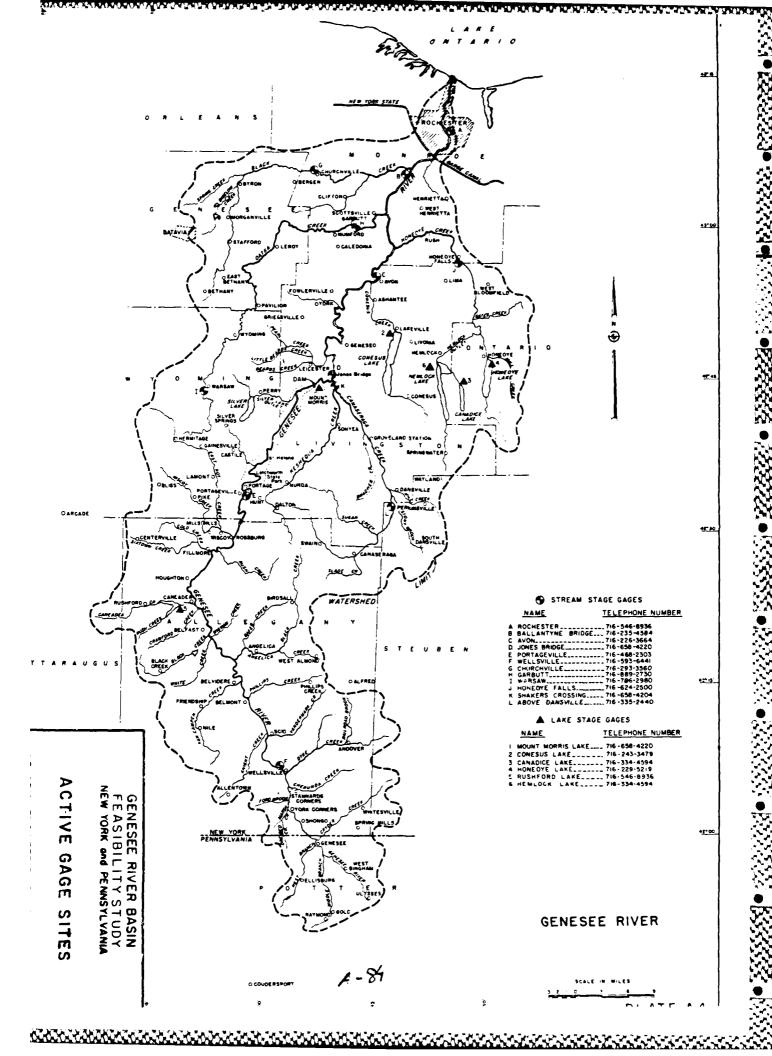
ENDEX POINT AT RIVER MILE 141.1 SHONGO, NY.

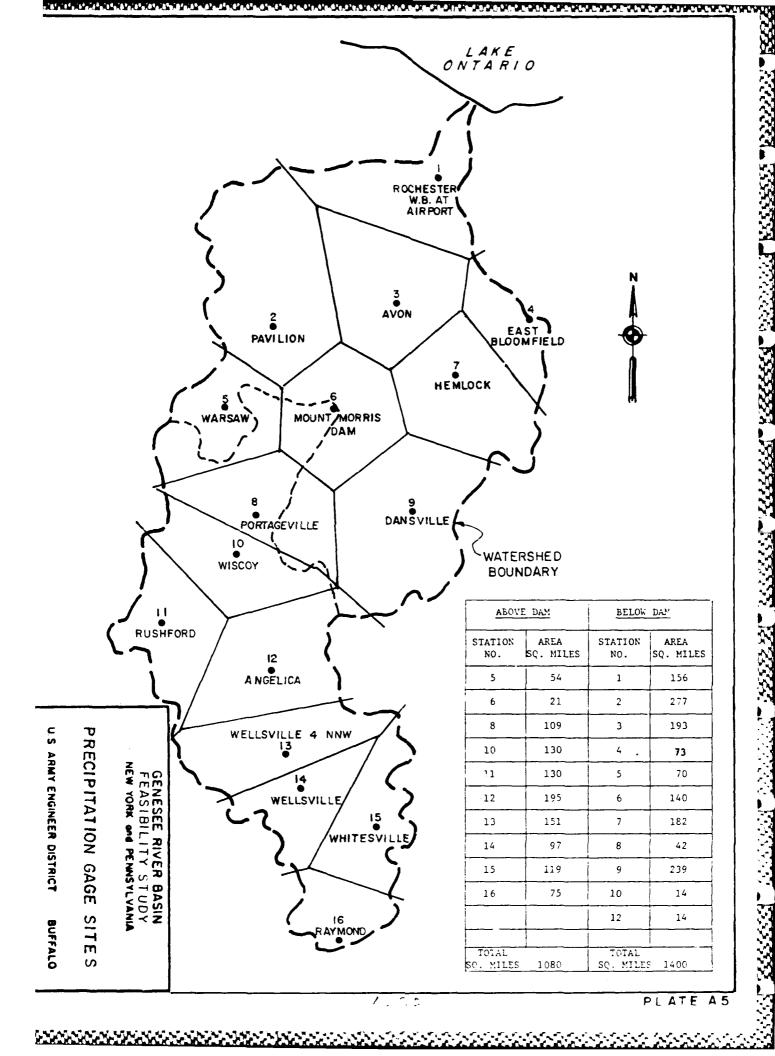
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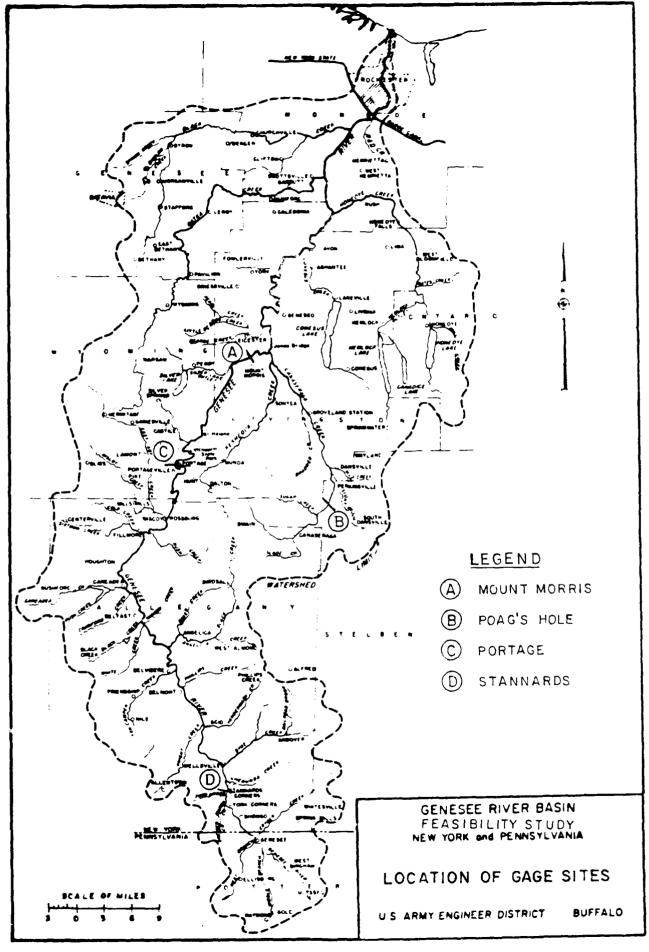
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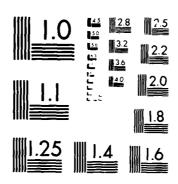
PLAN VIEW OF STANNARDS BUFFALO GENESEE RIVER BASIN FEASIBILITY STUDY NEW YORK ON PENNSYLVANIA CTAN MOUTES U.S. ARMY ENGINEER DISTRICT Crossonoline secure of sold Coy (L. (N) growing LEGEND FOR EXPLORATIONS - Parter inde secrité de Mer EXPORTS TO OF SEEDINGS GEOLOGIC PROFILE AT DAMSITE 100 10 1 80,00 180"CAL 3CALE 61455/64"ED CONCRETE SPILLWAY SECTION 200 200 200 ن HESTREAM ELEVATION سیمشرک شعث

PLATE A

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PLATE A 7

GENESEE RIVER BASIN STUDY VOLUME 2 SUPPORTING DOCUMENTATION(U) CORPS OF ENGINEERS BUFFALD BY BUFFALD DISTRICT JUM 88 80-8197 479 2/3 UNCLASSIFIED F/G 8/8



MICROCOPY RESOLUTION TEST CHART NATIONAL BUREAU OF STANDARDS 1963-A

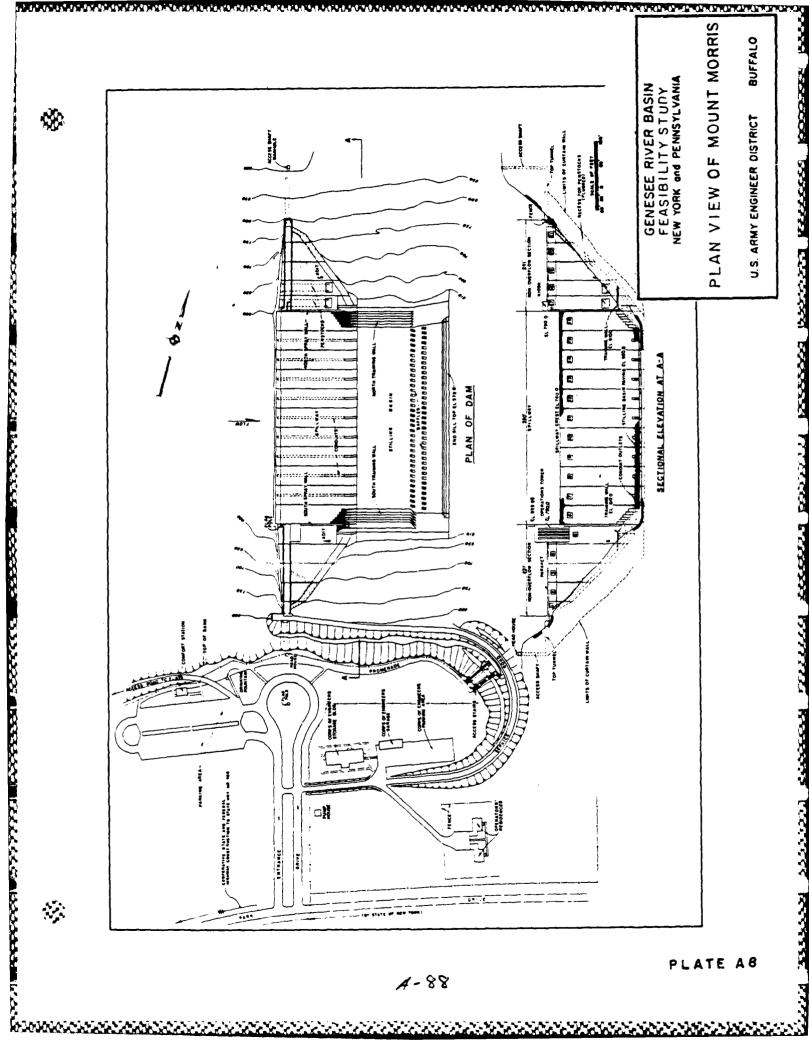


PLATE AB



APPENDIX B ECONOMICS





GENESEE RIVER BASIN STUDY
NEW YORK
FINAL FEASIBILITY REPORT

APPENDIX B

ECONOMICS

U.S. Army Engineer District 1776 Niagara Street Buffalo, New York 14207

APPENDIX B

ECONOMICS

BI. GENESEE RIVER BASIN WATERSHED

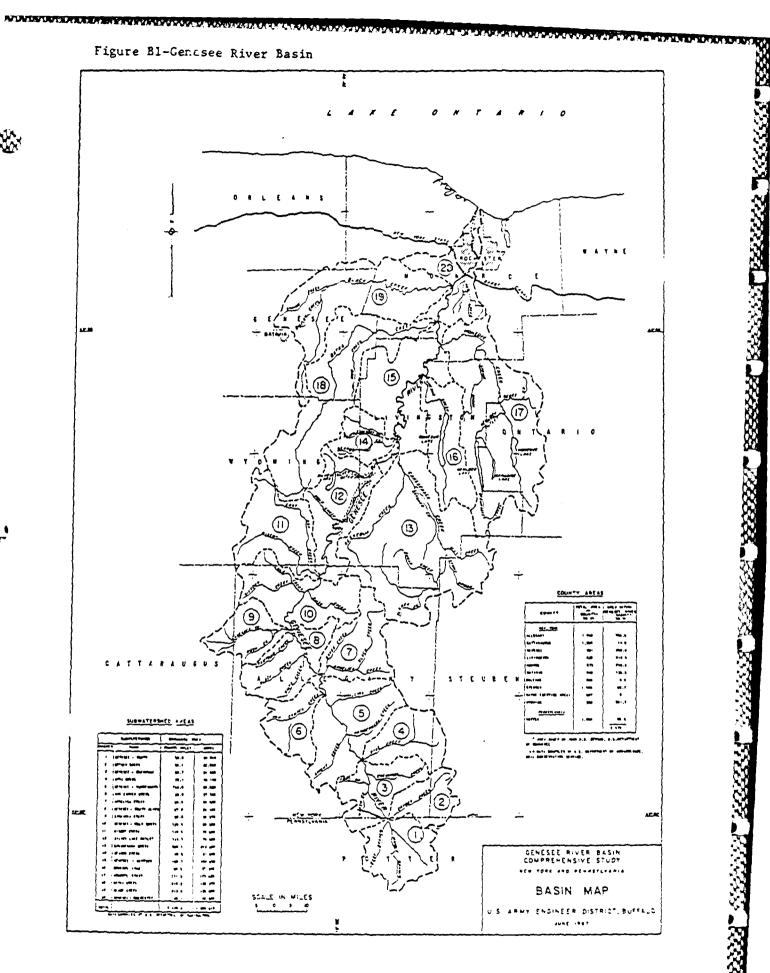
a. Location and Size. The Genesee River Basin is located in Allegany, Genesee, Livingston, Monroe, and Wyoming Counties. The basin encompasses a total drainage area of 2,480 square miles (Figure Bl). The watershed is roughly elliptical in shape, with a north-south major axis of approximately 100 miles and a maximum width of 40 miles. The river flows north for about 157 miles to Rochester, New York.

The Genesee River Basin is the watershed of the Genesee River. The headwaters are in extreme north-central Pennsylvania, and flows northward into Lake Ontario. The river descends down the northern edge of the Allegany Plateau. The plateau encompasses about 60 percent of the Genesee River Basin. The plateau consists of rounded hills with intervening valleys. The Genesee flows off of the Allegany Plateau at the Portage escarpment, near the village of Mount Morris. Here the river has a fairly steep gradient. The channel has eroded down through bedrock to form a deep gorge which is occupied by Letchworth State Park. The gorge is a natural reservoir site and Mount Morris Dam has been constructed in the gorge. Mount Morris Dam was built and is operated by the Corps of Engineers as a single function dam. The dam provides flood protection to the lower Genesee River Valley, principally to the city of Rochester and its surrounding suburbs. Flowing down the Portage escarpment, the river exists onto a lake plain before emptying into Lake Ontario. Once on the lake plain, the Genesee River meanders across it at a very low grade. A nearly flat valley, up to three miles wide has been formed. The river than flows into Lake Ontario in the city of Rochester.

The largest tributary of the Genesee River is Canaseraga Creek. This watershed drains 337 square miles. The Creek's basin is roughly square in shape with about 20 miles to a side. The main stem is 42 miles long. Its confluence with the Genesee River is near Jones Bridge, just downstream of Mount Morris. Canaseraga Creek resembles the Genesee River Basin. The Creek's reaches upstream of Dansville are steep and rugged. Downstream of Dansville, the creek flows through a flat alluvial plain to the Genesee River. The major tributaries of the lower basin (the main stem below Mount Morris) are: Canaseraga Creek, Honeoye Creek, Oatka Creek, and Black Creek. Other tributaries of the Genesee River have a wide range in size and topographic characteristics. Information on these tributaries and their drainage areas are presented in the Hydrology and Hydraulics Appendix.

b. <u>Historical Flooding</u>. Damaging floods have occurred in all months of the year except August. Summer floods are generally localized in a part of the watershed caused by convectively usable air conditions. Winter and spring floods are usually the result of frontal precipitation on saturated or frozen ground, or on a melting snow cover. Floods have occurred from melting snow cover alone. Some of the larger floods are: March 1865, March 1875, June 1889, May 1894, April 1896, March 1902, July 1902, March 1913, March 1916, May 1916, December 1927, July 1935, July 1942, March-April 1950, November 1950,





B-2

March 1956, March-April 1960, April 1961, June-July 1972, and February-March 1976. Since 1976, most of the flooding in the basin has been limited to the Black and Oatka Watersheds.

B2. PLANS OF IMPROVEMENT

- a. Reconnaissance Report Findings. Twelve plans were developed to meet the needs of the basin. The assessment indicated that seven plans, including the "No Action" Plan, warranted further investigation. These plans were evaluated in the feasibility phase.
- b. Plans Evaluated. Reconnaissance Report Plans 1, 2, 6, 7, 8, 10, and 11 were evaluated in the feasibility phase. Plan 2 was the "No Action" Plan. However, only Plan 2 and components of Plan 1 and Plan 10 (Plan 10A) were given further consideration. (See the Main Report for further information on Plan 10A). Plan 10A considered adding gates to the top of the spillway of the existing Mount Morris Dam for flood control purposes only. Regulation of outflows was also a component. The Economic Appendix presents the benefits and costs associated with three gate heights: Scenario 1: 12-Foot Gates, Scenario 2: 22-Foot Gates, and Scenario 3: 30-Foot Gates.

The "Plan" calls for the addition of tainter gates on top of Mount Morris' existing spillway. A total of ten tainter gates, 42 feet wide, varying in height from 12 to 30 feet, would be attached to eleven new eight foot wide concrete piers. (See Plate 4, in the Main Report.) A service bridge would span the dam at the top of the bridge. The bridge would be 16 feet wide and consist of precast box beams, 36 inches deep. The height of the gate would vary with the degree of desired flood damage reduction. Several schemes were evaluated for estimating the degree of reduction in average annual damages. These gates would substantially increase the storage available for flood control. This will allow the release of lesser flows to the basin downstream reaches over a longer period of time than is called for under the established reservoir pool evacuation in effect. This would reduce the occurrence of the more frequent, but less severe floods and would minimize erosion rates in the lower basin. Also the less frequent, but more severe floods, like the 1972 "Tropical Storm Agnes," would be more effectively controlled. A more detailed description of Plan 10A components are presented in the Main Report.

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B3. PROJECT AREA AND AFFECTED AREA

- a. Planning Region. All plans considered for further evaluation are located in the lower basin of the river. Consequently the Planning Region was restricted to those counties in the lower basin. These counties are: Livingston, Monroe, and Wyoming.
- b. Project Area. The Planning Region for the Final Feasibility Report consists of the above mentioned three counties. Lands along the main stem of the river in these counties were divided into three reaches: Reaches 4, 3, and 2. These reaches constitute the project area. Reach 4 extends from River Mile 65.5 to 35.3 (From the Rochester Gas and Electric Dam located 1.3 miles downstream of Mount Morris to the Avon Bridge). Reach 3 extends from River Mile 35.3 to 21.1 (From the gate site at Avon Bridge to Browns Bridge). Reach 2 extends from River Mile 21.1 to 11.6 (From Browns Bridge to the New York State Barge Canal).

- c. Affected Area. The area affected by the proposed plans consist of the flood plain in the three reaches plus all nearby areas likely to serve as an alternative site for any major type of activity that might use the flood plain if it were protected.
- B4. PROJECTED LAND USE ON THE FLOOD PLAIN
- a. Without Plan Conditions of Development. Again, the project area consists of the flood plain in the three reaches plus all nearby areas likely to serve as an alternative site for any major activity that might use the flood plain if it were protected. Basic economic data for the Planning Region was developed to determine "Without Plan" condition land use patterns.
- 1. Economic Variables Table B4.1 presents projections of such basic economic variables as population, employment, and personal income per capita for the three county Planning Region. Population shows a little over .3 percent annual growth from 1980 to 2035. Employment is projected to grow around .43 percent annually. Personal income per capita shows a l percent annual growth rate.

Table B4.1	- Projections of Basic Economic Variables in the	
	Lower Genesee River Basin's Planning Region (1) (2)	

Date	:	Population	:	Employment	:	Personal Income Per Capita
	:		:		:	
1980	:	799,139	:	359,353	:	\$ 10,227
1985	:	815,900	:	383,482	:	\$ 10,680
1990	:	855,000	:	418,900	:	\$ 11,920
2000	:	900,200	:	459,100	:	\$ 13,530
2015	:	942,400	:	471,200	:	\$ 15,420
2035	:	967,000	:	454,500	:	\$ 18,320
	:	·	:	,	:	·

- (1) The counties of the lower Genesee River Basin include: Livingston, Monroe, and Wyoming.
- (2) Projections are based on projected values for Upstate New York (New York State minus counties of the New York State portion of the New York PMSA) presented in Volumes 1 and 2 of 1985 OBERS, BEA Regional Projections.
- 2. Land Use Data The data in Table B4.2 reflects land use patterns in the Genesee River Basin on a county wide basis. Counties looked at include Allegany, Genesee, Livingston, Monroe, and Wyoming. With the exception of Monroe, all of the counties are predominantly rural counties.

Land use data based on Minor Civil Divisions (MCD's) with the five counties was also derived (River Strip Analysis). The land use data for MCD's in Allegany, Genesee, Livingston, and Wyoming Counties come from New York State Land Use and Natural Resource Inventory, 1968. Land use data for the MCD's in Monroe County are based upon 1982 data derived from the Assessment Roll and Levy Module provided by the Monroe County of Planning.

Table B4.2 is believed to provide a reasonable reflection of current land use patterns in the MCD's of the three counties in the lower basin. A degree of judgement was used in developing these estimates. Land use percentages developed in Table B4.2 were compared to land use percentages for Livingston County alone (Table B4.3). Livingston County land use is representative of agricultural land use in the lower basin of the Genesee River. Comparison of the two tables indicate Table B4.2 does provide a reasonable estimate of land use in the lower basin of the Genesee River.

Finally, Table B4.4 presents land use projections for the counties in the Genesee River Basin. Table B4.4 highlights the relative stability of the "Agriculture" and "Forest" land use category. The conclusion of Table B4.2 to B4.4 is current land use patterns will not change dramatically in the future in the lower basin. The lands will continue to be predominantly agricultural and forest in nature (77 percent in 1980 and 75 percent in 2035). This is especially true of Livingston County.

Table B4.2 - Land Use Patterns in the Genesee River Basin (1)

	: Three Counties					River Strip (2)			
	:	Square Miles	:	Percent	:	Square Miles	:_	Percent	
	:		:		:		:		
Land Use Category	:	1	:	2	:	3	:	4	
	:		:		:		:		
Agriculture	:	1,649	:	42.85	:	339	:	38.83	
Forest	:	1,337	:	34.75	:	297	:	34.02	
Wetlands	:	148	:	3.85	:	13	:	1.49	
Water	:	52	:	1.35	:	13	:	1.49	
Residential	:	130	:	3.38	:	58	:	6.64	
Commercial	;	14	:	.36	:	9	:	1.03	
Industrial	:	13	:	.34	:	5	:	.57	
Extractive	:	60	:	1.56	:	10	:	1.15	
Public & Semi-Public	:	25	:	.65	:	13	:	1.49	
Outdoor Recreation	:	52	:	1.35	:	30	:	3.44	
Transportation	:	16	:	.42	:	8	:	.92	
Non-productive	:	352	:	9.15	:	78	:	8.93	
-	:		:		:		:		
All Land Uses	:	3,848	:	100.00	:	873	:	100.00	
	:	•	:		:		:		

SOURCE: New York State Land Use and Natural Resource Inventory, 1968.

NOTES:

- (1) Six counties include: Allegany, Genesee, Livingston, Monroe, Orleans, and Wyoming.
- (2) Genesee River Strip includes: the municipalities, towns, and the city of Rochester, through which the Genesee River flows.

Table B4.3 - Land Use in Livingston County

	:	Area	:	Area
Land Use Category	:	(Square Miles)	:	(Percent)
	:		:	
Agriculture	:	356.06	:	55.8
Woodlands	:	193.34	:	30.3
Wetlands	:	8.29	:	1.3
Waterbodies	:	9.57	:	1.5
Residential	:	9.57	:	1.5
Commercial	:	1.28	:	• 2
Industrial	:	.64	:	.1
Extractive	:	1.28	:	• 2
Public & Semi-Public	:	3.83	:	.6
Outdoor Recreation	:	14.04	:	2.2
Transportation	:	.64	:	.1
Non-productive	:	39.56	:	6.2
•	:		:	
All Land Uses	:	638.10	:	100.0
	:		:	

SOURCE: New York State Land Use and Natural Resource Inventory, 1968.

Table B4.4 - Land Use Projections in the Counties of the Genesee River Basin (1) (2)

	:			Perce	ent	of Total	Land	Use		
Land Use Category	:	1968	:	1980	:	2000	: -	2015	:	2035
	:		:		:		:		;	
Agriculture	:	42.8	:	42.3	:	41.5	:	40.8	:	40.0
Forests	:	34.8	:	34.7	:	34.5	:	34.6	:	35.0
Urban (3)	:	5.1	:	5.3	:	5.5	:	5.7	:	6.0
Conservation &	:		:		:		:		:	
Recreation	:	6.5	:	6.6	:	6.7	:	6.8	:	6.9
Other Uses (4)	:	10.8	:	11.1	:	11.8	:	12.1	:	12.1
	:		:		:		:		:	
Total	:	100.0	:	100.0	:	100.0	:	100.0	:	100.0
•	:		:		:		:		:	

NOTES:

- (1) The counties of New York encompassed by the Genesee River Basin have been defined to include: Allegany, Genesee, Livingston, Monroe, and Wyoming.
- (2) Projections based on 1968 LUNR data: projections developed by the Economics Branch, Buffalo District.
- (3) Urban land use has been defined to include LUNR categories of: residential, commercial, industrial, public and semi-public, and transportation uses.
- (4) Other Uses include: the LUNR category of non-productive use and extractive use.

b. With Plan Condition of Development. The previous section evaluated land use and indicated that no future change is expected in the project area. Livingston and Wyoming Counties will continue to be predominantly rural in nature. The portion of Monroe County in the lower basin will show some growth in residential use. However, based upon development patterns of the last 15 years, development will be confined to the non-flood plain areas of Reach 2.

B5. URBAN FLOOD DAMAGES AND BENEFITS

a. <u>Introduction</u>. The previous section evaluated land use and indicated that no future change is expected in the flood plain area of the three reaches. Most of the flood damage is the result of first floor flooding of residences and commercial establishments.

Public and other damages include damages to public buildings and contents (police stations, firehouses, churches, schools), as well as roads in the project area. Also included in these damages are public sector emergency services and cleanup costs. Repair work to structures and roads are made upon the return of normal water conditions. Urban flood benefits will be the difference between "Without Project" urban flood damages and "With Project" urban flood damages. The "With Project" flood damages will look at three variations of gate height: Scenario 1: 12-foot gates, Scenario 2: 22-foot gates, and Scenario 3: 30-foot gates.

b. "Without Project" Conditions Damages. Urban damages includes residential structure and content damages, commercial structure and content damages, and public and other damages (public structures, contents, emergency services, cleanup costs, and infrastructure repair).

1. Damages Under Existing Conditions -

- (a) Introduction. The best information available on damages in the lower Genesee River Basin is contained in the Flood Report on Tropical Storm Agnes. Damage estimates for the lower basin (Reaches 2, 3, and 4) were revised to reflect current conditions based on field surveys and interviews performed in 1987 and 1988. Price levels were updated to January 1988 price levels. Table B5.1 presents study year 1988 existing urban inundation damages for the three reaches of the lower basin.
- (b) Methodology. Flood stages and corresponding damages at various water levels were calculated and plotted for each of the three reaches at their respective index points. These points were used to develop stage damage curves for each land use activity taking place at each of the three reaches. The Hydraulic Design Appendix explains the derivation of stage damage curves for the three reaches.

All commercial damage estimates are based upon personal interviews with the commercial establishments in the project area during 1972 and updated to January 1988 price levels. The interview included estimated damages to structures, inventory and machinery, lost wages, and expected cleanup costs. During the interviews the overall conditions of the building and equipment were identified. Also identified was the type and value of inventory and its susceptibility to being damaged by flood water relative to the first floor elevation of the individual structures.

Residential damages were estimated at various flood depths based on established Buffalo District depth-percent-damage relationships for typical residential structures in the project area. First floor elevations, type of structure, and value of structure were used to determine damages at various depths.

Public and other damages include flood clean-up costs on public roads and buildings, repairs to utilities and roads, emergency evacuation costs (additional on duty time for police, firemen, etc.), and temporary emergency shelter and relief costs (Red Cross). Estimates of these costs were updated to reflect 1988 price levels.

- (c) Existing Average Annual Damages, Without Project Conditions, Study Year 1988 Conditions of Development. Average annual damages are the expected value of flood damages for any given year. Discharge-frequency curves and stage discharge curves were developed for each land use activity type located in the three reaches. These curves were used with stage damage curves to determine damage frequency relationships existing under 1988 study year conditions of development. The resulting existing "Without Project" average annual damages at January 1988 levels of development and prices are presented in Table B5.1. Existing "Without Project" average annual damages at January 1988 levels of development and prices are \$1,014,300 (Table B5.1, Existing 1988). This is total average annual damages for Reaches 2, 3, and 4.
- (d) Existing Average Annual Damages, Without Project Conditions, Base Year 1995. "Without Project" damages as of the study year (January 1988) were projected to the Base Year 1995. Residential content damages at the study year (1988) were adjusted to reflect the projected increase in the real value of residential contents between the study year (1988) and the base year (1995) of the project. This adjustment was based upon the real increases in regional per capita income between the study year and base year.

Existing "Without Project" average annual damages as of the base year, 1995, are presented in Table 5.1 in the column labeled "Base Year, 1995." The respective existing average annual damages, in January 1988 prices, by reach are: Reach 2: \$494,000; Reach 3: \$406,000; Reach 4: \$123.600.

2. Damages Under Future Conditions - An evaluation of the potential for future growth in land use at each of the three reaches (Section B4. PROJECTED LAND USE ON THE FLOOD PLAIN) indicated there would be no change in land use in the three reaches. However, future residential content damages will rise due to an increase in residential content value over time. The value of residential contents is expected to increase as a result of rising regional per capita income. As people have more money available for personal consumption, they tend to increase the value of their stock of personal property. This increase in value is a direct result of regional per capita income growing over time. this increase in residential content value is calculated as follows.

Current guidance states residential content value can rise to 75 percent of structure value. The value of residential contents to residential structures during the study year (January, 1988), was estimated as 33 percent. The value of these contents are then allowed to grow at a given percent per year. This growth rate is assumed to equal regional per capita income growth. All of the urban damages evaluated were outside of the city of Rochester. It was felt the

percent change in regional per capita income for the Rochester SMSA would not be representative of the basin's affluence growth rate. A proxy for income growth for the lower basin was devised. New York FMSA income and population levels were subtracted from total New York State income and population levels (1985 OBERS BEA Regional Projections). The residuals were assumed to equal the lower basins affluence growth rate. This resulted in a per capita income value of \$10,334 in 1983 and a \$15,766 per capita income value in the year 2035. Per capita income will increase at an annual compound growth rate of 1.12815 percent. It is assumed that residential content growth occurs at the same rate as the regional per capita income growth rate. Given a 1.12815 percent compound growth rate, residential content value will increase to 75 percent in 73 years. Residential contents would need to grow from 1988 (year of the project evaluation) to 2061 for the ratio of residential content value to structural value to equal 75 percent. Residential content value growth was allowed to grow to Project Year 50 (2045). Residential content value was straightlined thereafter.

$$(1 + r)^n = Xt/Xi$$

$$n = \frac{Ln(Xt/Xi)}{Ln(1+r)}$$

$$n = \frac{Ln(.75/.33)}{Ln(1.0112815)}$$

$$n = 73$$

WHERE:

n = Number of years residential contents will increase in value

r = Compound growth rate (1.0112815)

Xt = Ratio of content value to structural value in the terminal year of growth.

Yi = Ratio of content value to structural value in the initial year of growth.

Table B5.1 shows the projected growth of "Existing Condition" residential content damages for a 100 year evaluation period, starting from the Base Year 1995 to the Terminal Year 2095. Total "Existing Condition" average annual residential content damages are \$136,600.

The average annual value of existing residential content damages for Reaches 2, 3, and 4 came to \$118,300, \$13,700, and \$4,600.

Table B5.1 presents "Without Project" total average annual urban damages for Reaches 2, 3, and 4. These damages came to \$517,400; \$408,700; and \$124,500, respectively. "Without Project" average annual urban damages for all three reaches came to \$1,050,600.

- c. Residual Average Annual Damages.
- 1. Introduction "Without Project" damages were calculated up to the .2 percent chance occurrence (500-year event). However, the three "Scenarios"

proposed will not eliminate all of the damages occurring in these three reaches. Therefore, existing and future damages (growth in residential content value) were developed under "With Project" conditions for the three scenarios. These total "With Project" average annual damages by scenario are presented in Table B5.2. These damages reflect January 1988 price levels and conditions of development, an 8-5/8 percent annual interest rate, and a 100 year evaluation period (1995-2095).

- 2. Plan 10A: Scenario 1: 12-Foot Gates Residual average annual urban flood damages for Reaches 2, 3, and 4 came to \$365,100, \$276,600, and \$34,500. Total residual average annual urban flood damages for all three reaches came to \$676,200.
- 3. Plan 10A: Scenario 2: 22-Foot Gates Residual average annual urban flood damages for Reaches 2, 3, and 4 came to \$285,200, \$207,200, and \$0. Total residual average annual urban flood damages for all three reaches came to \$492,400.
- 4. Plan 10A: Scenario 3: 30-Foot Gates Residual average annual urban flood damages for Reaches 2, 3, and 4 came to \$136,700, \$115,100, and \$0. Total residual average annual urban flood damages for all three reaches came to \$251,800.
- d. Other Costs of Using the Flood Plain. The impact of flooding on existing occupants is not limited to flood losses alone. Other incurred costs may also be NED losses.
- 1. Floodproofing Costs Floodproofing costs avoided are the reduction in future floodproofing costs due to a specific Flood Control Scenario being implemented. Implementation of a scenario would reduce the susceptibility of residential, commercial, and public structures to flood damages. Since the Scenario is to be implemented, the need to floodproof would diminish. These floodproofing costs can be claimed as a benefit for the implementation of the Scenario. Since new development in the flood plain area itself is projected to be minimal, under "Without Project" and "With Project" conditions, these added costs would be negligible.
- 2. National Flood Insurance Costs A National Flood Insurance Program is available to protect the individual in participating communities from extreme financial loss in the event of a disastrous flood. Savings of flood insurance operating costs can be credited to the NED benefit account. This happens when a project eliminates inundation of an area due to floods having an exceedence frequency of 1 percent or less. The national cost of the Flood Insurance Program is its administration costs. However, implementation of any of the Scenarios would not induce local flood plain residents to allow their flood insurance to lapse. Thus, there is no savings of flood insurance operating costs credited to these Scenarios.
- 3. Modified Use In some cases the flood hazard has caused structures to be used less efficiently than they would be with a project. No such underutilization of structures under existing conditions has been identified in the three reaches.

Table B5.1 - "Without Project" Conditions Exisitng and Future Flood
Damages by Decade With Affluence (January 1988 Price Levels
and 8.625 Percent Annual Interest Rate)

		: Base :	:	 :			:	:	
	Existing	Year :	:	:	;	:	:	:	
	: 1988	: 1995 :	2005 :	2015 :	2025	2035 :	2045 :	2095 :	AAE
REACH 2		; ; 	:	:	;		:	:	
TOTAL 2	•	· .	:	:	•		•	•	
Residential	:	:	:	:	:	:		:	
Structure		: 263.0:	263.0:	263.0:					
Content	: 86.8	: <u>94.9</u> :	106.2:	118.8:		148.7:			
Total	349.8	: 357.9:	369.2:	381.8:	395.9	411.7:	429.4:	429.4:	381.3
Commercial	. 97.2	: : 97.2:	97.2:	97.2:	97.2	97.2:	97 . 2:	97.2:	97.2
Public &	:	: :	;	:	:	:	:	:	
Other	: 38.9	: <u>38.9</u> :	<u>38.9</u> :	<u>38.9</u> :	38.9	_38.9:	38.9:	<u>38.9</u> :	38.9
Reach Total	: 485.9	: :	505 3.	517 0.	532 O	547 8	565 5.	: 565.5:	517 /
Reach Total	: 403.9	: 494.U: : :		31/•9.	332.0	. 547.6:	:	:	317.4
		:				· <u>·</u>		 :	
REACH 3	:	: :	:	:	:	:	:		
	:	: :	:	:	:	:	:	:	
Residential	: ;	::	•		20.5	• • •		• • • •	
Structure		: 30.5:	30.5:	30.5:					
Content	10.1 40.6	$\frac{11.0}{41.5}$:	$\frac{12.3}{42.8}$:	$\frac{13.8}{44.3}$:					$\frac{13.7}{44.2}$
Total	. 40.0	. 41.5:	42.0:	44.3:	43.9	4/./:	47.0	49.0	44.2
Commercial	: 251.1	: 251.1:	251.1:	251.1:	251.1:	251.1:	251.1:	251.1:	251.1
Public &	•	: :	:	:	:	:	:	:	
Other	: 113.4	: <u>113.4</u> :	<u>113.4</u> :	113.4:	113.4	113.4:	113.4	113.4:	113.4
Reach Total	: 405.1	:	407 3.		410 4		414 3.	: : 414.3:	40 8 7
keach lotal	: 405.1	: 400.0:	407.3:	400.0:	410,4	412.2:	414.5	. 414.3	400.
		: -		 :		:		:	
REACH 4	:	: :	:	:	;	:	:	:	
	:	: :	:	:	:	:	:	:	
Residential	:	::	:	:	10.0	:		:	10.0
Structure	: 10.2	: 10.2:	10.2:	10.2:					
Content Total	$\frac{3.4}{13.6}$	$\frac{3.7}{13.9}$:	$\frac{4.1}{14.3}$:	$\frac{4.6}{14.8}$:	<u>5.2</u> :				$\frac{4.6}{14.8}$
TOTAL	: 15.0	. 13.7.	14.5.	14.0.	15.4.	10.0.	10.0.	10.0.	14.0
Commercial	: 55.4	: 55.4:	55.4:	55 . 4:	55.4:	55.4:	55.4:	55.4:	55.4
Public &	•	: :	:	:	:	:		:	
Other	: 54.3	: _54.3:	<u>54.3</u> :	54.3:	54.3	54.3:	54.3:	54.3:	54.3
Reach Total	: : 123.3	: : 123.6:	124.0:	: 124.5:	125.1:	: 125.7:	126.3:	: : 126.3:	124.5
	<u>. </u>	: :	:	:		:			
Total								1	,050.6

Table B5.2 - "With Project" Conditions Exisitng and Future Flood
Damages by Decade With Affluence (January 1988 Price
Levels and 8.625 Percent Annual Interest Rate)

		: Base :			:	:	:	·:	
	: Existing		:	:	:	:	:	:	
	: 1988	: 1995 :	2005 :	2015 :	2025	2035 :	2045 :	2095 :	AAE
	:	: :	:	:	:	:	:	:	
Plan 10A: So	cenario l:	12-Foot	Gates .	- Resid	lual Dam	nages	:		
REACH 2	• •	· · ·	:	:	:	:	:	:	
	:	: :	:	:	: :	:	:	:	
Residential	:	:	:	;	: : : :	:	:	:	
Structure		: 185.6:		185.6:					185.6
Content	$: \underline{61.2}$: <u>66.9</u> :							83.4
Total	: <u>246.8</u>	: 252.5:	260.5:	269.4:	279.3	290.4:	302.9:	302.9:	269.0
Commercial	: : 68.6	: 68.6:	68.6:	68.6	68.6	68.6:	68.6:	68.6:	68.6
Public &	:	: 30.0.	:			:	:	:	****
Other	: 27.5	: 27 . 5:	27.5:	27.5	27.5	27.5:	27.5:	27.5:	27.5
;		::	:			:	:	:	
Reach Total	342.9	: 348.6:	356.6:	365.5:	375.4	386.5:	399.0:	399.0:	365.1
	•	· · ·	 :			•			
REACH 3	:	· .				•	:	:	
	:	: :	:		: :	:	:	:	
Residential	:	: :	:	:	: :	:		:	
Structure	20.6	: 20.6:	20.6:	20.6:	20.6	20.6:	20.6:	20.6:	20.6
Content	: 6.8	: 7.5:	8.3:			11.7:	13.1:	13.1:	9.3
Total	: 27.4	: 28.1:	28.9:	29.9:	31.0	32.3:	33.7:	33.7:	29.9
Commercial	: : 170.0	: : 170.0:	: 170 O:	170 0	170.0	: : 170.0:	: 170 . 0:	: 170.0:	170.0
Public &	. 170.0	. 1/0.0.	170.0.	170.0	. 17010.	. 1,0.0.	170.0.	1,0.0.	170.0
Other	: 76.7	: 76.7:	76.7:	76.7:	76.7	76.7:	76 . 7:	76.7:	76.7
other		·						:	
Reach Total	: 274.1	: 274.8:	275.6:	276.6:	277.7	279.0:	280.4:	280.4:	276.6
	:	<u>: :</u>	:		<u> </u>	:	 :	<u>:</u>	
DD A CH /	:	: :	:	:		:	:	:	
REACH 4	•								
Residential	• •	· .	:				•	:	
Structure		: 2.8:	2.8:	2.8:	2.8	2.8:	2.8:	2.8:	2.8
Content	: .9	: 1.0:							_
Total	: 3.7	: 3.8:				4.3:	4.5:		
	:	: :	:	: :	:	:	:	:	
Commercial	: 15.4	: 15.4:	15.4:	15.4	15.4	15.4:	15.4:	15.4:	15.4
Public &	:	: :	;	:	:	: ;	:	:	,
Other	: <u>15.1</u>	: <u>15.1</u> :	<u> 15.1</u> :	15.1	15.1	<u> 15.1</u> :	<u> 15.1</u> :	<u> 15.1</u> :	15.1
Reach Total	: 34.2	: 34.3:	34.4:	34.5	34.7	34.8:	35 . 0:	35.0:	34.5
		: :					:		
Total									676.2

Table B5.2 - "With Project" Conditions Exisitng and Future Flood
Damages by Decade With Affluence (January 1988 Price
Levels and 8.625 Percent Annual Interest Rate) (Cont'd)

		: Base :			:	:	:	:	
	Existing:	Year :	2005 :	2015 :	2025	2035	2045	2095 :	AAE
Plan 10A: So	: cenario 2:	22-Foot	Gates	- Resid	lual Dar	nages	:	:	
REACH 2		: :				:	; ;	:	
Residential :	•	: :	:	•	:		:	:	
Structure :		: 145.1:		145.1:					145.1
Content	: <u>47.8</u>	: <u>52.2</u> :							65.1
Total	192.9	: 197.3:	203.5:	210.5:	218.2	226.9:	236.6:	236.6:	210.2
Commercial :	53.5	53.5:	53.5	53.5	53.5	53.5	53.5	53.5:	53.5
Public & :	21.5	: <u>21.5</u> :	21.5	21.5	21.5:	21.5	21.5:	21.5:	21.5
Reach Total :	267.9	272.3:	278.5	285.5	293.2	301.9:	311.6:	311.6:	285.2
REACH 3								<u>:</u>	
TREACT 5	•	· · · :	:	•		•	:	:	
Residential:		: :	:	:	: :	:	:	:	
Structure :	: 15.4	: 15.4:	15.4:	15.4:	15.4	15.4:	15.4:	15.4:	
Content	: 5.1	: <u>5.6</u> :							
Total	20.5	: 21.0:	21.7:	22.4	23.3	24.2	25.2	25.2:	22.4
Commercial :	127.4	127.4:	127.4	127.4	127.4	127.4	127.4:	127.4:	127.4
Public & :	57.4	57.4:	57.4	57.4	57.4	57.4	<u>57.4</u> :	_57 . 4:	57.4
Reach Total	205.3	205.8:	206.5	207.2	208.1	209.0	210.0	210.0	207.2
REACH 4								:	
Residential :	•	. :	•		•		•	:	
Structure		0.0:	0.0	0.0	0.0	0.0	0.0:	0.0:	0.0
Content	0.0	: 0.0:							
Total	0.0	: 0.0:							
Commercial :	0.0	0.0:	0.0	0.0	0.0	0.0	0.0	0.0:	0.0
Public & :	0.0	0.0:	0.0	0.0	0.0	0.0:	0.0	0.0:	0.0
Reach Total	0.0	0.0	0.0	0.0	0.0			0.0:	0.0
Total	•	·				<u> </u>			492.4

Table B5.2 - "With Project" Conditions Exisitng and Future Flood
Damages by Decade With Affluence (January 1988 Price
Levels and 8.625 Percent Annual Interest Rate) (Cont'd)

		Base :	;	;	;	:	:	:	
	Existing	: Year : : 1995 :	2005 :	2015 :	2025 :	2035 :	2045 :	20 95 :	AAE
Plan 10A: S	: cenario 3:	: 30 - Foot	: Gates	- Resid	: lual Dan	: iages	;	:	
REACH 2	:	:	:	:	:	•	:	:	
Residential	: :	: :	:	•		:	:	:	
Structure	: 69.5	: 69.5:	69.5:	69.5:	69.5:	69.5:	69.5:	69.5:	69.5
Content	: 22.9	25.1:	28.0:	31.4:	35.1:				_31.2
Total	92.4	94.6:	97.5:	100.9:	104.6:	108.8:	113.4:	113.4:	100.7
Commercial Public &	: 25.7	25.7:	25.7:	25.7:	25.7	25.7:	25.7:	25.7:	25.7
Other	: 10.3	10.3:	10.3:	10.3	10.3	10.3	10.3	10.3:	10.3
Reach Total	: : 128.4	: : 130.6:	133.5:	136.9:	140.6	144.8:	149.4:	149.4:	136.7
	<u></u>	<u>:</u> :			:		:	:	
REACH 3	:	: :	:	:	:	:	:	:	
Residential	· :	· :	:	:	:	:		:	
Structure	: 8.6	: 8.6:	8.6:						8.6
Content	: 2.8	: <u>3.1</u> :	3.5:						3.
Total	: 11.4	: 11.7:	12.1:	12.5	12.9	13.5	14.0:	14.0:	12.4
Commercial Public &	: 70.8	70.8:	70.8	70.8	70.8	70.8	70.8	70.8:	70.
Other	: 31.9	: 31.9:	31.9	31.9	31.9	31.9	31.9	31.9:	31.
Reach Total	: : 114.1 :	: 114.4: : :		115.2	115.6	116.2	116.7	116.7:	115.
REACH 4	:	: :		:				:	
Residential	:	: :	:	; ;	: :	:	:	: : : :	
Structure	: 0.0	: 0.0:	0.0:	0.0	0.0	0.0	0.0	0.0:	0.
Content	: 0.0	: 0.0:	0.0	0.0	0.0			: _ 0.0:	0.
Total	: 0.0	: 0.0:	0.0	0.0	0.0	0.0	0.0		
Commercial Public &	: 0.0	0.0:	0.0		0.0	0.0	0.0	0.0	0.
Other	0.0	0.0		0.0				0.0:	0.
Reach Total	: 0.0	: 0.0:						0.0	0.
Total	·	<u>:</u> :				<u>:</u>	<u> </u>	·•	251.

e. NED Urban Inundation Benefits. Urban inundation reduction benefits accrue to the existing residential, commercial and public, and other activities located in the project area. Residential benefits include affluence applicable to residential contents.

Each Scenario's urban inundation reduction benefit is arrived at by subtracting from average annual urban inundation damages under "Without Project" conditions (Table B5.1) each Scenario's corresponding "With Project" residual urban inundation average annual damages (Table B5.2). This is performed in Table B5.3. Annual urban inundation benefits for Scenario 1: 12-foot gates, Scenario 2: 22-foot gates, and Scenario 3: 30-foot gates came to \$374,400, \$558,200, and \$798,800, respectively.

Table B5.3 - Average Annual Urban Inundation Reduction Benefits (1)

***************************************	. Ushana Davis		IVAL David		
	: Without Project		With Project	:	With Project
•	: Average Annual	:	Average Annual	:	Average Annual
	: Urban Inunda-	:	Urban Inunda-	:	Urban Inunda-
Damage Categories	: tion Damages	<u>:</u>	tion Damages	<u>:</u>	tion Benefits
D1 104 - C	- 1- 10 5 - 0 6 -	:		:	
Plan 10A: Scenario	o 1: 12-Foot Gates	. :		:	
. D	:	:		:	
Residential	: 6 202 300		A 200 000	:	A 0/ 700
Structure	: \$ 303,700 : \$ 136,600	:	\$ 209,000	:	\$ 94,700
Content		:	\$ <u>93,900</u>	:	\$ 42,700
Subtotal	: \$ 440,300		\$ 302,900	:	\$ 137,400
Commercial	: : \$ 403,700	:	\$ 254,000	:	0 1/0 700
		•			\$ 149,700
Public & Other	: \$ <u>206,600</u>	:	\$ <u>119,300</u>	:	\$ <u>87,300</u>
Makud	:	:	A (7(AAA	:	0 07/ /00
Total :	: \$ 1,050,600	:	\$ 676,200	:	\$ 374,400
Plan 10A: Scenario	: - 2. 22 East Catas	:		:	
Plan 10A: Scenario	o 2: 22-Foot Gates				
Residential	•	•		:	
	: . 6 303 700	•	¢ 160 500	:	0 1/2 200
Structure :	: \$ 303,700	:	\$ 160,500	:	\$ 143,200
Content :	: \$ <u>136,600</u>	:	\$ 72,100	:	\$ 64,500
Subtotal :	: \$ 440,300	:	\$ 232,600	:	\$ 207,700
: Commercial	: : \$ 403,700	:	\$ 180,900	:	e 222 900
Public & Other	-			•	\$ 222,800
Public & Other :	\$ <u>206,600</u>	:	\$ <u>78,900</u>	:	\$ <u>127,700</u>
T-4-1	: • \$ 1.050.600	:	¢ 400 400	:	A FED 200
Total :	: \$ 1,050,600	:	\$ 492,400	•	\$ 558,200
Diam 104. Commend	: - 3. 30 B 0	:		:	
Plan 10A: Scenario	3: 30-Foot Gates	_			
i Dooddamadal				:	
Residential :	. 6 202 700	;	6 70 100	:	A 005 (00
Structure :	: \$ 303,700	:	\$ 78,100	:	\$ 225,600
Content :	: \$ <u>136,600</u>	:	\$ 35,000	:	\$ 101,600
Subtotal :	: \$ 440,300	:	\$ 113,100	:	\$ 327,200
	. 6 /03 700	:	2 26 500	:	A 007 000
Commercial :	: \$ 403,700	:	\$ 96,500	:	\$ 307,200
Public & Other :	: \$ <u>206,600</u>	:	\$ 42,200	:	\$ 164,400
m . 1		:		:	
Total :	: \$ 1,050,600	:	\$ 251,800	:	\$ 798,800
:		:		:	

⁽¹⁾ Benefits are computed using January 1988 prices, an 8.625% annual interest rate, and a 100-year project life.

B6. AGRICULTURAL FLOOD DAMAGE REDUCTION

Plan 10A will reduce the flood hazard to agricultural areas downstream of the dam. This will result in agricultural benefits. Agricultural benefits were computed for Scenario 1 (12-foot gates), Scenario 2 (22-foot gates), and Scenario 3 (30-foot gates).

a. Methodology.

Agricultural information has been collected from local agricultural authorities. Local agricultural authorities include: SDS, ASCS, New York State College of Agriculture, Life Sciences at Cornell University, and the New York State Extension Service.

Crop budgets were derived for all crops affected. Crop budgets, along with crop yields, crop prices, and frequency of flooding were used to develop damages per acre by crop. These damages by crop were then used to obtain a weighted damage per composite acre.

This information was used to develop a stage-damage curve by individual reach. The stage damages reflect the secular growth in productivity that will take place during the evaluation period. This was input into a computer program that calculates damages (HEC-EAD). The output is expected annual damages. This process was used to determine "without project" and "with project" damages.

Information on intensified crop yields, crop prices, and intensified crop production costs were used to develop intensification benefits. A more detailed explanation of the agricultural benefit process follows.

b. General Agricultural Characteristics.

(1) Agricultural Land Use - Historical data on agricultural land use was obtained from a variety of sources. Agricultural land use patterns were obtained from 1966 New York State Land Utilization and Natural Resource (LUNR) Maps. The flood plain outline for the 1972 Tropical Storm Agnes was on quad sheets contained in the August 1973 Corps report "Report of Flood, Tropical Storm Agnes, 21-23 June 1972, Genesee River Basin." Mylars of the LUNR maps were placed over these report maps and the extent of the 1972 flood was transposed. All agricultural land in the 1972 flood area was planimetered. All agricultural parcels were visually checked against 1983 aerial photos of the same area. This resulted in agricultural land use in the flood plain area. Field trips to the flood plain indicated the LUNR maps do provide an effective means of identifying and quantifying the agricultural land use of the flood "Total Agricultural Acres" by reach, is presented in Table B6.1. "Total Agricultural Acres" includes "Inactive Acres." "Inactive Acres" were subtracted from "Total Agricultural Acres" to arrive at acres farmed under "Existing Conditions."

Table B6.1 - Existing Condition Agricultural Acres

Reach	:	Total Agricultural Acres	:	Inactive Acres	:	Existing Condition Acres
	:		:		:	
2 Chili	:	105	:	0	:	105
3 Avon	:	3,705	:	14	:	3,691
4 Geneseo	:	7,295	:	31	:	7,264
Total	:	11,105	:	45	:	11,060

- (2) Crop Yields, Flood Free Table B6.2 presents estimated crop yields for the principal crops grown on the flood plain. These yields are weighted average yields for the entire flood plain, given the yields and the spatial distribution of the principal soils of the flood plain. Flood-free yields are weighted average yields that could be obtained by flood-free farmers with elimination of the flood hazard. Intensified yields are weighted average yields that could be obtained by flood plain farmers with the elimination of the flood hazard and intensified management practices (additional production input). Flood-free yields have been used as the base to calculate agricultural inundation reduction benefits in accordance with Principles and Guidelines. Intensified yields only affect the calculation of future agricultural benefits under "with project" conditions.
- (3) <u>Crop Prices</u> In accordance with current guidance, prices used in this report are normalized prices (without the crop subsidy) provided by the Water Resources Council. The price for corn for silage is not surveyed by the WRC. This price is based upon an average price received by farmers in the study area. All prices are provided in Table B6.3.
- (4) Farm Schedules Monthly flood-free schedules of farm operations were developed for each of the principal crops grown on the flood plain. These schedules present data on variable production costs. A schedule was developed for each of the following crops: corn for grain; corn for silage; mixed hay, oats, winter wheat, soybeans; and miscellaneous crops. These costs were obtained through interviews of flood plain farmers and local agricultural authorities. A sample monthly schedule for corn for grain is presented in Table B6.4.

The monthly schedules of farm operations, show monthly crop loss with consideration given to profit from a catch crop. If a crop is not totally destroyed by a damaging flood, crop loss is a percent decline in yield as specified on the individual crop schedules. For crops totally destroyed by a damaging flood, crop loss is gross revenue minus unexpended variable production cost minus profit from the catch crop. Monthly crop loss values have been weighted by the percent distribution of historical floods recorded at the Alabama, New York, gate on Tonawanda Creek. The resulting monthly damages summed represent potential annual damage for an acre of each crop.

(5) Weighted Annual Damage per Composit Acre - Potential annual damages by crop were used to develop a weighted annual damage factor. Crop weights

reflect current cropping patterns in the lower reach. Crop weights were obtained from local agricultural authorities (ASCS in Livingston County and New York State Co-operative Extension). Table B6.5 summarizes the procedures used to obtain this weighted crop damage per composite acre value. Weighted crop damages per composite acre came to \$60.02.

Table B6.2 - Crop Yields or Principal Crops Grown on the Genesee River Flood Plain

Crop	:	Flood-Free	:	Intensi	fied
	:		:		
orn, Grain	:	100.0 Bu.	•	215.0	Bu.
orn, Silage	:	17.0 Tons	:	20.0	Tons
lixed Hay	:	3.5 Tons	:	4.0	Tons
Dats	:	80.0 Bu.	:	100.0	Bu.
inter Wheat	:	45.0 Bu.	:	55.0	Bu.
Soybeans	:	30.0 Bu.	:	35.0	Bu.
-	•		•		

⁽¹⁾ Weighted average yields reflect distribution of and crop yields for flood plain agricultural land in Erie, Niagara, and Genesee Counties based on predominate soil types on the flood plain in these counties.

Table 86.3 - Prices of Principal Crops Grown on the Genesee River Flood Plain

Crop		\$ Per Unit
	:	
Corn, Grain	:	1.91 per Bushel
Corn, Silage	:	15.67 per Ton
Mixed Hay	:	76.61 per Ton
Oats	:	.96 per Bushel
Winter Wheat	:	2.11 per Bushel
Soybeans	:	4.50 per Bushel
•	•	•

SOURCE: "Agricultural Price Standards, U.S. Water Resource Council, 24 July 1987 Reference Handbook", and "Field Crop Enterprise Budgets, 1987 Projections, Cornell University."

Table B6.5 - Weighted Annual Damage per Composite Acre

	:					Mixed						C 1	
	<u>:</u>	Grain	<u>:</u>	Silage	≟	нау	ᆣ	Uats	<u>:</u>	wnear	<u>:</u>	Soybeans	misc
	:	\$:	\$:	\$:	\$:	\$:	\$: \$
Annual Damage	:		:		:		:		:		:	:	:
per Acre	:	24.33	:	27.92	:	107.25	:	8.10	:	41.59	:	8.10	: 147.79
	:		:		:		:		:		;		:
Crop Dist.	:		:		:		:		:		:		:
by Percent	:	.5225	:	.0275	:	.05	:	.01	:	.09	:	.05	. 25
-	:	12.71	:	0.77	:	5.36	:	0.08	:	3.74	:		36.95
	:		:		:		:		:		:		:

Weighted Composite Annual Damage = \$60.02

Þ

	JA	N FEI	B KAI	RPR	KAY) Jun	300	AUA	SEP	DOT	KOV	150	TCTA
VARIABLE PRODUCTION	N COSTS												
GROWING COSTS	10.0	0 \$0.00	\$0.00	\$20.58	161.75	126.58	10.00	\$7.60	10.00	\$(.00	\$6.65	\$(,00	11(7.5
Harvesting costs	10.00	0 \$6,60			15.00					122.E4			
LAFOR COSTS	10.00	0 10.00	\$ (-), (-),	\$1.E0									\$17.5
HKTHLY EXPENDITURE	E \$0.00	\$0.00	\$0.00	122.36	167.14	122.56	10.00	\$6.00	10.00	12L.44	\$33,65	\$ E. L1	\$17E.00
CUMULATIVE COSTS	\$0.00	10.00	\$0.00	122.38	189.53	\$111.91	\$111.91	\$111.91	\$111.91	113E.34	1171.35	\$17E.00	
TOT. VAR FROD. COSTS				\$178.00	\$178.00	:17E.00	\$17E.00	1178.00	\$17E.00	117B.00	\$176.00	\$17E.00	\$17E.00
EXPENDED	\$0.00	\$0.00	10.00	122.58	189.53	\$111 91	1111 51	(111 01	C 111 P1	113E.34	C171 30	\$178.00	
UNEXFERDED	\$0.00	10.00								135.66			
POTENTIAL GROSS REVENUE-				\$151 AA	£191 fifi	c101 66	(151.66	(101.60	(121 60	61C1 A3	615: hh	1191.00	4161 V
PRICE ; YIELD PER ACRE	\$1.91 100			*******	*1:1:0V	*412.00	*171.00	11)1,00	1111,00	*171.00	*171.00	1.74.40	•47,•14
CROP LOSS	10.00	10.00	\$0.00	\$35.38 \$	102.53 (£172,81 (e171 D1	e192 61	e152 01	\$151.34	(22 22)	£15 10	
												•	
							******				720707		
(CROP SPECIFIC) -	:::::::			(\$6.10)(f15.25)(\$0.00			\$0.00	
		F CATCH :		0	Û	0 (544.30)	\$0.00	\$0.00					
(CROP SPECIFIC) - (REPLANTS DECLINE IN				0	Û	0 (544.30)	\$0.00	\$0.00	\$0.00	\$0.00	\$0.00	\$0.00	
(DROP SPECIFIC) (SEPLANTS DECLINE IN YIELD BY MONTH) (Apr.1-May.15-Je.30)	1		ORDP FRO	0 FIT (0,	O NO CATO	(\$44,30) 0 CH CROP I	SC.CO O SE PLANTS	\$0.00 0 ED.	\$0.00 0	\$0.60 p	\$0.00 p	sc.00	
(OROP SPECIFIC) (REPLANTS DECLINE IN YIELD BY MONTH) (Apr.1-May.15-Je.30) DJUSTED CROF 1088	1		ORDP FRO	0 FIT (0,	O NO CATO	(\$44,30) 0 CH CROP I	SC.CO O SE PLANTS	\$0.00 0 ED.	\$0.00 0	\$0.60 p	\$0.00 p	sc.00	
(CROP SPECIFIC) (REPLANTS DECLINE IN YIELD BY MONTH) (Apr.1-May.15-Je.30) DJUSTED CROF (CSS	\$0.00	\$0.00	ORDP PRO	0 FIT < 0,	0 NO DATO 102.53 €	(\$44,30) 0 CH CROP I	SC.CO O SE PLANTS	\$0.00 0 ED.	\$0.00 0	\$0.60 p	\$0.00 p	sc.00	
(CROP SPECIFIC) (REPLANTS DECLINE IN YIELD BY MONTH) (Apr.1-May.15-3e.30) DJUSTED CROF (CS) MARSE FACTOR (C) (NO. FLOOD EVENTS)	; \$0.00	\$0.00 18	0RBP FR6 \$0.00	0 FIT (0, I35.738 \$)	0 NO CATO 102.53 £	(\$44.30) 0 0H CROP I 124.91 \$	\$0.00 0 E PLANT!	\$0.00 0 ED.	\$0.00 0 \$124.55	\$0.00 p \$151.34	\$0.00 0	\$0.00	.5
(CROP SPECIFIC) (REPLANTS DECLINE IN YIELD BY MONTH) (Apr.1-May.15-Je.30) DJUSTED CROP (CSS	; \$0.00	\$0.00 18	0RBP FR6 \$0.00	0 FIT (0, I35.738 \$)	0 NO CATO 102.53 £	(\$44.30) 0 0H CROP I 124.91 \$	\$0.00 0 E PLANT!	\$0.00 0 ED.	\$0.00 0	\$0.00 p \$151.34	\$0.00 0	\$0.00	?\$ 2.000

c. Existing Agricultural Damages and Benefits.

Agricultural benefits can be classified as "Existing Condition Benefits" and "Future Condition" benefits. The former includes two distinct benefit classes: benefits resulting from the elimination of land loss due to streambank erosion and damages resulting from elimination of inundation damages. "Future Condition" benefits comes from intensification of pasture land.

(1) Elimination of Streambank Erosion Loss - Local agricultural authorities have identified streambank erosion as one of the most serious water resource problems in the Genesee River Basin. Streambank erosion is a highly visible problem.

A streambank erosion computer model for the main stem of the Genesee River was developed by the Buffalo District. This model predicted 22.2 acres would be lost annually.

This estimate includes land lost from the more severe and less frequent events such as Tropical Storm Agnes which occurred in June 1972. The model predicted 7.7 acres would be lost below Mount Morris annually. The remaining acres, 14.5, are lost above Mount Morris.

Plan 10A only affects erosion downstream of Mount Morris. The current market value of an acre of cropland on the affected flood plain was identified as \$650. The annual value of land lost below Mount Morris due to streambank erosion is \$5,000. It was assumed any scenario at Mount Morris (Scenario 1, Scenario 2, and Scenario 3) would eliminate 40 percent of the streambank erosion taking place below the dam. Streambank erosion benefits equals \$2,000.

- (2) Existing Agricultural Damages and Benefits Agricultural inundation damages presented in this report are based on the concept of agricultural loss as previously defined. A weighted annual agricultural loss value per composite acre of sown crop land was calculated (\$60.02). The weights were the percentage each crop is of total sown crop land in the basin. A stage area curve for agricultural land, which is defined as areas of sown crop land inundated from zero damage to SPF stage, was developed for the lower basin. These relationships were developed from the analysis of agriculture use patterns on the flood plain and by planimetering affected areas on USGS topographic maps for the 1972 flood, and bank-full stage. The above information was used to develop a stage damage curve by reach. Table B6.6 presents sample stage-damage inputs for Reach 4, for "Existing Conditions".
- (a) Projected Growth of Agricultural Inundation Damages. The value of agricultural output on the flood plain has been rising, and it is projected to continue to rise over time. This represents a regional component of a national secular trend of rising agricultural productivity and output in the future. This secular rise is acknowledged by Cornell University School of Agriculture and by the U.S. Water Resources Council OBERS projections, Regional Economic Activity in the United States.

The "without project" agricultural damages, as well as the "Residual Damages" have been estimated to grow at a rate of 2 percent per year from 1987-1995,

in accordance with Cornell University estimates. Both "without" and "with project" damages were allowed to grow at 2 percent annually for the period 1988 to 1995. This information was used in conjunction with stage area curves to develop stage damage inputs by reach. The HEC-EAD computer program was used to convert these damages to equivalent average annual damages (see Table B6.6).

Table B6.6 - Stage Damage Inputs for Reach 4

REACH	4 BENESEO							SECUL AR
. •			\$	CROP	DTHER	TOTAL	SECULAR	GROWTH &
- NUMBER			DAMAGES	DAMAGES	AGRICULT	AGRICULT	GROWTH	AGRICULT
DF POINTS	STAGE	ACRES	PER ACRE	PER ACRE	DAMAGES	DAMAGES	FACTOR	DAMABES
1	551.21	0	\$60.02	\$0.00	\$0.00	\$0.00	1.1717	\$0.00
2	551.21	0	\$60.02	\$0.00	\$0.00	\$0.00	1.1717	\$0.00
2	551.84	0	\$60.02	\$0.00	\$0.00	\$0.00	1.1717	\$5.00
4	552.69	0	\$60.02	\$6.00	\$0.00	\$0.00	1.1717	\$6.00
5	554.62	0	\$60.02	\$0.00	\$0.00	\$0.00	1.1717	\$0.00
Ь	557.16	0	\$60.02	\$0.00	\$0.00	\$0.00	1.1717	\$0.60
7	557.50 1	0	\$60.02	\$0.00	10.00	\$0.00	1.1717	\$0.00
8	558.87	6100	\$60.02	\$366,122.00	\$54,91E.30	\$421,040.30	1.1717	\$493,315.E1
9	560.88	6750	\$60.02	\$405,135.00	•	\$465,905.25		\$545,882.2£
10	562.37	7050		\$423,141.00	•	\$486,612.15		\$570,143.65
11	563.88	7250		\$435,145.00	•	1500,416.75		\$596,317.9E
12	564.50	7264		\$435,925.28	•	\$501,383.67		\$587,450.16
13	566.00 1	7264		\$435,985.28	•	\$501,3E3.07		\$587,450.18
14	568.00	7264		\$435,985.28		\$501,3ET G7		\$567,450.1B

*-Actual points on the curve.

Other Agricultural Damages-Crop Damages times 15%. This calculation starts 1 foot after bank full. This allows for damages to farm buildings, machinery, farm roads, etc.

Total Agricultural Damages- "Crop Damages" and "Other Agricultural Damages".

Secular Growth- Assumed to be 2% per year. Growth takes place from study year 1988 to the first year of the Project, 1995.

(b) Average Annual Agricultural Damages, Existing Conditions. Approximate agricultural damages for existing conditions by reach were calculated by multiplying the area of affected crop land at each stage, by the weighted annual loss value per composite acre of sown crop land in the Lower Basin. An example of this calculation is presented in Table B6.6 for Reach 4. "Other Agricultural Damages", which include damages to fences, to farm access roads, and to machinery and equipment have been estimated to be 15 percent of crop damage beginning at 1 foot above the zero damage stage. "Other Agricultural Damages" are included in the agricultural stage-damage curves. Table B6.6 also shows how secular growth in productivity was included in the stage-damage curve. The resulting products were inputs into the Expected Average Annual Damage program from the Hydrologic Engineering Center (HEC) to estimate "approximate" damage-frequency relationships for each reach.

Existing average annual damages, by reach, are presented in Table 86.7. These damages come to \$302,000. The damages, calculated at January 1988 prices, assume a 100-year project life, and an 8-5/8 percent annual interest rate.

The agricultural damages produced by this process are termed "approximate" because they underestimate agricultural damage. This is because the stage-frequency curves are based on annual peaks. Field inspection indicates that these curves understate frequency of growing season agricultural damage.

- (c) Average Annual Agricultural Damages, Improved Conditions. Residual average annual equivalent damages have been computed for each of the three scenarios using improved-condition stage-frequency curves. Table B6.7 presents the residual equivalent average annual agricultural inundation damages for each scenario in January 1988 prices and estimated 1988 agricultural land use. These damages, for Scenario 1, Scenario 2, and Scenario 3, came to \$222,800, \$181,300, and \$117,100. These damages are at January 1988 prices, and assume a 100-year project life, and an 8-5/8 percent annual interest rate.
- (d) Average Annual Equivalent Agricultural Benefits, Existing Conditions. Existing equivalent average annual agricultural benefits (agricultural inundation reduction benefits) for the three scenarios are summarized in Table B6.7. Average annual agricultural inundation benefits for Scenario 1 (12-foot gates), Scenario 2 (22-foot gates), and Scenario 3 (30-foot gates) came to \$79,200, \$120,700, and \$184,900.

d. Future Agricultural Land Use and Benefits.

Intensification - Agricultural intensification benefits take into consideration the change in agricultural damages that would result from shifting to a higher land use under "with project" conditions. Shifting of land currently used for pasture to crop production would fall under this category.

(1) <u>Future Land Use</u> - Analysis of cropping patterns in the Lower Basin under existing flooding conditions indicates that land subject to flooding once in 3 years or more frequently, currently is used as pasture. Land which is subject to flooding less frequently is used for high-value field and vegetable crops.

Flood plain farm owners, farm operators, and related regional agricultural authorities (Soil Conservation Service personnel, local conservation district personnel, and New York State Agricultural Extension Service field crop specialists) indicate this land use pattern will be maintained under "with project" conditions.

Future land use patterns have been estimated using the reduction in flood frequency as well as information collected from the above named agricultural authorities. Table B6.8 presents a reach-by-reach illustration of total acres in pasture and acres of pasture shifted to higher use.

It has been projected that 87 percent of flood-free (existing condition) pasture (425 acres) will be shifted to a higher use under "with project" (improved) conditions for any of the three scenarios evaluated. This shift in

Table 86.7 - Existing Agricultural Benefits by Reach

			Scenari	o 1 -	12-Foot Gates	3	
		:	Existing	:	Residual	:	Benefits
F	Reach	:	Damages	:	Damages	:	1988 Prices
		:	\$:	\$:	\$
		:		:		:	
2	Chili	:	3,000	:	2,200	:	800
3	Avon	:	228,300	;	187,900	:	40,400
4	Geneseo	:	70,700	:	32,700	:	38,000
Tot	tal	:	302,000	:	222,800	:	79,200
		:		:		:	

		Scenari	LO 2 -	22-Foot Gates	<u> </u>	
	:	Existing	:	Residual	:	Benefits
Reach	:	Damages	:	Damages	:	1988 Prices
	:	\$:	\$:	\$
	:		:		:	
2 Chili	:	3,000	:	1,800	:	1,200
3 Avon	:	228,300	:	166,700	:	61,600
4 Geneseo	:	70,700	:	12,800	:	57,900
	:		;		:	
Total	:	302,000	:	181,300	:	120,700
	:	•	:	-	:	•

		Scenari	to 3 -	30-Foot Gates	<u> </u>	
	:	Existing	:	Residual	:	Benefits
Reach	:	Damages	:	Damages	:	1988 Prices
	:	\$:	\$:	\$
	:		:		:	
2 Chili	:	3,000	:	1,000	:	2,000
3 Avon	:	228,300	:	112,100	:	116,200
4 Geneseo	:	70,700	:	4,000	:	66,700
	:		:		:	\
Total	:	302,000	:	117,100	:	184,900
	:	•	:		:	

land use is estimated to occur at a linear rate over the 10-year period, 1995 through 2005. Thus, 42.5 acres would be shifted yearly into a higher use, for the first 10 years of the evaluation period.

Table B6.8 - "With Project" Condition Intensified Acres

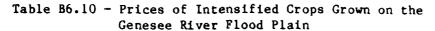
Reach		Acres of Existing Pasture	:	Pasture Areas Shifted
	:		:	
2 Chili	:	0	:	0
3 Avon	:	187	:	137
4 Geneseo	:	298	:	288
	:		:	
Total	:	485	:	425
	:		:	

(2) Crop Yields, Intensified - Table B6.9 presents estimated crop yields for the principal intensified crops grown on the flood plain. These crops are corn silage and hay. Intensified yields are weighted average yields that could be obtained by flood plain farmers with the elimination of the flood hazard and intensified management practices (additional production inputs). Intensified yields affect the calculation of future agricultural benefits under "with project" conditions.

Table B6.9 - Crop Yields for Intensified Crops Grown on the Genesee River Flood Plain

Стор	: Average Yield per Acre (Units/Acre) : Intensified (1)
Corn, Silage	: 20.0 tons
Mixed Hay	: : 4.0 tons

- (1) Weighted average intensified yields reflect, distribution of and crop yields for, flood plain intensified agricultural land in Erie, Niagara, and Genesee Counties, based on predominate soil types on the flood plain in these counties.
- (3) Crop Prices In accordance with current guidance, prices used in this report are normalized prices (without the crop subsidy) provided by the Water Resources Council. The price for corn for silage is not surveys by the WRC. This price is based upon an average price received by farmers in the study area. All prices are provided in Table B6-10. Crop prices do not change even though the crops are intensified. Production costs increase and yields increase.



Crop	:	1988 Prices
Corn, Silage	:	\$15.67 per ton
Mixed Hay	: :	\$76.61 per ton
	<u> </u>	

(4) Farm Schedules (Budgets), Intensified - Based on information from the above named agricultural authorities, land projected to shift upward in use from pasture to high value crops has been projected to be managed more intensively under "with project" (improved) conditions. Additional drainage measures, fertilizer, herbicides, and seed expenditures will result in higher production costs. Table B6.11 presents the calculation of these higher production costs for corn silage. A similar table was developed for mixed hay.

These increased costs were used in conjunction with the rise in output to develop intensified crop budgets. Like the upward shift in land use, the intensified management practices are projected to be implemented over the 10-year period (1995-2005) under "with project" conditions.

Monthly schedules of intensified operation budgets (Table B6.11 for corn silage) presents data on gross revenue, total variable production costs and crop loss for each of the principal crops grown on the flood plain which are subject to intensified management - corn for silage and mixed hay. All other crops have not been intensified.

(5) Weighted Annual Benefit per Composite Acre for Intensified Crops - Benefits attributable to intensified crop land is defined as "Gross Revenue per Acre" minus "Total Variable Production Cost". These two inputs were obtained from the intensified crop budgets. Intensified crop budgets were developed for corn grain, corn silage, winter wheat, and hay. There was no shift into corn grain since this crop already accounts for 52.25 percent of existing cropped land. Winter wheat had negative net profits per acre of -\$.38 using unsubsidized crop prices. Based upon conversations with local agricultural crop specialists, all intensified acres were shifted to corn silage and hay. There would be equal acres of both planted.

All acres shifted were previously used as pasture. There is some net profit foregone from using this pasture land for intensified crops. This net profit foregone was given a value of \$50.00 per acre. The profit foregone on the pasture land was subtracted from the weighted annual net profit per composite intensified acre. This resulted in an adjusted net profit per composite intensified acre of \$112.07 (Table B6.12).

Table B6-11 Crop Budget And Damage Factors For Corn Silage-Intensified

	JAN	, 128	r.a.i	r aps	k KAY	. Juk	11 1	aua	SEP	DIT	VCA	DEC	LATOT
VARIABLE PRODUCTION	C0515												
GROWING COSTS	10,00	10.00	10.00	119.95	159.85	\$19.95				\$6.00			199.75
HARVESTING COSTS	10.60	\$0.00	10.00	\$6.00	\$0.00	10.00				\$11.77		10.00	
LABOR COSTS	\$6.50	10.00	10.00	12.93	15.72	12.51	10.00	10.00	17.26	17.26	10,00	10.00	\$25.05 \$6.00
MICHLY EXFERDITURE	\$0.00	\$6.00	\$0.00	122.66	\$ \$5E.57		10.00	10.00	\$15.65	\$15.63	10,00	\$6.66	1112.03
COMPLYINE COSIS	10.00	10.00	10.00	122.86	191.42	1114.28	1314.26	1114.28	1155.30	\$152.55	\$6.60	\$6.00	
TOT. VAR PROD. COSTS				152.33	152.33	152.53	152.53	152.33	152.33	152.53	0	o	152.23
******	44.44		48.66	400 51		26	**** 20	/11/ DB		\$152.33	ፋስ ሰስ	\$6.60	
	10.00	\$0.00	10.00	12.00	171.72	1114.28	\$114.28 470.67	1114.20	70.016	10.00	16.00	50.00	
UNEXPERDED	30.00	\$0.00	. 30.00	1124.48	- 180.41	108.05	1,5.08	135.08	141.64	,,,,,,	,,,,,		
BRDSS REVENUE-				\$313.40	1015.40	\$313.40	1313.40	\$313.40	\$313.40	\$313.40	\$0.00	\$0.00	1313.40
PRICE- S	\$15.67	(1)						• •					
YIELD PER ACRE	20												
DRAINAGE COSTS	_			\$4.58	\$4.53	24.58	\$4.58	\$4.58	\$4.58	\$4.58	** **	10.00	302.82
TOTAL BROSS REVENU	•			\$308.82	\$308.EZ	\$305.52	\$309.62	1302.62	\$308.62	\$308.E2	30.00		000100
CROP LOSS	\$6.00	\$0.00	10.00	\$179.34	\$247.91	1270.77	\$270.77	\$270.77	\$209.79	\$308.92	\$0.00	\$0.00	
CATCH CROP PROFIT (CROP SPECIFIC) (REPLANTS PECLINE IN YIELD BY MONTH)	i			¥125.61	1110.17	\$83.84	£0.00	_\$0.00	£0.00	\$0.00	\$0.00	\$0.00	
(Apr.1-May.15-Ja.30)													
ADJUSTED CROP LOSS	:0.00	10.00	10.00	\$53.74	£137.74	\$20£.92	\$270.77	\$170.77	1287.73	\$308.82	\$0.00	\$0.00	
DAMAGE FACTOR (I)													
(NO. FLOOD EVENTS)	?	19	27	20	2	2	ũ	1	2	0	. 4	10	95
(TONAWANDA DKLY)	0.074	0.127	0.224	0.211	0.032	0.021	0.000	0.011	0.032	- 6,000	0.642	0.105	1.000
Pamages	10.00	10.00	\$0.00	\$11.71	F4.05	\$4.35	\$6.00	12.85	\$9.15	\$0.00	\$0.00	10.00	\$72.02
	1). Free	: Table :	? Date .	11. 'Fis	id Ores	Enterori	is luis	ete', Co	rnell Ur	ivensity	Aşrisul	tural	

^{(1).} Free Table 2 page 11, "Field Orop Enterprise Budgets", Cornell University Apricultural Experisent Station, Inhate Hew York, Merch 1987.



	:	Gross	:	Total	:		:		: 1	leighted Annual
	:	Revenue	:	Variable	:		:	Percent	:	Net Profit
	:	per	:	Production	:	Net	:	of Land	:	per Composite
Crop	:	Acre	:	Cost	:	Profit	:	by Crop	: I1	ntensified Acre
	:	\$:	\$:	\$:	(percent)	:	\$
Corn, Silage	:	308.82	:	152.33	:	156.49	:	50	:	78.25
Hay	:	301.86	:	134.23	:	167.64	:	50	:	83.82
	:		:		:		:		:	162.07
Net Profit fo	or	Pasture	La	nd					:	
Weighted Anni	ua	l Benefit	: p	er Composite	e 1	intensifi	ed	Acre	:	50.00
									:	112.07
									:	

(6) Future Agricultural Benefits, Intensification - Future agricultural benefits attributed to any of the three scenarios (Scenario 1, Scenario 2, and Scenario 3) are increased profits less the increase in income foregone attributed to the shift in land use and intensified farm management. Table B6.13 presents the increase in gross profit and the resulting benefit attributed to the project in the future. The increase in gross profit has been estimated to grow at a rate of 2 percent per year for the period 1995-2005, in accordance with the trend of rising agricultural output and productivity previously documented (see "Projected Growth of Agricultural Inundation Damages"). These benefits, as presented in Table B6.13, are projected to accrue at a linear rate over the 10-year period 1995 to 2005 (project year 1 to project year 10).

These benefits are discounted at the project interest rate of 8.625 percent per annum. The discounted average annual benefits at January 1988 price levels amount to \$37,800. Intensification benefits for Reaches 3 and 4 came to \$12,200 and \$25,600, respectively.

Table B6.13 - Time Stream of Intensification Benefits

	:		:		:		:		:	Total	:	Cumulative
	:		:		:		:	Secular	:	Intensifi-	:	Intensifi-
	:		: (Cumulative	2:	Weighted	:	Growth	:	cation	:	cation
Projec	t:	Acres	:	Acres	:	Net Profit	:	Compound	:	Benefits	:	Benefits
Year	: 5	Shifted	:	Shifted	:	per Acre	:	Factor	:	per Year	:	per Year
	: ((acres)	;	(acres)	:	\$:		:	\$:	\$
1	:	42.5	:	42.5	:	112.07	:	1.02	:	4,858	:	4,858
2	:	42.5	:	85.0	:	112.07	:	1.0404	:	4,955	:	9,813
3	:	42.5	;	127.5	:	112.07	:	1.0612	:	5,054	:	14,867
4	:	42.5	:	170.0	:	112.07	:	1.0824	:	5,155	:	20,022
5	:	42.5	;	212.5	:	112.07	:	1.1041	:	5,258	:	25,280
6	:	42.5	;	255.0	:	112.07	:	1.1262	:	5,363	:	30,643
7	:	42.5	:	297.5	:	112.07	:	1.1487	:	5,471	:	36,114
8	:	42.5	:	340.0	:	112.07	:	1.1717	:	5,580	:	41,694
9	:	42.5	:	382.5	:	112.07	:	1.1951	:	5,692	:	47,386
10	:	42.5	:	425.0	:	112.07	:	1.2190	:	5,806	:	53,192
11	:	0	:	425.0	:		:		:	•	:	53,192
100	:	0	:	425.0	:		:		:		:	53,192
	:		:		:		;		:		:	•

e. Summary of Agricultural Benefits.

Table B6.14 summarizes estimated average annual agricultural benefits by scenario. The three agricultural benefit categories are Erosion Benefits, Inundation Reduction Benefits, and Intensification Benefits. Total agricultural benefits attributable to Scenario 1 (12-foot gates), Scenario 2 (22-foot gates), and Scenario 3 (30-foot gates) came to \$119,000, \$160,500, and \$224,700, respectively. These benefits are in January 1988 prices, assume a 100-year project life, and an 8-5/8 percent annual discount rate.

	(12-Foot Gates)	:	Scenario 2 (22-Foot Gates)	•	Scenario 3
÷	(12-root dates)	 -	(22-FOOL Gates)	<u> </u>	(30-Foot Gates)
•	ş	•	Ş	:	\$
:		:		:	
:	2,000	:	2,000	:	2,000
:		:		:	
:		:		:	
:	79,200	:	120,700	:	184,900
:	•	:	•	:	
:		:		:	
:	37,800	:	37.800	:	37,800
•		•		•	3,,000
•	119,000	•	160, 500	•	224,700
		; \$; 2,000 ; ; 79,200 ; ; 37,800 ; 119,000	.: 79,200 : .:	: 79,200 : 120,700 : : : : : : : : : : : : : : : : : :	: 79,200 : 120,700 : : : : : : : : : : : : : : : : : :

Table B6.14 - Agricultural Benefits by Scenario

B7. SUMMARY OF BENEFITS

a. Introduction.

Table B7.1 summarizes average annual benefits for the three scenarios evaluated. There are two major benefit categories: Urban Inundation Benefits and Agricultural Benefits. Scenario 3 had average annual benefits of \$1,023,500. Scenario 2 had average annual benefits of \$718,700. Scenario 1 had average annual benefits of \$493,400.

Benefits developed in this section reflect conditions of development over the evaluation period (1995-2005), January 1988 price levels, an 8.625 percent annual interest rate, and a 100-year project life. The components of two major benefit categories follows.

b. Urban Inundation Benefits.

The majority of the benefits derived from the three scenarios are primarily inundation reduction benefits. These benefits accrue to the existing residential, commercial and public, and other activities located in the project area. Residential benefit include affluence applicable to residential contents. Other benefit categories analyzed were Floodproofing Costs Avoided, National Flood Insurance Costs Avoided, and Area Redevelopment Benefits.

- (1) Inundation Reduction Benefits Section B4 evaluated future flood plain land use and concluded no future change in land use is expected. Land use is the same "without" and "with" the project. Each scenario's urban inundation reduction benefit is arrived at by subtracting from average annual urban inundation damages under "without project" conditions (Table B5.1); each scenario's corresponding "with project" residual urban inundation average annual damages (Table B5.2). This is performed in Table B5.3. Annual urban inundation benefits for Scenarios 1, 2, and 3 came to \$374,400, \$558,200, and \$798,800, respectively.
- (2) <u>Floodproofing Costs Avoided</u> There are no floodproofing costs avoided benefits associated with any of the scenarios evaluated.
- (3) National Flood Insurance Administration Costs Avoided The national cost of the flood insurance program is its administration costs. Savings of flood insurance administration costs can be credited to the NED benefit account when a project eliminates inundation of an area due to floods having an exceedence frequency of 1 percent of less. As outlined in Section B5d(2) (National Flood Insurance Costs), none of the scenarios would cause a reduction in administration costs.
- (4) Area Redevelopment Benefits The applicability of area redevelopment benefits was investigated. In labor market areas which have been designated as redevelopment areas, current planning guidance directs that project benefits shall be increased by the value of unemployed or underemployed local labor required to implement the project. Otherwise, it is assumed, such labor would not be used or would be underutilized.

In order to achieve designation as a redevelopment area, a region must meet the established criteria for substantial and persistent unemployment. None of the counties met this criteria. There are no area redevelopment benefits associated with any of the scenarios.

c. Agricultural Benefits.

There are three agricultural benefit categories: Agricultural Erosion Benefits, Agricultural Inundation Reduction Benefits, and Agricultural Intensification Benefits. Total agricultural benefits for Scenarios 1, 2, and 3 came to \$119,000, \$160,500, and \$224,700, respectively.

Table B7.1 - Summary of Benefits by Scenario (1)

Benefit Categories	:	Scenario l	:	Scenario 2	:	Scenario 3
	:	\$:	\$:	\$
Flood Damage Reduction	:		:		:	
	:		:		:	
Residential	:		:		:	
	3		:		:	
Structures	:	94,700	:	143,200	:	225,600
Contents	:	42,700	:	64,500	:	101,600
	:		:		:	
Subtotal	:	137,400	:	207,700	:	327,200
	:		:		:	
Commercial	:	149,700	:	222,800	:	307,200
	:		:		;	
Public and Other	:	87,300	:	127,700	:	164,400
	:		:		:	
Subtotal	:	374,400	:	558,200	:	798,800
	:		:		:	
Agricultural	:		:		:	
	:		:		:	
Erosion	:	2,000	:	2,000	:	2,000
Inundation	:	79,200	:	120,700	:	184 ,9 00
Intensification	:	37,800	:	37,800	:	37,800
	:		:	\	:	
Subtotal	:	119,000	:	160,500	:	224,700
	:		:		;	
Total Benefits	:	493,400	:	718,700	:	1,023,500
	:		:		:	

⁽¹⁾ Benefits are computed using January 1988 prices, an 8.625 percent annual interest rate, and a 100-year project life.

- (1) Erosion Benefits Each scenario would eliminate some of the erosion of agricultural land downstream of Mount Morris. Given 7.7 acres of agricultural land are lost to erosion a year, an average value of \$650 per acre, and a 40 percent decrease in erosion below Mount Morris from implementation of any of the three scenarios, erosion benefits came to \$2,000 annually.
- (2) Agricultural Inundation Benefits Each scenario's agricultural inundation reduction benefit is arrived at by subtracting from average annual agricultural inundation damages under "without project" conditions, each scenario's corresponding "with project" residual agricultural inundation average annual damages. This is performed in Table B6.7.

Stage-damage curves were developed for agriculture. Crop budgets, crop yields, crop prices, and frequency of flooding were used to develop a weighted damage per composite acre. This was used in conjunction with acres affected to develop agricultural stage-damage relationships. The stage-damage curves reflect subsidy-free crop prices and the secular growth in agricultural productivity. This was input into a computer program (HEC-EAD) that was used to develop "without" and "with project" condition agricultural damages.

Agricultural inundation reduction benefits for Scenarios 1, 2, and 3 came to \$79,200, \$120,700, and \$184,900, respectively.

(3) Intensification Benefits - Currently, land subject to flooding once in 3 years, or more frequently, is used as pasture. Under "with project" conditions, a portion of this pasture land (87 percent) will be shifted to a higher use (corn silage and hay). Benefits for such a shift are increased profits less the increase in income foregone attributed to the shift in land use and intensified farm management.

A total of 425 acres were shifted under any of the three scenarios. This shift in land use was assumed to be linear and take place over a 10-year period, starting in 1995.

Table 86.13 outlines the time stream of these benefits. Increased profits per composite acre were developed using intensified crop yields, intensified crop production costs, and net profit foregone from the previous use (pasture). This time stream of agricultural intensification benefits was converted to an average annual value using an 8.625 percent annual interest rate and a 100-year project life. Agricultural intensification benefits for Scenarios 1, 2, and 3 were the same - \$37,800.

B8. SUMMARY OF AVERAGE ANNUAL COSTS

° Introduction.

Table B8.1 presents a summary of average annual costs for each of the three scenarios. Annual charges were based on an 8.625 percent annual interest rate and a 100-year project life. Annual maintenance costs applicable to each scenario are also included. Interest during construction was computed based on monthly compoundings.

B9. ECONOMIC EFFICIENCY

° Introduction.

Net discounted benefits and B/C ratio are the two methods of economic efficiency used to determine the economic justification of the project alternatives. Table B9.1 summarizes average annual benefits, average annual costs, net benefits, and benefit-cost ratios for the three scenarios evaluated. None of the scenarios had a B/C ratio above 1. Scenario 1 (12-foot gates) had average annual benefits of \$493,400, average annual costs of \$942,800, average annual net negative benefits of \$449,400, and a B/C ratio of .52. Scenario 2 (22-foot gates) had average annual benefits of \$718,800, average annual costs of \$1,127,600, average annual net negative benefits of \$408,800, and a B/C ratio of .64. Scenario 3 (30-foot gates) had average annual benefits of \$1,023,500, average annual costs of \$1,226,100, average annual net negative benefits of \$242,600, and a B/C ratio of .81.

Table B8.1 - Investment Costs and Annual Charges (January 1988 Price Levels, 8.625 Percent Annual Interest Rate)

Scenario 1 (12	-Foot Gates)	
	: \$	
Total Investment for the Project	:	
	:	
Total Project Cost Excluding Lands	: 10,100,000	
Interest During Construction	: 712,700	
Lands and Damages	:0	
Total Investment Including Lands	: 10,812,700	
Annual Charges for the Project	· :	
Interest	: 932,600	
Amortization	: 200	
Maintenance	10,000	
Total Annual Charges	$\frac{942,800}{}$	
Scenario 2 (22		
	: \$	
Total Investment for the Project	:	
	:	
Total Project Cost Excluding Lands	: 12,100,000	
Interest During Construction	: 853,800	
Lands and Damages	12.052.000	
Total Investment Including Lands	: 12,953,800 :	
Annual Charges for the Project	:	
Interest	· : 1,117,300	
Amortization	: 300	
Maintenance	: 10,000	
Total Annual Charges	$=\frac{1,127,600}{1,127,600}$	
	:	
Scenario 3 (30		
Total Investment for the Desiret	\$	
Total Investment for the Project	• •	
Total Project Cost Freduction Lands	• 13 600 000	
Total Project Cost Excluding Lands Interest During Construction	: 13,600,000 : 959,770	
Lands and Damages	. 7,77,770	
Total Investment Including Lands	14,559,700	
Total Investment Including Lands	: 14,339,700	
Annual Charges for the Project	:	
Tatanaat	1 255 900	
Interest	: 1,255,800	
Amortization	: 300	
Maintenance	10,000	
Total Annual Charges	: 1,266,100	

Table B9.1 - Benefit/Cost Analysis (1)

	:			Scenario 2		
	:	(12-Foot Gates	:	(22-Foot Gate	es):	(30-Foot Gates)
	:	\$:	\$:	\$
Average Annual Benefits	:	493,400	:	718,800	:	1,023,500
Average Annual Costs	;	942,800	:	1,127,600	:	1,266,100
Benefit-Cost Ratio	:	.52	:	.64	:	.81
New Benefits	:	-449,400	:	-408,800	:	-242,600
	;		:		:	

⁽¹⁾ Benefits and costs are computed using January 1988 prices, an 8.625 percent annual interest rate, and a 100-year project life.

APPENDIX C DESIGN

GENESEE RIVER STUDY

APPENDIX C DESIGN

GENESEE RIVER STUDY

APPENDIX C DESIGN

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С3	Cost Estimate for 32' High Gate Alternative	
	PLATES	
NUMBER	TITLE	
C1	Modification to Mt. Morris Dam Alternative - Elevation	Upstream
C2	Modification to Mt. Morris Dam Alternative ~ :	South

GENESEE RIVER STUDY

APPENDIX C DESIGN

Cl. GENERAL

This appendix gives the rationale and assumptions used to size and estimate costs of the considered installation of tainter gates on Mount Morris Dam. The proposed structures were sized and estimated using similar Corps of Engineers gated dams as a guide. Costs were estimated for three alternative gate heights of 12, 22, and 32 feet. Height of the gates are measured from the contact point on the spillway to the top of the skinplate.

C2. DESCRIPTION OF CONSIDERED STRUCTURES

Ten steel tainter gates approximately 42 feet long would be installed on the crest of the spillway. The tainter gates and their operating machinery would be installed on concrete piers approximately 8 feet wide. The eleven piers would be constructed on the centerline of each overflow monolith. The piers would also support a service bridge, used to provide access to the tainter gates and their operating machinery for maintenance. Plates Cl and C2 show details of the considered structures. These plates are intended to show only general details of the considered structures and are not to be scaled.

C3. STABILITY OF DAM

A recent report prepared by the Buffalo District presented overturning and sliding stability analyses for the existing dam for various flood and earthquake loading conditions. The report found the dam to be very stable and to meet all the required stability criteria. An overturning stability analysis of the dam with a 32-foot high tainter gate and upper pool to the top of the gate was performed, and the base was found to be in 100 percent compression. The recent stability report found the existing dam to have high safety factors against sliding; therefore, it was assumed that a gated dam would meet all sliding stability requirements.

C4. COST ESTIMATE

Costs were estimated at \$10,000,000; \$12,100,000; and \$14,000,000 for the 12, 22, and 32-foot high gate alternatives, respectively. Cost breakdowns for each alternative are included in this appendix in Tables Cl, C2, and C3.

Table Cl - Cost Estimate for 12' High Gate Alternative

PRELIMINARY GOVERNMENT ESTIMATE	JANUARY 'PP'					
MT. MORRIS DAM TAINTER SATES INVITATION NO. DESCRIPTION SETIMATE UNIT ESTIMATED NO. COMMITTY FRICE AMOUNT		PRELIMINARY GOVERNMENT ESTIMATE				
### MT. MORRIS DAM TAINTER SATES ITEM:	FROJE	CT GENESEE RIVER BASIN STUI	7Y			
NO. (QUANTITY: FRICE AMOUNT	·					TINVITATION NO.
NO. (QUANTITY: FRICE AMOUNT	TTEMT	T FSTIMATED				
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b) Extension of Non-overflow: 1,000 C.Y. 600.00 600.000 Section C) Filling Upper Gallery &	2	a)Concr. Pier. Reinf. &	 6,500		: : 600.00	1 1 0,900,000
c)Filling Upper Gallery % 728 C.Y. 310.00 225,680 Fortion of 3	;	b)Extension of Non-overflow	1,000	C.Y.	600.00	600,000
a) Precast concr. box beams 20 footers 12 EA. 4.000 48,000 50 footers 60 EA. 9,900 594,000 b) Concrete Deck 10,000 S.F. 5.25 52,500 c) Concrete Curbing 1,100 L.F. 9.55 10,505 d) Steel Guard Railing 1,420 L.F. 46.55 66.101 REROUTING EXISTING 3' VENT IN DAM 2,400 C.F. 18.45 44,760 b ELECTRICAL - CABLE. CONDUITS L.S. 61,500 ELECTRICAL - CABLE. CONDUITS L.S. 61,500 DOWNLINGS EMBEDDED METALS L.S. 25,000 DOWNLING INTO EXTISTING CONCR. FOR ATTACHMENT TO NEW CONCR. L.S. 5,200 TOTAL CONTRACT EARNINGS PLUS CONTINGENCIES 9,400,000 ENGINEERING & DESIGN 290,000 SUPERVISION & ADMINISTRATION 720.000	;	c)Filling Upper Gallery &	728	ic.y.	310.00 	225,680
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C)Concrete Curbing	; ;	20 footers	1 1 12 1 60	: !EA. !EA.	1 4,000 9,900	1 48,000 1 594,000
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4 RERCUTING EXISTING 3° VENT	:	c)Concrete Curbing	, 1,100	L.F.	! 9.55 !	10,505
IN DAM 2.400 C.F. 18.65 44,760 5 ELECTRICAL - CABLE, CONDUITS L.S. 61,500 6 MISCELLANEOUS METALS - HANDRAILINGS EMBEDDED METALS L.S. 25,000 7 DOWELING INTO EXTISTING CONCR. L.S. 5,200 FOR ATTACHMENT TO NEW CONCR. L.S. 5,200 7,524,246 CONTINGENCIES © 25% 1,975,754 TOTAL CONTRACT EARNINGS 7,400,000 ENGINEERING & DESIGN 290,000 SUPERVISION & ADMINISTRATION 320,000		d)Steel Guard Railing	1,420	L.F.	, ! 46.55 !	66,101
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TOTAL CONTRACTORS EARNINGS PLUS CONTINGENCIES 9,400,000 ENGINEERING & DESIGN 2000 SUPERVISION & ADMINISTRATION 2000		TOTAL CONTRACT EARNINGS	1 f 1	;	; ;	7,524,246
SUPERVISION & ADMINISTRATION ID0.000	;	CONTINGENCIES @ 25%	i 	; [i ; ;	1,975,754
SUPERVISION % ADMINISTRATION	; ; ;	TOTAL CONTRACTORS EARNINGS PLUS	5 CONTING: '	ENCIE: !	5 :	9,400,000
	!	ENGINEERING & DESIGN	1 3 1	! !	• • •	: IRO,000
TOTAL PROJECT FIRST COST	;	SUPERVISION & ADMINISTRATION	} !	!	1 1	;
	:	TOTAL PROJECT FIRST COST		!	! !	10,000,000

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Table C2 - Cost Estimate for 22' High Gate Alternative

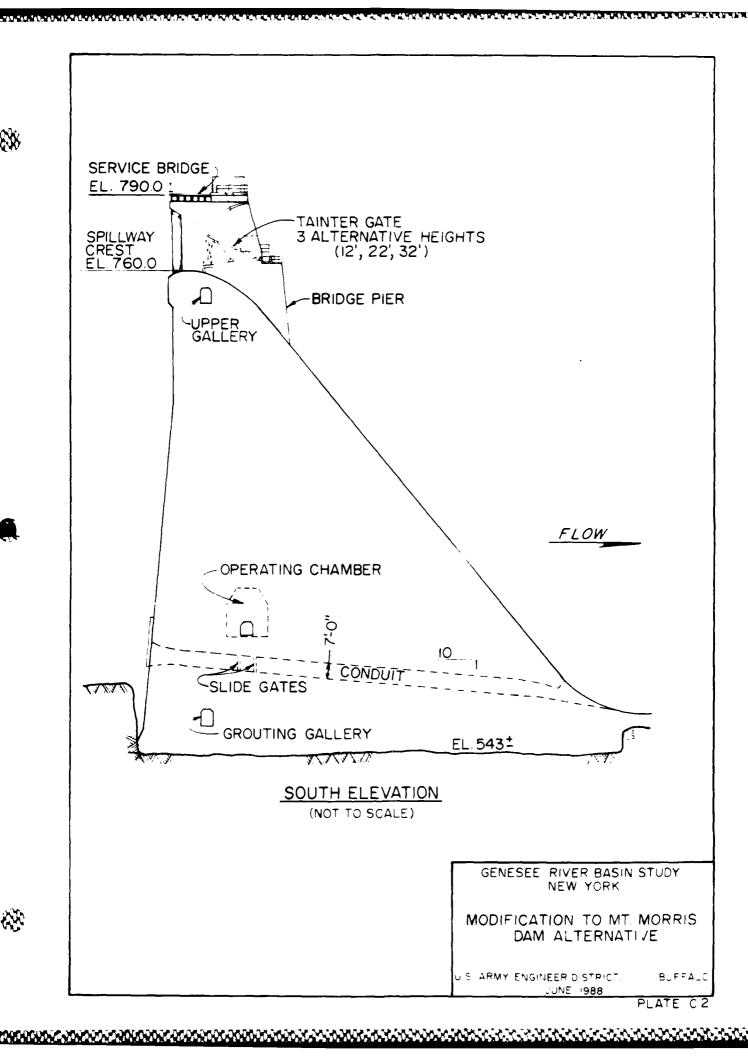
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	PRELIMINARY GOVERNMENT ESTIMATI				SHEET 1 OF 1
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	MT. MORRIS DAM TAINTER GATES				INVITATION NO.
ITEM	DESCRIPTION	ESTIMATE	TIMUE	TINUTT	ESTIMATED
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!	b)Extension of Non-overflow Section	1,000	IC.Y.	600.00 	600,000
1	c)Filling Upper Ballery & Portion of 3° p vent	728	IC.Y.	310.00	1 225,680
3 :	SERVICE BRIDGE	1 1 1	1	} { 1	!
1	a)Frecast concr. box beams	!	!		1 48 000
! !	20 footers 50 footers		EA.	4,000 : 9,900	
i ! !	b)Concrete Deck	10,000	S.F.	5.25 	52,500
, !	c)Concrete Curbing	1,100	L.F.	: 9.55	10,505
;	d)Steel Guard Railing	1,420	LL.F.	46.55	66,101
4	REROUTING EXISTING 3' Ø VENT IN DAM	: 2,400	C.F.	: 18.65 !	44,750
5 :	ELECTRICAL - CABLE, CONDUITS	! !	L.S.	! !	61,500
6 :	MISCELLANEOUS METALS - HANDRAILINGS EMBEDDED METALS	; ; ;	: :L.S.	1 1 1 1	1 1 26,000
	DOWELING INTO EXTISTING CONCR. FOR ATTACHMENT TO NEW CONCR.	 - -	L.S.	! 1	; ; 5, 200
;	TOTAL CONTRACT EARNINGS		;	: !	9,094,246
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, ;	ENSINEERING % DEBIGN	· • •	; ;	! !	330,000
;	SUPERVISION & ADMINISTRATION	: !	:	; ; ;	; 370,000 ;
;	TOTAL PROJECT FIRST COST	1	; !	<u>;</u>	12,100,000

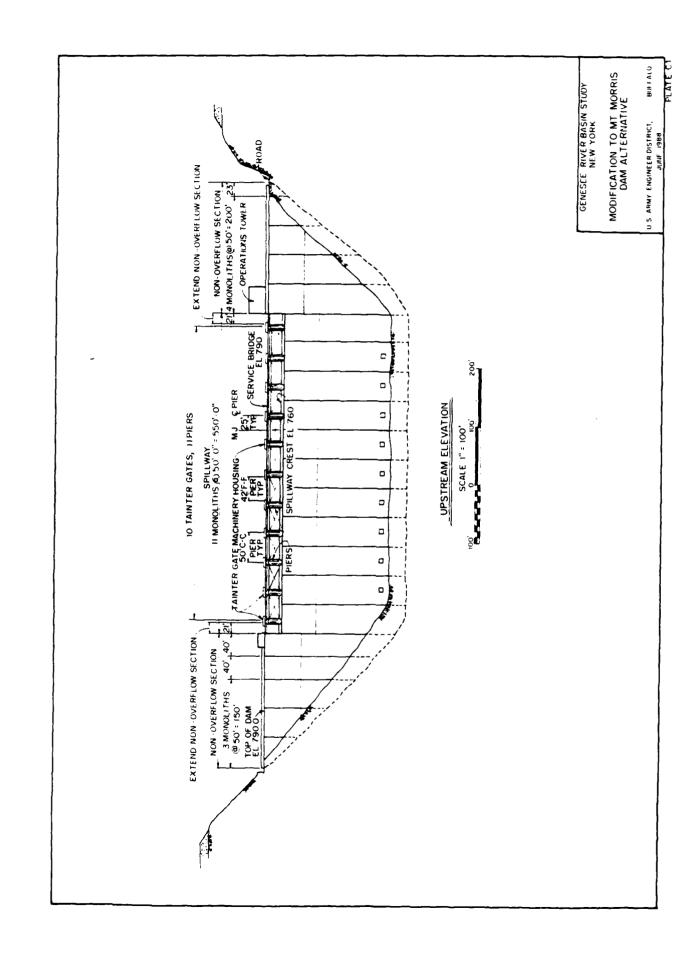
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Table C3 - Cost Estimate for 32' High Gate Alternative

			ANUAF	(Y 7987)	SHEET 1 DF 1	
	PRELIMINARY GOVERNMENT ESTIMATE					
PROJECT GENESEE RIVER PASIN STUDY					: INVITATION NO.	
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i .	b) Extension of Non-overflow	1,000	10.Y.	1 600.00	600,000 1	
	: Section ! c)Filling Upper Sallery & ! Portion of 3° p vent	728 }	E.Y.	310.00 	225,480	
 J 	 SERVICE PRIDGE a)Frecast concr. box beams 20 footers 50 footers		LEA.	4,000 9,900		
1	b)Concrete Deck	10,000	S.F.	5,25	52,500	
; ;	l c)Concrete Durbing	1,100	ic.F.	9.55	10,505	
}	 d)Steel Guard Railing	1,420	il.F.	46.55	65,101	
; ; 4 ;	REROUTING EXISTING 3° \$ VENT	2,400	C.F.	18.65	44,760	
: 5	: ELECTRICAL - CABLE, CONDUITS	1	iL.S.	}	(61,500	
6	MISCELLANEOUS METALS - HANDRAILINGS EMBEDDED METALS	•	 L.E.	!	26,000	
7	DOWELING INTO EXTISTING CONCR. FOR ATTACHMENT TO NEW CONCR.		1.5.	1	5,200	
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1	(CONTINGENCIES @ 20%	1	;	1	2,249,754	
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;	 ENGINEEFING & DESIGN		;	; ; ;	590.000	
:	: SUPSRVISION & ADMINISTRATION	;	;	i :	420.000	
;	 TOTAL PROJECT FIRST COST	; ; ;	i ! !	;	14,000.000	

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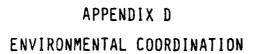


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SUMMARY OF ENVIRONMENTAL COORDINATION

This section briefly summarizes environmental coordination to date on the Genesee River Basin Study with the U.S. Fish and Wildlife Service (USF&WS), New York State Department of Environmental Conservation (NYSDEC), and New York State and Pennsylvania Historic Preservation Offices. However, since No Action is being recommended for this basin study, coordination and compliance with environmental statutes, regulations, and Presidential directives was not pursued further. A No Action recommendation implies that no Federal project would be constructed. Therefore, an environmental impact statement, Section 404(b)(1) Evaluation and U.S. Fish and Wildlife Coordination Act Report was not required for inclusion in this Corps feasibility report. Coordination accomplished to date with the aforementioned agencies follows:

U.S. Fish and Wildlife Service - In order to initiate fulfillment of requirements for Section 7 of the Endangered Species Act of 1973, as amended, coordination letters in this regard were sent to the USF&WS Ecological Services offices in both Cortland, New York, and State College, Pennsylvania. Letters dated 15 April 1985 to the Cortland office and 17 April 1985 to the State College office requested identification of any known threatened and endangered species or critical habitats within the Genesee River Watershed in New York State and Pennsylvania. A letter to the Corps of Engineers, Buffalo District Office, dated 24 April 1985 from the USF&WS field supervisor at the State College office, indicated that the information requested was passed on to the USF&WS office in Cortland. The USF&WS response letter from the Cortland field office, dated 30 April 1985, informed the Corps of Engineers that the bald eagle was nesting and winter-feeding in the general area of Hemlock and Canadice Lakes - which is considered to be critical habitat for this species. The letter also stated that the study area is in the historic range of the Indiana Bat (an endangered species). Excluding the above mentioned species, except for occasional transient individuals, no other Federally listed or proposed threatened and endangered species under their jurisdiction are known to exist in the study area.

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A planning aid letter dated 25 June 1986 (herein enclosed) from the USF&WS Cortland office provided a preliminary evaluation of the Genesee River Basin Study on a number of alternatives that were being considered. The planning aid letter also provided a recommendation for future studies in the vicinity of potential project impact sites that were suggested as necessary to more fully evaluate potential effects. Studies recommended included: a Habitat Evaluation Procedure (HEP) analysis in the Stannards, New York area, a comprehensive fish, wildlife, and botanical species inventory up to the maximum pool elevation, and a study of deer movement patterns within and immediately adjacent to the proposed project areas at Stannards as well as at Mount Morris, New York.

A three season biological field study of existing fisheries, wildlife, benthos, wetlands, and vegetation on the Genesee River at the Stannards and Mount Morris sites was initiated with the USF&WS Cortland office in 1987. The studies were concentrated in the Stannards area (up to approximately the Pennsylvania State line) south of Wellsville, New York and in the Mount Morris area from about Route 36 upstream to the vicinity of St. Helena in Letchworth State Park. Summer season field studies were accomplished at Stannards and at Mount Morris during the week of 20 July 1987, and fall season studies were done during the week of 19 October 1987. Since the proposed flood control dam at the considered Stannard site was eliminated from further consideration early in the feasibility

study phase, spring season biological sampling studies were not conducted at this site. However, spring season biological studies were completed during the week of 25 April 1988 in the vicinity of Mount Morris, New York. A detailed draft biological survey report was prepared in May 1988 by the U.S. Fish and Wildlife Service on results of the aforementioned field studies, and is on file at the Buffalo District Corps office.

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New York State Department of Environmental Conservation - Initial coordination by the Corps of Engineers with the NYSDEC was established by letter dated 30 November 1984 from Colonel Robert R. Hardiman, the Buffalo District Commander at that time, to then Commissioner Williams, advising their Department of the Genesee River Basin Study. A letter of acknowledgment dated 31 January 1985 relative to the aforementioned correspondence was received from Mr. Eric A. Seiffer, Region 8 Director of the NYSDEC Regional office in Avon, New York. Additional Corps letters of coordination requesting baseline data on fish and wildlife resources were sent to the following NYSDEC offices on the dates indicated: Region 8 Headquarters in Avon (16 April 1985), Region 9 office in Olean (12 April 1985), and the Significant Habitat Unit Office in Delmar (15 April 1985). On 9 September 1985, Mr. Jack Cooper (biologist at the NYSDEC Region 8 Avon Office) transmitted some baseline environmental information to the Buffalo District Corps office that provided background data on wetlands, significant habitats, and fisheries in Livingston and Monroe Counties - principally in Genesee River Basin areas downstream of Mount Morris Dam.

Coordination with NYSDEC offices was also made by telephone to obtain environmental information in the Genesee River Basin. On 29 August 1985, the NYSDEC Region 8 office was contacted to obtain air quality information. Additionally, personnel communication on 5 June 1986 with biologists at both the NYSDEC Region 8 and Region 9 offices provided further background data on fisheries in several tributary streams to the Genesee River.

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New York State and Pennsylvania Cultural/Historic Preservation Offices - The New York State Historic Preservation Office and the Pennsylvania Historical and Museum Commission were informed of the Genesee River Basin Study, and related plans under consideration, by Buffalo District letter dated 26 April 1985.

The New York State Historic Preservation Office, in a letter dated 2 July 1985, listed significant numbers of properties which are listed either in the National Register of Historic Places or the New York State Historic Inventory. The same letter also stated that "...it is the opinion of the SHPO that your project lies in an area that is archeologically sensitive." An archeological survey of the impacted project areas was recommended to determine the nature and extent of archeological resources in the project area.

The Pennsylvania Historical and Museum Commission, in a letter dated 13 June 1985, stated that "...there is a high probability that historic and/or archaeological resources exist in the project areas. We would advise that project planners conduct investigations or surveys to identify any possible resources before final plans are formulated."

Because of the above recommendations and the lack of comprehensive prior cultural resources studies in the project area, a cultural resources survey was planned to be contracted for any selected proposed project sites. However, since No-Action was recommended for the Genesee River Basin Study, no cultural resources field or contract work was either awarded or performed.



UNITED STATES DEPARTMENT OF THE INTERIOR FISH AND WILDLIFE SERVICE Suite 322

315 South Allen Street State College, Pennsylvania 16801

April 24, 1985

0F12. MGHT. UAS

Colonel Robert R. Hardiman District Commander, Buffalo District U.S. Army Corps of Engineers 1776 Niagara Street Buffalo, NY 14207

Dear Colonel Hardiman:

This responds to your April 17 letter requesting identification of any known threatened or endangered species or critical habitats within the Genesee River Watershed in Pennsylvania.

Since only a small portion of the Genesee is in Pennsylvania, we have provided information to the Cortland office regarding threatened and endangered species. You will be hearing from them soon.

We appreciate your interest in protecting threatened or endangered species.

Sincerely,

Thale,

Charles J. Kulp Field Supervisor

New York State Department of Environmental Conservation

6274 East Avon-Lima Road, Avon, New York 14414

Telephone: (716) 226-2466



Eric A. Seiffer Regional Director

January 31, 1985

Robert R. Hardiman, Colonel Department of the Army Buffalo District, Corps of Engineers 1776 Niagara Street Buffalo, NY 14207

Attention: Timothy E. Byrnes, P.E.

Dear Colonel Hardiman:

RE: Genesee River Basin Study - New York

Thank you for your letter of November 30, 1984 to Commissioner Williams advising us of the subject study.

We will be pleased to assist Mr. Byrnes in the reconnaissance phase within the limits of our staffing capabilities. I have asked Albert Butkas at this office to serve as our primary liaison with your office on a semi-formal basis. Of course, formal position statements of the Department will be in writing to you from Commissioner Williams or his designee.

We look forward to working with Mr. Byrnes on the Genesee basin project.

Sincerely,

Eric A. Seiffer

PFS:1m

cc: Commissioner Williams
Dan Barolo/Ed Karath
Bill Romer
Edward Holmes
Paul Schmied/Bruce Butler
Al Butkas

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UNITED STATES DEPARTMENT OF THE INTERIOR OF 0. MGHT. OAS FISH AND WILDLIFE SERVICE

2 Mar 85 11 55 B

100 Grange Place Room 202 Cortland, New York 13045

April 30, 1985

Colonel Robert R. Hardiman District Engineer, Buffalo District U. S. Army Corps of Engineers 1776 Niagara Street Buffalo, New York 14207

Attention: Leonard Bryniarski

Dear Colonel Hardiman:

This responds to your April 15, 1985, letter to our office and your April 17 letter to the State College Field Office requesting identification of any known threatened or endangered species or critical habitats within the Genesee River Watershed in New York and Pennsylvania.

Information provided by the New York State Department of Environmental Conservation indicates that the bald eagle, an endangered species, is nesting and winter feeding in the Hemlock and Canadice Lakes area. The state lists this area as critical habitat for the bald eagle.

Your study area is located within the historic range of the Indiana bat, an endangered species. The distribution of this species is strongly correlated with the major rivers within its range and it has been postulated that the major rivers are migration routes. If your project will impact any caves or tunnels likely to maintain temperatures above freezing throughout the winter, the Service and the New York State Department of Environmental Conservation or the Pennsylvania Game Commission should be notified. A specific investigation may be required to determine if Indiana bats would utilize the caves. At this time we are not aware of any such wintering caves in the study area.

Excluding the above-noted species (the bald eagle and Indiana bat), except for occasional transient individuals, no other federally listed or proposed endangered or threatened species under our jurisdiction are known to exist in the study area. Therefore, no Biological Assessment or further Section 7 consultation is required with the Fish and Wildlife Service (FWS). Should project plans change, or if additional information on listed or proposed species becomes available, this determination may be reconsidered. A list of federally listed endangered and threatened species in New York is enclosed for your information.

This response relates only to endangered species under our jurisdiction. It does not address other FWS concerns under the Fish and Wildlife Coordination Act or other legislation.

Thank you for your interest in endangered species. Please contact us if we can be of further assistance.

Sincerely yours,

Paul P. Hamilton Field Supervisor

Enclosure

cc: Peter Nye, Endangered Species Unit, NYSDEC, Delmar, NY



COMMONWEALTH OF PENNSYLVANIA PENNSYLVANIA HISTORICAL AND MUSEUM COMMISSION

BUREAU FOR HISTORIC PRESERVATION BOX 1026 HARRISBURG, PENNSYLVANIA 17108-1026

June 13, 1985

19 JUH 85 18 UMB

Robert R. Hardiman Colonel, Corps of Engineers District Commander Department of the Army Buffalo District 1776 Niagara Street Buffalo, New York, 14207

Re: ER #85-0409-042-A

Subject: Genesee River Basin (Authorization Report), NY & PA,

Study

Dear Mr. Fardiman:

The above named project has been reviewed by the Bureau for Historic Preservation in accordance with Section 106 of the National Historic Preservation Act of 1966, Executive Order 11593 and the regulations of the Advisory Council on Historic Preservation (36 CFR 800).

Because this planning study indicates that a large area is under consideration and a much smaller area will ultimately be affected, it is impractical to consider project impact on historic and archaeological resources at this time. When planning specific alternative project locations, provisions should be made for the identification of historic properties listed in or eligible for the National Register of Historic Places and for the assessment of the effects of the project will have on these resources. If you need any advice or assistance in conducting these kinds of investigations, please contact the Division of Planning and Protection, Bureau for Historic Preservation.

A preliminary review of this project indicates that there is a high probability that historic/and or archaeological resources exist in the project areas. We would advise that project planners conduct investigations or surveys to identify any possible resources before final plans are formulated. For assistance in conducting and organizing a survey, please contact the Division of Planning and Protection.

If you need further information in this matter, please consult Kurt Carr or Dr. Paul Raber of the Bureau for Historic Preservation at (717) 783-8947.

Sincerely,

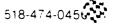
Dan G. Deibler, Acting Chief Division of Planning & Protection Bureau for Historic Preservation

(717) 783-8946



New York State Office of Parks, Recreation and Historic Preservation

The Governor Nalson A. Rockefeller Empire State Plaza Agency Building 1. Albany, New York 12238



July 2, 1985

Mr. Tod Smith U.S. Army Engineer District 1776 Niagara Street Buffalo, NY 14207

Dear Mr. Smith:

Re: Genesee River Basin Study
Multi-County; Cattaraugus Creek
Watershed-multi-county;
Buffalo River Section 205
West Seneca

The Field Services Bureau of the New York State Office of Parks, Recreation and Historic Preservation has received your request for information on properties or sites which are included in or may be eligible for inclusion in the National and State Registers of Historic Places.

Based upon the information which you provided and a file search conducted by our staff, we have been able to determine that the following numbers of resources are located in or in the vicinity of the project area which you identified. Individual listings are too numerous to mention here. If more specific information is required, you are welcome to use our files.

- I. National/State Register of Historic Places listed or eligible properties:
- A Genesee River Basin Study: Allegany County, 8 listings; Genesee County - 10 listings; Livingston County - 15 listings; Monroe County - 54 listings; Ontario County - 23 listings; Wyoming County - 5 listings.
- B Cattaraugus Creek Watershed: Allegany County and Wyoming County, same as above.

 Cattaraugus County 3 listings; Chautauqua County 27 listings
- C Buffalo River Section 205 Flood Control Casimir Avenue Eaton Site; 2985 Seneca Street
- II. Properties included in Statewide Inventory
- A. Genesee River Basin Study: Allegany County approximately 450 properties in various towns; Genesee County ...408 properties...;

Mr. Tod Smith July 2, 1985 Page Two

Livingston County - ...760 properties...; Monroe County - ...2174 properties...; Ontario County - ...776 properties...; Wyoming County - ...468 properties...;

B - Cattaruagus Creek Watershed: Allegany and Wyoming Counties same as above. Cattaraugus County - approximately 260 properties in various towns; Chautauqua County - ...1014 properties....

In addition, with regard to archeology, it is the opinion of the SHPO that your project lies in an area that is archeologically sensitive. This determination is based upon the SHPO's archeological sensitivity model. Archeologically sensitive areas are determined by proximity to known archeological sites, as well as the area's likelihood of producing other archeological materials.

Based upon the use of the model, it is the SHPO's recommendation that unless substantial ground disturbance can be documented, an archeological survey be undertaken to determine the nature and extent of archeological resources in your project area. If you wish to submit evidence regarding ground disturbance, it should include statements concerning the nature and date of the disturbances as well as a map indicating the locations and depths of such activities. Photographs of recent construction activities keyed to a map are very useful in this regard. Once we have had an opportunity to review the additional information provided as a result of the archeological survey, or the evidence regarding prior disturbance, we will be able to complete our review of this project and issue our final comments. This statement applies to all three referenced projects.

Sincerely,

Jalia S. Stokes

Deputy Commissioner for Aistoric Preservation APPENDIX E
CORRESPONDENCE

KANA DISPOSI BEDING DIKESA BESINS



STANNARD PRO & CON





18 SEP 87 09 12

September 15, 1987

Army Corps of Engineers Genesee River Basin Study 1776 Niagara St. Buffalo, NY 14207

Re: Proposed Stannards Dam Project

While I'm sure that if my home or business were in the area to be affected by the project, I would be as concerned and upset as those who own property in the area are, I also cannot help but see this project as having a very high probability of being extremely beneficial to Allegany County, and more specifically Wellsville, from an economic standpoint. Everyone talks about encouraging the tourism business based on currently existing resources, but that simply isn't happening.

Assuming property owners in the area affected by the proposed project are adequately compensated for loss of property, loss of income and inconvenience associated with relocation and are assisted in relocating within the county in equivalent areas, I feel that the long term economic benefit to the area as a whole would outweigh the short term problems the project would create.

Best regards,

Susan Ideker

63 Herman

Wellsville, NY 14895

(716) 593-5893

cc: Hasper Lur

Lundine Houghton

Present Cuomo D'Amato Moynihan

ck

CUMMINGS. EMBSER & WOLTAG

ATTORNEYS AT LAW DENNISON BUILDING 100 NORTH MAIN STREET WELLSVILLE, NEW YORK 14895-1289

J. TIMOTHY EMBSER MARC I. WOLTAG G. WILLIAM GUNNER. III

October 6, 1987

TELEHHONE 116 593 1090

Army Corps of Engineers Genesee River Basin Study 1776 Niagara Street Buffalo, New York 14207

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Att: Mr. Weiner Cadet

RE: STANNARDS LAKE

Dear Mr. Cadet:

COCKERS DESCRIPTION OF THE PROPERTY OF THE PRO

I am writing to voice my approval of the Army Corps' proposal to build a lake south of Stannards. This letter will act as a supplement to my comments at the public hearing on the matter held at Mount Morris on September 10, 1987.

Primarily I would like to comment on the regional economic impact of the recreational aspects of the lake. I believe that the corallary benefits of flood control and erasion protection are already well documented in the Corps' preliminary study.

Several months ago this project was brought to my attention and my immediate response was that it would be great for the local economy. Virtually everyone with whom I have spoken agrees with this assessment, although many have suggestions regarding the management of the lake to maximize the potential of various recreational uses. Of those I have spoken to who are undecided about favoring the lake, the most frequent reason cited is the lack of a clear quantification of the regional economic impact. Of those opposed, various reasons are given but the common denominator of the opposition seems to be a fear that fair compensation will not be offered for condemned properties. As one landowner, Mr. Fanton, stated in a recent newspaper article, "I'll get the assessed value of my land and that's not enough to buy one tractor."

I have given much thought to the issue of regional economic impact in an effort to substantiate my gut feeling that "It would be good for the area." However, since I am in no way an economist, I have been unable to compute just what the impact might be. While I suspect seasonal residents and visitors would spend money on gas, food and other supplies, I cannot translate this into a dollar figure. For this reason, I think it is important that the Corps include in their study a detailed analysis of the regional economic impact of a lake in Stannards.

While I have been unable to quantify precisely the regional economic impact, I am not without a reasonable guess. I have obtained a study of the regional economic impact of the Great Sacandaga Lake which was prepared by Peat Marwick in Washington, D.C. (copy enclosed). This study highlighted for me the similarities between Sacandaga Lake and the proposed Stannards lake. Sacandaga was created in 1925-29 for purposes of water regulation in the Hudson River following devestating floods in 1902, 1913 and 1922. It has since attracted numerous seasonal residents and visitors as a recreational facility.

I believe the actual regional economic results of Sacandaga can be taken as something of a litmus test for the potential regional economic impact of a lake in Stannards, as follows (comparisons based on relative number of miles of shoreline where appropriate):

- regional permanent population growth exceeding state average by a factor of 4
- increased variety of businesses, particularly seasonal
- 3. market value of seasonal property (eventually) of \$38 million (thus expanding town, county and school tax bases)
- 4. annual maintenance expense on seasonal property of \$190,000
- 5. expenditures per person per day ranging from \$7.40-32.25 resulting in annual influx in recreation dollars of \$5.5 million annually (plus multiplier effect)

I believe these impacts would have a tremendously positive effect on the economy of the Willing-Wellsville area particularly and on Allegany County in general. I believe that unemployment would be reduced drastically (helping to break the welfare cycle), that the tax base would be greatly expanded (thereby improving education and services) and that the area would be much more attractive as a location for new industry and commerce.

I have not exhausted my efforts to more specifically assess the economic value of a lake in Stannards to the regional economy, nor am I a lone voice in the wilderness. I am sending this letter now to comply with the 30-day comment period for the Mount Morris public hearing. In the weeks and months ahead I hope to urge proponents of the lake to become organized and to speak out. At this time I believe there has been a small but well organized and very vocal opposition to a lake in Stannards and a large but

unorganized and mute group of supporters. As evidence that this project has its proponents I am enclosing a preliminary list of signatures of supporters.

Very truly yours,

WILLIAM GUNNER, III

GWG/ap Enclosure

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COCCOSO DOS PERO APPRECED DOS PROPERTY

To lokan It May Commen We wish & how our wind and opinions to the proposed Stamula from Fragict. De ne of the unkertenling, that along with flood control, the project would provide you conducted of this The resulting evening import on the area would be heretis I've certainly needed Because of the recreational agreets and the some injust, we are very much in favor of the project and would like to cooperate in any way sossible We read with interest the very of some who are organized in apportion to the project and feel that are opinions in fact are unique for the following ressons: 1) Despite living along the siver north of where the day would be located, we are in fever of the project. We am almost 3CC ares of land which would be directly

menty wated tale tief as an inter one with Est by the imase River for about 3/4 mile South of about to proposed dan would be located. Espete this, in we We are intracted in participating in, or contributing to, the We are interested in leaving more about the

PETITION FOR APPROVAL OF STANNARDS DAM PROJECT

We, the undersigned residence of Allegany County hereby articulate our approval of the US Army Corps of Engineers' proposal for the construction of a dam in Stannards, New York provided that the completed project will be managed as a recreational facility, including no draw-down of the high water level during the summer months, Memorial Day through Labor Day and fish management.

Wellsulle, 11. 9 Wellsville 114 Wellsirel, M.C

689 TRAPPING BROOK RD. WEUSWILE MAY.

Williams fra MLSO

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Patrick J. Regen	R.D#1 Box 68 Willsville. n.y.
Decide	R.D#1 Box 68 Willwille. n.y.
Theresa Leenman	44 Ryke St. andover wry
Marilyn Wycklam	300 Brenen St. Rockester, M.
Skaff Rowly	Anvale Ro. Indover NY. 1480
Charles P. Scott	314 E. Dyke St., Wellwill, Ny
Linoily J. Mixe	49 School St. Wellowll ny
Sugar Mallery	White Hill Ra Bolwar, 414
Land Bolander	25 Orchard Al- Millsulle
Robyn S. Smith age 30	110 Wellswill St. Bolivar, My
WM Pratelle	RO# Wellsulle NY.
Alchard Das	165 Madus St. Woldville
Mary C. Syler	/ l
Wat Marking	165 WARSON ST. WORLS
Boberta Sprague	171 & Dyke St. WISO. M'X.
Mishal Ruft	104 Whitesuille

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Day through Labor Day, and fish mana	agement.
Celle Davhay	220 E. Pearl WISU 593 4057
NF BONONS	Box 75 WSD 593-652
Eva Benedit	LI#1 Box 75 Wellswille ny.
Bill Sweet	RO+4 Box 104A Wellaid My
Ruby J. Hubla	R.D. #3 Wellwills
Lightin Ramsing	3214 huinsids des West
TRISHA CASCHERA	RVERSIDEDR. WUSU.
Steve 1 CASCHERA	11 11 11
deg Gostley	Parl St. Wallsville N.Y
B of Jum	102 Wms AVE Walland MY
Alganne Dyd	17 Central, Wellswillo
Ken Cochera	
Mary Joine June	Pith Hill Dollandle
Shelly O Thank	Do Bax 231 Wellselt M
Christopher & andutor	102 EARLY ST. WLSU, N.Y.
Markey	RRIBONH8 Scio.N.C

Do we need any more?

PETITION FOR APPROVAL OF STANNARDS DAM PROJECT

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O. William Dunner II	Welkville NY
On of the State of	RO#3 Weresuce, 71.4.
How Almor	WELLSVILLE, N.Y.
Ton Brown	Wellsville N.4.
Orem Down	WEUSUNE NY
Fras CE half	WELL SYNCE
Mrs. Jack Donown	Weelswell
Dual Padden	Richardson, Texas; Hunt Hill Hilton
DANIEL A. MCLAUGHLIN	ALLEGAN CONT OFFICE OF DEVELOPMENT
BUEZ ERICSON	WELLS
DE J. Cliff	Stemas Willing
Tom Maria	
Tomis (Birty	1201 51 5 Worlde
Die & Men	(12.70)
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Dovid J. Lorek	138 W. Stat Wellwill ny
Dorda Bundeel	52 DYKE ST. WELLSUILLE, N.Y
Bill Dorden	13 Hella St. Willwill
asph Coate	4078 Pine ST Seio, D.4.
Saul Cozzi	EScirta St. Scion.4, 14880
for Small	Main of Belmont my.
Dan Quin	309 WEST SLAVE VEILSWILLE
John Ting	18 Mondowhout G. Willoute
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HAILROOM-HOBIN-S

28 SEP 87 11 39

September 23, 1987

Eugene Miller Co-Pastor: Yorks Corners Mennonite Church RD #1, Box 194 Wellsville, NY, 14895

Col. Daniel Clark
Department of the Army
Buffalo District, Corps of Engineers
1776 Niagara St.
Buffalo, NY 14207-3199

Dear Col. Clark,

I am opposed to the proposed construction of a dam near Stannards, NY [as outlined in the U.S. Army Corps of Engineers, Buffalo District, "Reconnaissance Report," "Genesee River Basin Study" Volume I, dated August, 1986, revised December, 1986] for several reasons.

First and foremost, it would destroy a community. The lives of many would be severely disrupted. Many of those living in this valley have deep roots in this community. The Church at Yorks Corners is active and growing and has been an integral part of this community for many years. We do not wish to see the graveyard across from the church disturbed.

Secondly, the area currently provides excellent wildlife habitat with good hunting, fishing, hiking and other outdoor recreation. This would all be destroyed.

Thirdly, from an economic viewpoint, I believe that the construction of a dam cannot be justified.

Sincerely,

Eugene Miller Co-Pastor: Yorks Corners Mennonite PROPERTY LINEARING STANDARD DESCRIPTION

Church

HAILROUM-NOBIM-S

6 0ct 87 09 32

Date:

Department of the Army
Buffalo District, Corps of
Engineers
1776 Niagara Street
Buffalo, NY 14207-3199

ATTN: Col. Daniel Clark

District Commander

Dear Col. Clark:

Please be informed that I am opposed to your proposed construction of a dam (reservoir on the Genesee River, at or near Stannards Corners, NY). Said proposal as outlined in the U.S. Army Corps of Engineers, Buffalo District, "Reconnaissance Report," "Genesee River Basin Study" Volume I, Main Report, dated August, 1986, revised December, 1986. Let it be known that I am opposed to any or all of the twelve (12) plans as outlined in said Report.

I remain.

Name: _____ Address:

City, Zip:

HAILROOM-NCBIM-S

20ct 87 09 12

Date:

Department of the Army Buffalo District, Corps of Engineers 1776 Niagara Street Buffalo. NY 14207-3199

ATTN: Col. Daniel Clark District Commander

Dear Col. Clark:

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I remain, indecrely 'ours, foam loss

Torna nad Joan Tyna : Torida Ave . Name: Address: City, Zip: And wills,

Date: Alph 12 10 ...

Department of the Army
Buffalo District, Corps of
Engineers
1776 Niagara Street
Buffalo, NY 14207-3199

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Constant section between territorial

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I remain.

Name: Address: City, Zip:

582E81 15 20

HALL ROOM-HOBIM-S

MT. MORRIS
PRO & CON

HALL ROOM- NOOM-\$

10 Delaware Avenue Rochester, New York 14623 October 4, 1987

Colonel Dan Clark, Director

Mr. Wiener Cadet, Project Manager

Army Corps of Engineers, Buffalo District

1776 Niagara St., Buffalo, New York 14207

Gentlemen:

We live at East River Road and Delaware Avenue in the Genesee River flood plain (elevation approximately 520 ft.), as shown on the attached map.

Even with the flood protection of the Mt. Morris dam, Federal Flood Insurance was required when we purchased this property in 1986.

we want to be sure that the Mt. Morris installation is operated and maintained to provide maximum flood protection with or without incidental hydroelectric generation.

If any major modifications or changes are contemplated or realized at Mt. Norris, the flood protection factor should take precedence.

We are also sensitive to the cost of electric energy and would have serious reservations about the addition of any hydroelectric systems that would result in increased charges to consumers of electric energy.

Yours respectfully

Bradshaw Burnham.

Bradshow Burnham



GLEN-COE CONSERVATION SOCIETY, INC.



COLDEN, N.Y.

April 8, 1988

Colonel Daniel R. Clark
District Engineer-Buffalo District
US Army Corps of Engineers
1776 Niagara Street
Buffalo, NY 14207

Dear Colonel Clark,

The members of The Glen-Coe Conservation Society would like to extend a note of Thank-You to you for allowing Mr. Wiener Cadet and Mr. Dan Kelley to give a presentation on the proposed Damn modifications of the Genesee River at Mount Morris, New York. The presentation at our March 29, 1988 monthly meeting was extremely enlightening and very thorough. Mr. Cadet was well prepared to discuss and explain all background information as well as current studies and future effects of the project.

The discussions that followed the presentation, amoung members, were all in agreement. Given the information presented the members felt that the project was a step in the right direction to provide better storage capacity at the damn site and thus conserve land and soil under extreme conditions. We were all greatful to have had the opportunity to take part in the program that was presented.

Sincerely

William H. Solak, Club Secretary

cc: Mr Wiener Cadet
Mr Dan Kelley

TONN A HAMN'
2 SHAREN CT
(HEEK TOWAGA, NY 14335
9/22/87

FORPS OF ENGINEERS

 \mathfrak{I}

I WOULD LIKE TO STRONGLY PROTEST ANY
IDEA THE CURPS MAY HAVE FOR BUILDING
A DAM IN LETCHWORTH STATE PARK.

I DON'T UNPERSTAND HOW A BEAUTIFUL
PUBLICLY OWNED PARK, CAN BE TAKEN DWAY
FROM THE PEOPLE OF MY STATE FOR
PRIVATE PROFIT.

MY FAMILY AND I HAVE ENJOYED THAT

PARK FOR YEARS, AND I EXPECT IT TO

BE INTACT FOR MY GRAND CHILDREN.

THAT PARK WAS GIVEN TO THE PEOPLE

FOR THEIR ENJOYMENT, AND NOT FOR

SOME ONE TO FLOOD FOR PROFIT.

LEAVE IT THE WAY IT IS! NO DAMS.

Johna Hahn

9-23-87 8506 Heath Road Colden, M. Y. 14033 We don't want to see anything lappen to our beautiful Letchworth State

Park

Sept 28, 1987 4418 Canal Rd. 5penceepat, N.y. 14559

MAIL ROOM-NCBIM-S

30 SEP 87 11 28

Dear Sus!

I am appalled that you are even considering allowing the mitex corp of Boston, Mars to build doms in the Letchworth Park Ocha+ The flooding that would result from the Mount Morris Dom to wolf Geek would after He recreational and scenic wonders of the resultigage. It is my understanding that the new dams will not aid in flood control and their sole purpose is to make money for a private corp. Isn't it time the present administrate Stop selling us four environent for a feir prieces of silver. Sincerely yours Thomas W. Lewis

\$

1141830M-MOSIM-S

20 Renselaer Et. Rocherter, Dy 14618 October 15, 1917

Cot. Dan Clark, Dientor Mr. Wiener Cadet, Project Munujer Army Corps of Engeneer, Buffalo Finthiert 1776 Majara & Frent Buffalo, New Uport 14207

Dear Col. Clurk and Mr. Cadat, It has come to our attention recently that the army Corps of Engineers has pro-posed a reservoir within Le teliworth & Park. We understand that the premary become benefit would be to the contex Corp. of Bros For RGE of Rochester Rus testified that power costs from the facility would increase local nates, nather than decrease Oside from this factor, we are strongly opposed to any project that would so drastically after the character of Letch worth Park It is one of the prime scenic and recreational areas in the state, and is Enjoyed annually by through with this wronge you not to carry through with this project. Mr. and Mrs. James J. Bosden Col Danniel Clark U.S. Army Corp of Engineers 1776 Niagara Street Buffelo Niy, 14207

MAILROOM- NOBIM-S

5007 97 11 20

Dear Sir.

I AM Writting you in regards to the Two dams That
you Have Proposed to build in the Genesee river. I AM
Against The building of These dams in the Genesee
Tiver As it would Ruin Some good Trout breeder Streems
And wood duck Nesting Areas. Which we realy Need to Protect.
I wish the Corp Could find Some other Projects to
emprove the environment instead of destroying it. It seems
to me that every Project the Corp doe's you Make Matters
worse instead of enproving Them. If you Need A project
Why Cen'tyou build More Harbers And Boot Excess sites
Along Lake Eric Which We need badely in the South Towns
Hree.

yours for Conservation Calvin mathew

Calvin M. ArTHUR 84694 Morgan Parkway Hamburg Niy, 4075 Set Baston abtain their awn water Paewer - "Dawn with down dom" - Dresume Letch warth!!

"Dawn With the Dom"

Lusar Inc larthy 1217 Delaware One Buffalo D. 9 14209 MAIL ROUM-NCBIM-S

THOMAS G. WHITE

220 SEXIRMEX ROUXE

56 Sable Palm Drive

1007 87 11 27

PHONE (716) 332X 8374X

684-4931

September 29, 1987

Mr. Wiener Cadet, Project Manager Corps of Engineers 1776 Niagara St. Buffalo, N. Y. 14207

Dear Sir:

My wife and I have read with great interest and concern the Letters to the Editor which appeared in the Buffalo News a week or so ago.

One of these letters, by a Batavia resident, concerned plans underway to build another dam and flood part of the Genesee River gorge, thereby damaging and destroying thousands of acres of park land by burying it under water.

As people who have enjoyed Letchworth Park in its present state, we are opposed to this needless undertaking.

Yours sincerely,

Thomas G. White

Wrama C. White

Virginia C. White

MAIL ROOM-NOBIM-S

900787 1116

24 Park Place Holley, New York 14470 October 6, 1987

Col. Dan Clark, Director Weiner Cadet, Project Engineer Army Corps of Engineers, Buffalo Dist. 1776 Niagara Street Buffalo, New York 14207

Dear Col. Clark:

My family, at least three generations, has loved and enjoyed Letchworth Park for many, many years. Without a doubt the Gorge has been its most fascinating aspect.

The additional flooding of at least 11 miles of the Gorge will have a detrimental impact on the beauty and activities associated with it.

I urge that the proposal for additional flooding of the Gorge for hydropower purposes be abandoned.

Sincerely, Kenture D. Mark.

Lawrence D. Mark

129 West Avenue Fairport, New York 14450 September 26, 1987

Wiener Cadet Project Manager Corps of Engineers 1776 Niagara Street Buffalo, N.Y. 14207

Gentlemen:

Please consider this a strong protest against the proposed plan to flood the Letchworth gorge! The very thought of tampering with this park is an abomination.

This land was a gift to future generations so they could enjoy nature in its finest array. Mr. Letchworth, not unlike Henry David Thoreau, was a man of vision.

Let us not rape the land, nor tamper with the balance of nature. Men are blinded by dollar signs when they even consider such a venture.

In this time of bulldozing, stripping, polluting, and dumping, may we have the good sense to keep hands off the few virgin areas we have left.

I cannot emphasize this point enough.

Sincerely,

Donna Wilcox Edgerton

25 21 13 7856

S-MISON-MODEL AN

MAIL ROOM- NOBIM-S

7 007 87 10 21

Col. Dan Clark, Director Army Corps of Engineers, Buffalo District 1776 Niagara St. Buffalo, NY 14207

Dear Col. Clar::

respected appropriate president problems

It has come to my attention that the Corps has plans to alter the Mt. Morris dam to create a reservoir involving the Senesee River gorge at Letchworth. I also note that this construction will not be to the benefit of the citizens of New York; that, on the contrary, it will be to our detriment: raising electricity rates to New Yorkers, while a private concern in Massachusetts profits from the hydropower facility; conserting a popular white-water river into a lake, destroying a recreation area used by canoeists, rafters and kayakers, as well as inundating woods and biking trails in the Gorge; and all this with no net improvement in flood-control capabilities at the dam.

I would like to go on record as opposing this effort to damage the ecosystem and recreational opportunities at one of our state parks.

Sincerely,

uJohn S. Howell, MJ $_{ ext{F.S.}}$ Bax 1150

Fairport, NY 14450

cc: Sen. Alfonse M. D'Amato, NY Sierra Club, Atlantic Chapter 0.00-07 09 31

TO:

Col. Dan Clark, Director Mr. Wiener Cadet, Project Manager Army Corps of Engineers, Buffalo District 1776 Niagara St.

Buffalo, New York 14207

I oppose any projects which would alter the character of the Genesee River and Letchworth Gorge within Letchworth State Park. The proposed creation of the Letchworth Reservoir would severely compromise the scenic beauty of the Park, destroy whitewater recreational activities, and innundate and destroy the unique ecosystem existing within the Gorge.

Letchworth State Park was created for the use and enjoyment of all the people of New York State through the generosity and foresight of Mr. Letchworth. I oppose any attempt to wrest this stretch of the Genesee River and the beauty of the Gorge away from the residents of New York State.

> James J. Caffery 793 Chili Ave Signed: Address: Rocketon NV 14611

29 SEC 37 99 16

TO:

Col. Dan Clark, Director Hr. Wiener Cadet, Project Hanager Army Corps of Engineers, Buffalo District 1776 Hiagara St. Buffalo, New York 14207

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Signed:	- Tarrell !	1 12
Address:	1/1/ 1/200	r 18. J
	· · · · · · ·	1 1 1116

TO:

Col. Dan Clark, Director Hr. Wiener Cadet, Project Hanager Army Corps of Engineers, Buffalo District 1776 Niegara St. Buffalo, New York 14207

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Citibater, Her - rok 14550

Signed:

Address:

25 21 13 332

S-MIBON -HODEN NEW

TO:

3385.87 89 16

Col. Dan Clark, Director
Mr. Wiener Cadet, Project Hanager
Army Corps of Engineers, Buffalo District
1776 Niagara St.
Buffalo, New York 14207

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Signed:

Address:

456 Coverond Blue

₩ 1345 1 174**631M-\$**

To: 42

Col. Dan Clark, Director Mr. Wiener Cadet, Project Manager Army Corps of Engineers, Buffalo District 1776 Niagara St. Buffalo, New York 14207

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Letchworth State Park was created for the use and enjoyment of all the people of New York State through the generosity and foresight of Mr. Letchworth. I oppose any attempt to wrest this stretch of the Genesee River and the beauty of the Gorge away from the residents of New York State.

Signed:

Address:

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TO:

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Signed:

Address:

Clara Lockwood

Box 517 RAZ

Randolph, N.y. 14772

PETITION to SAVE LETCHWORTH

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Jan Strenmager	13800 Hensker Al. Alden NY.
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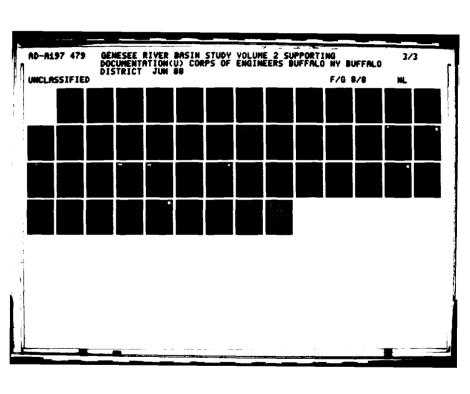
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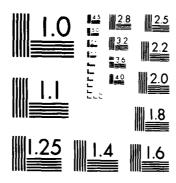
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MICROCOPY RESOLUTION TEST CHART NATIONAL BUREAU OF STANDARUS 1964 A

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NAME **ADDRESS** und outhwere ary Cin 5AWYER ARISTOL TON NY

We only have until October 10th! Please return by October 8th to: Shirley A. Scherlein, 12917 Main St., Alden, NY 14004 - 937-6337.

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ADDRESS NAME 2549 Conton 14200 309 MUNIZZI

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NAME **ADDRESS** Barbara Weimar 5+. 14206

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NAME	<u>ADDRESS</u>	
JOHN BEST	3843 N MILLGROUC	ALDEN 14504
Laver Barry	1195 Exchance St.	HLDEN 14004
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Chris Elleiser	902 Exchange at.	alden 14009
KICHARD STERNER	1562 CRITTONDEN X	AWON 14004
Tony Stein Ir.	14/82 /he//can st.	ALDEN 14004
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SCOTT FITSCHESS	• /	ALDEN 14004
Jim Wanty		Alden 14031
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Robert N Kulrald		Fancaster N. V. 14086
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James C. Martin	1468 Helloga St	alden 14 out
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Philosophia State	16 Buchanan SI. 18 1593 ENERSON ST.	Albany N.Y. 12306 Alban Ny 14000
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Darry C. Malona	Calgary Alberta	CONTRACTOR CONTRACTOR
Mich de Wasoluth	12971 BERDENAL	ALDEN NY
PAULIAURINONIC	13/3/2 BROADWAY	ALDEL AY.
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Sundre Hope	3672 Branwood Ct. Hamburg
Debbe Kargmanayk	52 Barks Depew, NY 4660 Boncust Sast, Svillas, NY
Mara Jenerso	4660 Boncust East, Svillag, NY
Debbie Karmanczyk Mara Janezsa Storsuncel	43 Broezel Lancaster NY
Carry Battistoni	and Melody dane Cheok toward, N.Y.
goff Warsham	172 Central Ave Lancaster NY
Edition Person	21 Grandel and fancatter N.Y.
many depleye	15 Ashford Pl. Depurky 14043
- Juano Diemoor	256 Pleasantree Dr Jane 1408
- Cara Bruss	24 Mars Hara Lanc 1406
Herrin Scatherborrace	19 South Irwin wood
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Michelle Boyd	221 Marrare Dr. Peper 47043
Carrie tratt	221 Travarion Dr. Pipu HOH3
Chan Friedmann (Fred)	79 Southwest Parkway Lanca Ster
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NAME	<u>ADDRESS</u>
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Helm M. Parce	BG11 Breadway n. 4.14500
Thomas Ricey	12896 Brushing Cleken & Phy. 14004 1570 Energy Stellen My 14004
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Daniel H. Smelan	1570 menson St Ween My 4004
Willard Eastwood	1974 Sandridge al ellen 7.4 14004 13051 Broadway alden 74, 14004
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Elaine Nasa	al 13369 Bullis Bl. alden My 14004
Patrick OSylvi	NEVin 1135 EXCHANGE ST alden My MANGE
Law Mica	1745 MEADOW DE ALDEN N.Y 1400Y
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Sandra a andros	di 2326 Williston Hoto Marilla My 1102
Carol Freen	an 2368 Wands Rd alder HINA
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NAME	ADDAESS
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ill Welton	Swilliam W. Walter 35 Morville Dr. Rochester NY 14620
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p. O C. 76	LA DAVER CHOWLAND 52 ILLIYOUS ST Rocketes M.Y. 14809
27	James Court Studiesk SCI fond Ville Heir 115 Kechester 17 19617
22	mult Tony Mostrodonoto 54 Gler Ocks Dr. Flack, NY 14624
2000	Huma Enris Siterra IT Avon Rd. Roch NV 17625
-Judy	DAVID S=TZ 280 N. MAINST. PANANDAIGH IY 14424
Antho	Luch SAM LESH 45 Gold St. Roch. N.Y. 14620
Lichary D.	Sun Michael D. Sperr 818 TWKER TAVERN RD. WEBSTER NY 14580
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NEWSPAPER ARTICLES



MT. MORRIS — A public meeting to discuss the results of the reconnaissance study of the Genesee River basin will be held Sept. 10 at 7:30 p.m. at Mt. Morris Central School on Bonadonna Avenue.

The meeting has been scheduled by the U.S. Army Corps of Engineers. It will discuss the past findings that valuable Genesee River basin resources "are yet to be developed to their fullest potential to provide for additional flood protection throughout the basin," the Corps said in a press release.

Discussion on the development of hydroelectric power generation, recreational activities, irrigation and other related water and land resources will also be included in the meeting.

The Corps said two potential project sites to be used to develop the river basin are under construction: the Stannards site, located on the Genesee River two miles upstream from Yorks Corners (south of Wellsville), and the Mt. Morris site, where an existing Corps dam is located on the river about 70 miles above the river mouth at Lake Ontario in Rochester.

More information on the meeting can be obtained by calling Wiener Cadet, study manager, at 716-876-5454, or by writing the U.S. Army Corps of Engineers, Buffalo District, 1776 Niagara St., Buffalo, N.Y.

Mt. Morris Dam Meeting Large Turn Out

At a public meeting to discuss two proposed Genesee River Projects, the Army Corp. of Engineers answered questions. The proposed \$65 million project would build a new flood control dam in Allegany County and add a hydropower station to the existing dam at Mt. Morris. The Mt. Morris project has a cost analysis cost ratio of 3.8+; the Stannard project at 1.4+ cost ratio This means an annual net of \$4.6 million for Mt. Morris and \$200,000 for Stannards.

There was a fair amount of negative reaction to the Stannards project and a few questions as to the visual effect and the effect of water rafting from the Mt. Morris project.

The Army Corp. will progress to a more detailed feasibility study to determine whether to continue with the project.

Don't Flood Letchworth Gorge

On Sept. 10, the U.S. Army Corps of Engineers held a public informational meeting in Mount Morris to present its ideas on two dam projects for the Genesee River: one at Standards and one at the existing Mount Morris Dam at Letchworth State Park.

The shocking reality is that the Corps proposes to flood at least 11 miles of the Leichworth Gorge from the Mount Morris

Dam to above Wolf Creek.

This area represents approximately two-thirds of the river gorge within the state park. The permanent flooding of this area will have a significant impact on the scenic areas in the northern and middle sections of the park. People who enjoy this scenic area will tace an entirely different epicture if this project is built. Hikers and canoeists will find their activities and the scenic areas they have grown to love and enjoy changed forever.

Part of the white water section, from the Lower Falls to St. Helena, will be affected, with a significant impact for those people who enjoy white water canoeing, kayaking and public family raiting. These activities currently give the participant a very unique view of Letchworth State Park looking up from the river and up the tall canyon walls of the gorge itself. Completion of the project would severely compromise the enjoyment of the river and the park.

The state Department of Environmental conservation and the National Park Service recently completed a study that categorized this stretch of the Genesee River through Letchworth Gorge as a Category 2 river, having "statewide recreational importance." The study found that this portion of the Genesee River provides recreational opportunities that are exceptional or unique in New York State. The hydroelectric project at the Mount Morris Dam and the resulting reservoir would clearly destroy this section of the Genesee.

Mitex Corp. of Boston, Mass., has federal approval to participate in further development of this study. If this project is completed, and generators owned and operated by this corporation are installed, Rochester Gas & Electric Corp. is required by law to pay this corporation 6 cents per kilowatt hour for power produced.

A Rochester Gas & Electric Corp. spokesman at the meeting stated when questioned that it is a cost burden to RG&E to purchase power generated by private hydropower generators. Since RG&E can produce its own power for much less, the cost of purchasing this power is passed on to the rate payers. Thus the myth of cheaper power for local residents was dispelled. The money generated by this project will not even go into the Rochester area.

For the sake of profit by a corporation outside of this state, the citizens of New York will lose 11 miles of scenic river gorge within their Letchworth State Park unless a

strong public outcry is heard.

Another fact made clear at the Mount Morris hearing was that the proposed addition of 27-foot high gates on top of the current spillway would provide no additional flood protection. They would be needed to keep protection at current levels when the reservoir would be created. Thus the entire project is being considered mainly for the purpose of generating a minuscule amount of hydropower.

Letchworth State Park was created for the use and enjoyment of all the people of New York State through the generosity and foresight of Mr. Letchworth. We must oppose any attempt to wrest this stretch of the Genesee River and the beauty of the gorge away from the state residents.

We urge all those who love Letchworth State Park to write to the U.S. of Engineers in Buffalo by

STEVE and NANC MATCHELL





Residents debate plan to make power in Letchworth

By KAREN KRENIS

Times-Union

MOUNT MORRIS - The gray stone mountain that sprawls across the Letchworth State Park gorge hardly looks like a source of controversy.

A man-made pillar of strength, the Mount Morris Dam appears as permanent as the 300-foot gorge it spans; as indestructible as it seems inflexible.

But officials from the U.S. Army Corps of Engineers are proposing a plan that would alter the dam to create hydropower.

And that plan has both environmental groups and the power company that serves much of the Mount Morris area ready to take up arms.

The controversial project calls for a standing reservoir to be retained behind the south side of the dam, one that would stretch back over 11 miles of the spectacular gorge.

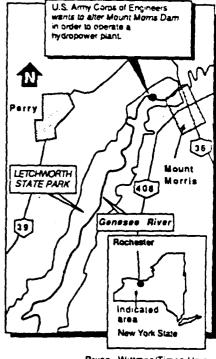
The 36-year-old dam wall would be raised 27 feet, to a new height of 242 feet above the streambed.

The result would be the generation of hydropower, in which electricity is produced by channeling falling water into a standing reservoir.

The hydropower plant is an idea that the Army Corps has been kicking around since 1962, boasting benefits that include long-term energy savings and better flood control, said Wiener Kadet, project manager for the Army Corps. But for various reasons, no plans have made it past the drawing board, he said.

Rochester Gas & Electric Corp. officials say the hydropower plant would force them to charge customers in the Mount Morris area higher prices. They could not predict when, or if, those prices might go down.

Please turn to page 3B



Bryan Wittman/Times-Union





Opposition mounts to Letchworth hydro plan

MOUNT MORRIS (UPI) — Environmentalists have mounted a campaign "to the hilt" to stop flooding a part of the scenic Genesee River gorge in Letchworth State Park to produce hydropower.

The 1,000-member Genesee Valley Chapter of the Adirondack Mountain Club maintains the Army Corps of Engineers proposal to raise the dam 27 feet.

The group says the proposal would "destroy whitewater recreational activity, inundate and destroy the unique ecosystem within the gorge, and severely compromise the scenic beauty of the park."

A suburban Rochester couple, Steven and Nancy Mitchell of Webster, have organized a letterwriting campaign to let federal officials know that outdoors enthusiasts are opposed to the project.

"We firmly intend to fight this to the hilt," said Steven Mitchell, 32. "William Pryor Letchworth would not have wanted a flooded gorge when he gave his property to the state."

On Sept. 1, the Corps of Engineers proposed raising the 36-year-old Mount Morris Dam to provide an 11-mile-long pool of water for hydroelectric facilities.

The Corps also proposed building a dam in Stannards, Allegany County, to protect against flooding and provide a reservoir for conservation and recreation.

Wiener Cadet, a civil engineer and study manager with the Corps of Engineers in Buffalo, stressed that the Mount Morris project is just a proposal and said those opposed to the project are basing their arguments or misinformation.

"I have received a number of letters ... from people expressing concern about any project that would flood the gorge and the falls at Letchworth State Park," Cadet

"What they are concerned about is not something that is real. We have not planned to flood the gorge all the way up to the areas where they are recreating," he said.

The 300-feet-deep Genesee

The 300-feet-deep Genesee River gorge at Letchworth has been referred to as "the Grand Canyon of the East" and is the major attraction in Letchworth Park.



Wiener Cadet Speaks on Letchworth Power Project

by Don Badura

The Genesee Valley Chapter of the Adirondack Mountain Club has launched a campaign against the proposed hydropower project at the Mt. Morris Dam. The chapter is circulating flyers in Livingston and Monroe Counties asking people to send letters of protest to the U.S. Army Corps of Engineers.

According to the flyers, "...the project would alter the character of the Genesee River Gorge within Letchworth State Park. The proposed creation of the Lechworth Reservoir would serverly compromise the scenic beauty of the park, destroy whitewater recreational activities, and innundate and destroy the unique ecosystem existing within the gorge..."

Steve and Nancy Mitchell of Webster are leading the opposition effort. The club is posting appeals in area outdoor stores and is seeking support from environmental groups.

The following is a rebuttal to the club's claims from Weiner Cadet, Corps. Project Manager:

•The Corps has no plans as of

yet to build a plant. At this point it is just a series of ideas.

The planning process is still in the very early stages. It will be

open to input until a plan is

developed that is acceptable to

public and private interests.

The feasibility study is expected to be complete within the next two years. Then the study would be reviewed by headquarters. The design and the construction phase

would follow:

•There is no construction firm yet Enercan Construction of Cuba is to study the feasiblity of constructing a hydropower plant. •The plant has to meet the Corps' standard of quality. The Corps will not build anything which would disturb the dam or flood control plan. The project will actually provide more flood control. The Corps is aware of the environmental impact on recreation. The reservoir would not reach or affect recreation areas within Leichworth State Park.

•The size of the reservoir has not vet been determined.

•By the year 2000 there won't be enough power to meet consumer demand. The resource is there, just not developed yet.

Hydro projects hurt rafting

By MARY ESCH
Associated Press Writer

A coalition of whitewater rafters and kayakers, citing "a genuine natural resource emergency," pleaded with state legislators last spring to stop a flood of small hydroelectric projects on New York rivers.

The American Whitewater Affiliation issued a press release warning that more than 360 hydro projects were in the works across the state. The National Audubon Society distributed a report calling small hydro "a resource ripoff rivers and ratepayers cannot afford."

"If installed, these projects will either flood or dry up by 1990 virtually all of New York's rapids and waterfalls not protected by wild and scenic designation," wrote rafting outfitter Ron Smith in the AWA statement.

According to developers, however, the small hydro flood has slowed to a trickle -- dammed by environmental restraints, regulatory roadblocks, falling oil prices and federal tax reform.

"The industry is against the ropes," says Paul Elston, president of Long Lake Energy Corp. of New York City, a public corporation whose portfolio includes plans for 32 hydroelectric projects totaling 500 megawatts of power.

"The majority of projects that are in the pipeline now will be completed," says Mike Tucker, vice president of the Albany-based Mercer Company, which operates five hydro projects in northeastern New York. "But there will be a much different business analysis in making the decision to develop future sites."

The seed for the independent hydro industry was sown in 1978 when Congress passed the Public Utilities Regulatory Policies Act, which set up a favorable financial climate to foster private development of alternative energy sources.

The Federal Energy Regulatory Commission has received more than 7,500 small hydro project applications in the last seven years, according to FERC spokeswoman Sharon Hyland.

In New York state, more than 300 projects have been proposed on about 170 sites, according to analyst Pete Smith of the state Energy Office.

Tucker's company is the largest operating independent hydro firm in New York, with a total capacity of 50 megawatts being sold to Niagara Mohawk Power Corp. for more than \$12 million a year. But Tucker said Mercer has no other hydro projects in the works.

"The hydro industry has changed," Tucker said. "Back when oil prices were high and interest rates were going down, everything was rosy. Now oil prices have dropped (thus lowering the 'avoided cost' rate paid by utilities for hydropower). And Congress has reshaped the tax law to phase out investment tax credits and accelerated depreciation, which was a tremendous incentive to the development of this industry."

"Then you have the whole new environmentalrecreational issue that didn't exist in the early days," Tucker said. "In the early days the principle focus was to encourage hydro because it wasn't burning fossil fuel. But now there's a tremendous increase in public scrutiny from the environmental and recreational community."

The American Whitewater Affiliation has been a formidable foe to hydro developers.

Rafting outfitter Ron Smith found his livelihood threatened by Mercer's largest project, a \$40 million, 32-megawatt plant on the Black River near Watertown. Smith, whose Adirondack River Outfitters takes thousands of whitewater rafters down that stretch of river, joined with the AWA to intervene in Mercer's federal licensing process until the company gave in to a list of demands.

"We gave them \$125,000 a year worth of energy production to provide 500 hours of whitewater recreation during the summer," Tucker said.

Elston figures whitewater kayakers cost him \$3 million in construction delays and project alterations at Long Lake Energy's 12.5-megawatt plant on the Moose River north of Utica. He says he had invested five years and \$700,000 in a project on the Black River before the AWA intervened and got the state to hold up the license for a recreational study.



Id be given not only for red," such as equipment, is lost" such as sales and accurate," he said. It is said that in past integard to reporting flooders, "they were underestives why the meeting was eople to be more efficient reporting damage. If a resident of Andover, damage that occurs in all the included on the because the waters start of travel to Wellsville. It is said it "will be taken into to what extent I don't want Congressman Amory R-Corning, wants "con-Estimates Needed For Flood Control

By MARK WHITEHOUSE Times Herald Staff Writer

ANDOVER - Residents of the village and town of Andover may get a floodcontrol program and relief from the Sept. 12-13 flooding, but it's going to take considerable work.

County Legislative Chairman Leonard Watson, R-West Almond, Robert Pederson, district conservationist for the Allegany County Soil and Water Conservation District, and John Tucker, county director of emergency services, were on hand at the Andover fire hall Friday afternoon to explain to residents the necessary steps to go through to obtain relief and a prevention program.

Mr. Watson explained to those present - about 35 people - that the U.S. Army Corps of Engineers is considering conducting a reconnaissance study with regard to damage from the September flooding. This will include the possible implementation of a flood control project "not only to fix what's happened, but to prevent it from happening again.

HE STRESSED, though, that the Corps "needs figures - and lots of them.

"It isn't going to happen without lots of work on a lot of people's parts," Mr. Watson said.

Mr. Pederson told residents that "a cost-benefit ratio is necessary" if the Corps is to consider implementing the flood-control program.

This ratio, he said, can best be arrived at by "putting down the exact damages" incurred by the flooding. He said every federal agency requires a positive costbenefit ratio. In other words, he said for every dollar spent, there must be a benefit of at least \$1.01.

He said in 1979, a study was conducted in Andover and the benefit was determined at only 30 cents per dollar spent. Subsequently, the possibility of a floodcontrol plan was dropped.

"Put down the exact damages - either the cost of replacing or properly correcting what has been lost," Mr. Pederson emphasized. "We need those figures."

AS AN EXAMPLE, Mr. Pederson said I a person lost a 15-year-old freezer, they shouldn't report it at it's value, but the cost of replacing it.

"The only way you can have a project is if you substantiate and justify it with a substantial dollar figure," Mr. Perderson

Jacqueline Bellamy, superintendent at Andover Central School, told Mr. Pederson that "\$15,950 in damages" had been incurred on "school grounds and athletic fields."

Mr. Pederson suggested "getting a pro-fessional estimate" for the cost of re-

pairing, "or replacing" the foundation.

Mr. Watson handed out damage estimate forms to those present and noted that others are available at the town and village offices.

Mr. Pederson advised that it would be best to have the forms filled out and sent to the Corps "in two or three weeks." Mr. Watson noted that "the sooner you get them in the better," the Corps can review them.

MR. PEDERSON SAID residents should fill out estimated damages for flooding that occurred in 1972 and 1984 as well; each should be placed on a separate form. The infomation, he said, "will then be summarized by the Corps.'

Mr. Pederson added that for businesses,

"estimates should be given not only for property damaged," such as equipment, "but for business lost" such as sales and

"Be detailed and accurate," he said...

Mr. Pederson said that in past instances, with regard to reporting flooddamage estimates, "they were underestimated.

He said this "is why the meeting was called," to get people to be more efficient and accurate in reporting damage.

Leon McNeill, a resident of Andover. said that any damage that occurs in Wellsville "should be included on the damage report because the waters start in Andover" and travel to Wellsville.

Mr. Pederson said it "will be taken into consideration - to what extent I don't

He added that Congressman Amory Houghton Jr., R-Corning, wants "concrete suggestions on other possible programs," and "if you're serious about a program get as many people as possible to fill out forms."

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Group forming to lobby for a dam in Stannards

By KATHRYN ROSS

WELLSVILLE — Concern over the Stannards Dam issue has prompted the establishment of a citizens' action group in favor of the dam

The Greater Allegany Lakefront Association, organized to promote a responsible public discussion of the proposed lake south of Stannards, is in the early stages of forming.

Even though the group has only been in existence since a meeting with the Army Corps of Engineers about a proposal for a dam on the Genesee River in Sentember, its

spokesman, William Gunner, said the group has many supporters.

The goal of the group, Gunner said, is education so that people can make a vell informed decision about the project.

In a statement of the purpose of the Greater Allegany Lakefront Association (GALA), Gunner said the proposal has met with very vocal opposition, and not without some reason.

There will be some adverse results if the lake is built, including the relocation of some homes and businesses.

However, these seem to be outweighed by the development and improved habitat that will

result if the lake is built and managed properly, he said.

"GALA's initial task is to understand and evaluate the Army's proposal and disseminate the information," he said.

After the Mt. Morris meeting, Gunner said, he saw a need for a group to evaluate the information presented by the Army engineers and to research the possible local economic and recreational benefits of a dom in Stannards.

"It's important that there is a public discussion or forum presenting both sides of the issue," he said.

The group, encouraged by the success of a citizens' group in another part of the state, believes

that a dam in Stannards can have a great economic impact on the area through recreational usage.

Gunner explained that a citizens' group promoted the Great Sacandaga Lake, another Army Corps of Engineers project, from a reservoir for flood protection in the Troy and Albany area, into a positive economic factor.

Although the Sacandaga Lake project is bigger than the Stannards Lake proposal, Gunner said that by scaling down the results from the big lake to the 60 miles of shoreline of the Stannard's lake, the group can get a good idea of the

kind of economic impact the take can have.

"We're not exactly sure of the Army's intentions, but a watchdog group that would promote the recreational aspects of a dam is needed," he said.

Of the Corps proposal, Gunner said that to date the group has found that recreation is a stated goal of the Army, although it is not primary:

"It has been said that the Army's goal is not to create a recreational area and that the result of the project will be a muddy hole. The Army's primary goal is flood control and the prevention of soil erosion, however it has considered the dam in the terms of national economic development. In my opinion," he said, "Who cares what it will mean nationally? We want to cnow what the economic impact will be locally. GALA can help determine what that effect can be."

Gunner said that according to the Army's proposal for water level and winter draw down, the lake should be able to support recreational usage and new habitats for wildlife.

Discussing the difference between the lake level at normal stage and flood level and a prohibition on building within the areas concerned, Gunner said that was a problem the Sacandaga group dealt with successfully. "At Sacandaga, they have docks built on tracks that can easily be raised or lowered depending on the water level. The property owners also have easements to the lake and the green space maintained around the lake adds to its scenic beauty.

At this point Gunner said, GALA wants to help clear the air on the matter of the dam and to promote public awareness.

In the long run if the dam is built, he said, it will be important to have a citizens' group in place to watch over the Army.

Gunner said membership in GALA is open to anyone interested in developing a healthy public dialogue regarding the Stannards lake project. Those interested may contact him.

Corps Won't Desecrate Letchworth

Congratulations on the Letchworth Dam Project editorial in The News Nov. 15. It was fair and accurate to report that the proposed modification to the existing Mount Morris dam would not flood the entire Letchworth gorge or wipe out all the waterfalls and hiking trails.

In fact, the project would not change or submerge any waterfalls. Nor would it wipe out any of the white-water reaches now used by canoeists and rafters. Neither would it destroy trails in areas where hiking is popular.

During the past two months, I have read more than a thousand letters about proposed projects in the Genesee River Basin. It's clear that there is an enormous amount of heartfelt opposition to any change in the Letchworth gorge. Many seem to feel that the Corps of Engineers seeks to destroy the beauty of the gorge. That's just not so. I will not recommend any plan that degrades existing recreational opportunities or significantly alters the existing character of Letchworth gorge.

No one can be sure of whaf William Pryor Letchworth would think or say about the proposal. As a farsighted man interested in the public good, he would probably reserve judgment until the study was completed and all the facts were in. No one favors "tinkering," but not many objected to the carefully planned and well built dam constructed in 1952.

That fine and useful structure, by the way, currently impounds a conservation pool extending five miles upstream between June and November, which neither impairs recreation nor offends the eye. It may well be that there is a chance to increase flood protection, realize a hydropower opportunity and gain other water resource benefits without significantly changing the character or appearance of Letchworth gorge.

I will recommend no project that is not socially acceptable, environmentally sound, economically smart, and practical from an engineering viewpoint. Moreover, no project will be built unless a local sponsor, willing to share construction costs, is identified.

COL. DANIEL R. CLARK
District Commander
U.S. Army Corps of Engineers
Buffslo

Letchworth Is Safe For the Time Being

State Senator, 57th District One of the great treasures in the 59th Senate District is Letchworth: State Parkoften called the Grand Canyon of the East. To many who live here, it's something we take for granted-sort of like a person in Manhattan who has never been to the top of the Empire State. Building.

Over the past six months. events have led us to look a little more closely at Letchworth Park because its future has been threatened. I'm talking about a hairbrained scheme by the Army Corps of Engineers to raise the dam at Mt. Morris by 30 feet-in the process creating a 2,000 acre reservoir and destroying the landscape of the park forevermore.

the days when my father used to take my brother and me fishing along the Genesee River. My background leads me to be an Albany advocate for sportsmen and this inclination is reinforced by the tremendous number of my constituents who enjoy fishing and limiting. I'm not going to burden you with a bunch of fisherman's tales-suffice it to say that to be

One of the factors that led to the decline was the construction of the exisiting flood control dam at Mt. Morris. Built in 1952, it has prevented flood damage in the area with minimal ento live with it.

The problem no we was the Leichworth Gorge. have some engineer lypes. To make it as clear as

beavers who never met a body of water they wouldn't like to dam up, is at it again. This time they've teamed up with 1 some - entrepreneurs what develop hydropower and who see a dollar sign in every drop of water that flows over one of the Corps' concrete monstrosities.

This isn't the first time the federal "beaver patrol" has contemplated raising the dam at Mt. Morris: In the '70s, at the height of the energy crisis, the Corps of Engineers advanced a similar. proposal. The enormous environmental tradeoff for the new energy to be produced made, little sense, then; and even less now. Zime (1999)

Under state law > Rochester Electric and Gas would be mandated to purchase any hydropower pro-My own personal mem duced at such a facility. The ories of the park go back to ourchase price would actual. purchase price would actually be higher than the cost for the utility to produce its own power, thus increasing the cost to ratepayers.

Back in November, assembymen Bill Paxon and John Hasper joined me in a meeting with Congressman Jack Kemp: The four of us agreed to oppose any tinkering with the Mt. Morris Dam and Jack let it be known that the fishing isn't what it used funding requests from the Are my Corps of Engineers would be viewed with great disfavor if they persisted with their foolhardy proposal.

That was enough to kill the idea with the eager beaver crowd from the Corps, who understand where the bark is vironmental harm and we located on the log. That left ishermen have had to learn to live with it.

who think that bigger is bet possible that we aim to proter and that much bigger tect Letchworth State Park, a would be much better yet bill that I sponsored in the The Army Corpsor Senate is now sitting on Engineers, a group of eager Governor Cuomo's desk.

GENERAL CORRESPONDENCE



New York State Office of Parks, Recreation and Historic Preservation

Genesee Region—1 Letchworth State Park, Castile, New York 14427-1124

716-493-2611

Orin Lehman, Commissioner Ronald F. Foley, Regional Director

Hission Chairman W Brown hission Vice-Chairman; by P. Swain nission Members ent B. George J. McKenna Miskell

O'Connor y Potrzebowski 20 Ara 83 11 06

April 18, 1988

Col Daniel R. Clark, Commanding U. S. Army Corps of Engineers Buffalo District 1776 Niagara Street Buffalo, NY 14207

Dear Col. Clark:

On Wednesday, April 27, 1988, the Genesee State Park Commission will be meeting in the Letchworth Room at the Glen Iris Inn at 11:00 A. M.

We would be very pleased if either yourself or your representative, Mr. Weiner Cadet who addressed the Commission previously, would attend this meeting and bring the Commissioners up-to-date on the Mt. Morris hydropower situation.

The material Mr. Cadet shared with the Commissioners and members of our staff was informative and helpful, and we are looking forward to an update.

Thank you for your consideration of this request.

Yours truly,

George L. Momberger

Assistant Regional Director

GLM: cp

CC: W. Cadet

Counties of Genesee, Livingston, Monroe, Orleans, Wyoming An Equal Opportunity Employer

w York State Department of Environmental Conservation MAIL ROOM-NCBIM-S Wolf Road, Albany, New York 12233 -1750

vision of Regulatory Affairs

Thomas C. Jorling Commissioner

28 Ma= 88 1110

March 24, 1988

Mr. James Prez c/o Visual Study Workshop 31 Prince Street Rochester, NY 14607

> Re: Mount Morris Hydroelectric Project FERC Project No. 10076-000

Dear Mr. Prez:

This is in response to your March 21, 1988 telephone inquiry regarding Enercan Construction, Inc.'s proposal to develop hydroelectric generating facilities at the referenced dam site.

I have enclosed copies of ECI's application to the Federal Energy Regulatory Commission (FERC) for a preliminary permit, a copy of the preliminary permit, and a description of the proposed project. The preliminary permit gives ECI priority rights to the site during a specified three-year study period; it does not authorize ECI to construct and operate a hydroelectric station.

No modifications to the federal dam are proposed. However, the permittee's anticipated studies do include an evaluation of current flood control operations and drawdown schedules. Any proposals to change the established operating regime, among other considerations, would initiate an intensive environmental review by this Department.

Please do not hesitate to call me at (518) 457-2224 if you need additional information.

Very truly yours,

Murdock M/ MacKenzie, Chief

Project Review Section

MMM/pm Enclosures

cc: (w/o enc.) - L. Cashell (FERC)

Col. D.R. Clarke (COE-Buffalo)

W. Kiang (OPR)

C. Morrison

V. Husek

A. Butkas

File

Consulting Engineers

Surveyors

310 Delaware Avenue

Buffalo, New York 14202

(716) 853-7582

Oswego, New York (315) 342-3010

Rochester, New York (716) 442-5330

ERIE-NIAGARA CHAPTER NEW YORK STATE SOCIETY OF PROFESSIONAL ENGINEERS ENVIRONMENTAL AFFAIRS COMMITTEE

NOTICE OF MEETING

DATE: Wednesday, February 24, 1988

TIME: 12:00 Noon

PLACE: Buffalo Yacht Club

Colonel Daniel R. Clark, District Commander of the U.S. Army Corps of Engineers, Buffalo District, will be our speaker.

Colonel Clark's talk will be on the proposed dam project at Letchworth State Park, Genesee River.





n Formulation Pranch

adBJECT: Proposed Hydro-Electric Project in Letchworth State Park

Mr. Ross W. Roberts Chairman, Wyoming County Board of Supervisors P.O. Box 232 Warsaw, New York 14569

Dear Mr. Roberts:

THE COOKS AND STATES OF STATES AND SECOND SE

Thank you for your letter dated February 3, 1988 requesting information on the proposed hydro-electric project in Letchworth State Park.

Regarding the hydropower study, as of this date, I have no information from the potential non-Federal developer on the progress of this study. However, I am in receipt of a letter informing me that Project Services International, Inc. of Philadelphia, Pennsylvania, has permit rights to study the feasibility of a hydro project. For specifics on the status of any studies Project Services may be conducting, I suggest you write to Mr. Doug D. Wilner, President, Project Services International, Inc., 755 F. Passyunk Avenue, Philadelphia, Pennsylvania 19147.

As background on the Buffalo District's studies in the Genesee River Basin. I am currently studying the feasibility of installing gates on top of the spillway section of the existing Mt. Morris dam to provide additional flood storage capacity. This additional capacity, now under consideration, would allow the Corps to further reduce flow releases to the lower Genesee River during normal and emergency operations of the reservoir and provide greater flood protection to the lower basin. I expect to make a determination on the feasibility of installing gates on the spillway this summer, at which time I will inform you of the results and solicit your input to my proposal for modifying the existing dam.

My point of contact pertaining to this matter is Mr. Wiener Cadet of my Plan Formulation Branch, who can be contacted at commercial number 716-876-5454, extension 2247 or by writing to the above address.

The Buffalo District -- Leadership in Engineering.

DANIEL R. CLARK Sincerel COL., U.S. ARMY COMMANDING

92 21 88 83 Baniel R. Clark
Commanding
Commanding

CF: CENCB-PD CENCB-PD-PF



Colonel Daniel R. Clark
District Engineer - Buffalo District
U.S. Army Corps of Engineers
1776 Niagara Street
Buffalo, N.Y. 14207

Dear Colonel Clark;

GlenCoe Conservation Society is very concerned about the proposed dams on Cattaraugus Creek and Genesee River.

Would you, therefore, provide a speaker and/or audio or visual presentation which will explain the purpose, reasons, and proposed timetables for these projects.

The next available GlenCoe Society meeting date is Tuesday, March 29, 1988, 8:00 P.M., at the clubroom, Foote Road, East Concord, New York.

Your prompt, positive response will be greatly appreciated.

Please mail all correspondence to:

Richard J. Solak
President, GlenCoe Society, Board of Directors
8722 Supervisor Road
Colden, New York 14033

Please call for directions to clubrooms:

Office - 717-827-9895

Home - 716-941-5656

Sincerely,

Richard J. Solak

cc:

Mr. Wiener Cadet



ROCHESTER SECTION
Established in
1993

Daniel H. Cornwall, P.E. President
Bruce G. Boncke, P.E. First Vice President
James D. Smith, P.E. Second Vice President
James A. Kaniecki, P.E. Secretary
Robert K. McCubbin, P.E. Treasurer Russell J. Harding, P.E.

Past President

Elizabeth M. Dwyre, P.E.

Director

Sergio Esteban, P.E.

Director

Charles M. Frenz, P. (1)

February 2, 1988

Mr. Wiener Cadet, P.E. U.S. Army Corps of Engineers-Buffalo District 1776 Niagara Street' Buffalo, NY 14207

Dear Mr. Cadet,

THE POPULATION ASSESSED MENTAL SECONDARY SECONDARY SECONDARY SECONDARY SECONDARY SECONDARY SECONDARY SECONDARY

On behalf of ASCE, I'd like to thank you for agreeing to speak to our group about the modifications to Letchworth Dam currently under study by the Corps of Engineers. The meeting will be held on Wdnesday, March 9, 1988 at 12:00 noon at the Sheraton Inn-Rochester Airport, 1100 Brooks Avenue, located approximately as shown on the enclosed map. Luncheon will be served prior to your presentation. Please let me know if you would like to have any audio-visual equipment.

As you requested, I've enclosed some information describing ASCE. The "News Release-Background" describes the national organization. ASCE-Rochester section has approxiamtely 300 members. I've enclosed excerps from this month's copy of "The Rochester Engineer", an "umbrella" organization formed by 41 local engineering societies. This publication serves as our Section newsletter.

As we discussed by telephone, the notice will read as follows:

March Luncheon Meeting: The U.S. Army Corps of Engineers-Buffalo District is currently studying potential modifications to Letchworth Dam to provide additional flood control to the lower reaches of the Genesee River Basin. Also, non-Federal interests are studying the feasibility of developing hydropower at the existing Mt. Morris reservoir. Mr. Wiener Cadet, P.E., Study Manager for the Genesee River Basin Comprehensive Study, U.S. Army Corps of Engineers-Buffalo District, will speak on the COE's study.

The meeting will be held at the Holiday Inn Airport, 911 Brooks Avenue on Wednesday, March 9, at 12:00 noon. The luncheon is a choice of Julienne Chef Salad or Open-faced Rueben Sandwich. Non-members are always welcome.

Please call Bonnie (H&A) at 232-7386 by Friday March 4 for reservations. Please note that space is limited. If you make a reservation, but are later unable to attend, cancellations are appreciated.





ROCHESTER SECTION Established in 1923 Daniel H. Cornwall, P.E.
President
Bruce G. Boncke, P.E.
First Vice President
James D. Smith, P.E.
Second Vice President
James A. Kaniecki, P.E.
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Sergio Esteban, P.E.
Director
Charles M. Frenz, P.E.
Director

February 2, 1988

Please let me know if you have any addditional comments on this notice. I am looking forward to your presentation.

Sincerely yours,

Swith m. Dye

Elizabeth M. Dwyre, P.E. Director

Enclosure

EMD/kb

PROJECT SERVICES INTERNATIONAL, INC.

February 2, 1988

R. Peter Lalor Vice President Long Lake Energy Corporation 420 Lexington Avenue, Suite 440 COPY

RE: Mt. Morris Dam (FERC 10076) Cancadea Dam (FERC 9993)

Dear Mr. Lalor:

New York, NY 10170

Project Services International, Inc. ("PSI"), through its wholly-owned subsidiary **Enercan Construction**, Inc. ("Enercan"), is the Permit Holder of FERC 10076 and FERC 9993 preliminary permits for the above-captioned hydro projects.

PSI and Long Lake Rnergy Corporation ("Long Lake") have executed a joint development and conditional purchase agreement regarding the projects, and Long Lake desires to review available documents and data and discuss the project with appropriate government authorities and other interested parties.

Long Lake is hereby designated as the authorized representative of the Permit Holder in these matters and all authorities, agencies, organizations, and individuals are requested to respond to inquiries and information requests from Long Lake as though coming directly from the Permit Holder.

Very Truly,

Douglas Down

President

cc: Wiener Cadet
Project Manager
U.S. Army Corps of Engineers
Buffalo District

New York State Department of Environmental Conservation 50 Wolf Road, Albany, New York 12233-HARRICE MC원에운



1880-87 1250

November 12, 1987

U.S. Army Corps of Engineers Buffalo District 1776 Niagara St. Buffalo, NY 14207

Dear Colonel Clark:

This is in partial response to your letter of October 29, 1987 requesting an expression of interest and support for General Investigations studies. This letter addresses your study of the Genesee River Basin. We support the completion of the current feasibility study. Our criteria for sponsorship of flood protection projects is to support those projects which are engineeringly and environmentally sound, economically justified, meet local needs and for which satisfactory cost saving arrangements can be made with benefitting local governments.

With regard to the Mt. Morris Dam project, we believe that the potential negative environmental impacts and the current cost sharing requirements will likely preclude DEC sponsorship of the flood control element of the project.

Concerning the proposed dam/reservoir at Stannard's, the current cost sharing requirements and the local opposition to the project will probably preclude DEC sponsorship. Of course, a final determination cannot be made until the feasibility report and environmental impact statement are completed.

Sincerely,

James F. Kelley

Chief

Flood Protection Bureau

cc: E. Seiffer

J. Spagnoli

A. Buddle

T. Myers

JFK/ks



21 December 1987

To Whom It May Concern:

AT this time we are notifying those concerned that Enercan Construction, Inc., including the Preliminary Permits for Hydroprojects at Mt. Morris and Caneadea Dam, has been sold by Enercan's Corporate headquarters to:

Project Services International 755 East Passyunk Philadelphia, PA 19147 Phone (215) 922-1616 Attention: Mr. D.D. Wilner

Any further correspondance you may have concerning either Hydroproject should be addressed to Mr. Wilner.

Thank you for your considerations.

Sincerely,

ENERCAN RESOURCES CORP.

Glenn T. Herdman

V.P. Business Development



DEPARTMENT OF THE ARMY

BUFFALO DISTRICT, CORPS OF ENGINEERS 1778 NIAGARA STREET BUFFALO, NEW YORK 14207-3199

NOV 1 9 1987

ESSENTE ESSESSE PROCESSES NO

Subject: Genesee River Basin Study

The Editor
Buffalo News
One News Plaza
Buffalo, New York 14224

Dear Sir:

Congratulations on your Letchworth Dam Project editorial of November 15, 1987. It was fair and accurate to report that the proposed modification to the existing Mt. Morris dam would not flood the entire Letchworth Gorge, or wipe out all the waterfalls and hiking trails. In fact, the project would not change or submerge any waterfalls. Nor would it wipe out any of the white water reaches now used by canoeists and rafters. Neither would it destroy trails in areas where hiking is popular.

During the past two months, I have read more than a thousand letters about proposed projects in the Genesee River Basin. It's clear that there is an enormous amount of heart-felt opposition to any change in the Letchworth Gorge. Many seem to feel that the Corps of Engineers seeks to destroy the beauty of the Letchworth Gorge. That's just not so. I will not recommend any plan that degrades existing recreational opportunities or significantly alters the existing character of Letchworth Gorge.

No one can be sure of what William Pryor Letchworth would think or say about the proposal. As a far sighted man interested in the public good, he would probably reserve judgment until the study was completed and all the facts were in. No one favors "tinkering," but not many objected to the carefully planned and well built dam constructed in 1952. That fine and useful structure, by the way, currently impounds a conservation pool extending five miles upstream between June and November which neither impairs recreation nor offends the eye. It may well be that there is a chance to increase flood protection, realize a hydropower opportunity, and gain other water resource benefits without significantly changing the character or appearance of Letchworth Gorge.

I will recommend no project that is not socially acceptable, environmentally sound, economically smart, and practical from an engineering viewpoint. Moreover, no project will be built unless a local sponsor, willing to share construction costs, is identified.

Daniel R. Clark

Daniel R. Clark

Colonel, Corps of Engineers

District Commander

MEMORANDUM FOR RECORD

SUBJECT: Genesee River Basin Study, Agricultural Production on the Lakeplain

1. Location: Comstock Foods, 1 Fisher Road, Pittsford, New York

2. Date: 7 April 1987, 1:30 p.m.

3. Participants: See attached sheet

4. Summary:

a. Introduction. Project Manager, Wiener Cadet, gave a brief synopsis of the study in general, the study's current phase (Feasibility Study), and cost-sharing formulas for study and project costs. The meeting's purpose was two-fold. First, to determine if there is a local sponsor that will share in the cost of any irrigation component of the multi-use project. Secondly, to obtain feedback on the irrigation analysis performed in the Reconnaissance Report.

The irrigation portion of the project consists of substituting water currently being taken from the Barge Canal with water from the Genesee River. This water would then remain in the Barge Canal and be available for irrigation of the Lake Ontario Plain.

b. Potential Sponsors for the Irrigation Portion of the Study.

Various potential sponsors (Soil Conservation Service, Cooperative Extension, local Food Processors, The Grange Association, and the State Agriculture & Markets Agency) were identified, but none were committed as probable potential sponsors. The Food Processors indicated they favor such an irrigation proposal for the lakeplain. Tom Nally agreed to contact the State Agriculture & Markets Agency regarding sponsoring the irrigation component of potential multi-purpose project.

c. Discussion of the Irrigation Benefit Evaluation in the Reconnaissance Report.

Ray Waxmonsky explained the assumptions made for the Irrigation Benefit Evaluation presented in the Reconnaissance Report. Two major questions were discussed.

- (1) Is there a need and demand for irrigation produce production?
- (2) If yes, what are the location, and number of acres by vegetable and fruit type?
 - (a) Need for Irrigated Vegetable Production.

CENCB-PD-PF

SUBJECT: Genesee River Basin Study, Agricultural Production on the Lakeplain

All of the Food Processors indicated the percentage of their growers who could potentially use Barge Canal water is limited because of the location of the farms with respect to the Barge Canal. However, an indication of the number of acres that could potentially irrigated with barge water was given by Comstock Foods as follows:

Acres	:	Produce	:		•					
2,000 2,500 400		Corn Beans Cabbage	:	Potential	irrigated	average	by	crop	(Comstock	Foods)

The need for irrigation water for fresh fruit production was also discussed.

(b) Location and Number of Acres the could use Irrigation Water from the Barge Canal.

The consensus reached was that there is a need for irrigated vegetable and fruit production. The number of acres involved and their location, for at least processed produce, could be generally determined by the Food Processors. They all agreed to consider determining the number, location, and types of produce grown by their members who could use Barge Canal Water for irrigation.

Further investigative work needs to be performed to obtain reliable data on the number of acres used for fresh produce production along the Barge Canal. Their location, types of crops grown, percent now irrigated, capability of irrigating acres, etc.

Mieneradet Proper Manager

5. The meeting adjourned at 4:00 p.m.

GENESEE RIVER BASIN STUDY

Agricultural Production on the Lakeplain Comstock Foods, 1 Fisher Road, Pittsford, New York 7 April 1987

Name	Representing	Telephone		
Ray Waxmonsky	U.S. Army Corps of Engineers	716-876-5454 x2178		
Ron Guido	U.S. Army Corps of Engineers	716-876-5454 x2177		
Tom Nally	Monroe Co. Coop. Extension	716-461-1000		
Bob Bocksler	General Foods Corporation	716-226-3344		
Gib Scott (Tom Facer)	Comstock Foods	716-385-6580		
Steve Klus	Seneca Foods	315-926-4284		
Roger Haberly	U.S. Army Corps of Engineers	716-876-5454 x2265		
Wiener Cadet	U.S. Army Corps of Engineers	716-876-5454 x2247		
Cardle Rackowski	Orleans Co. Coop. Extension	716-589-5561		





New York State Department of Environmental Conservation 50 Wolf Road, Albany, New York 12233-



August 11, 1986

Colonel Daniel R. Clark District Commander U.S. Corps of Engineers Buffalo District Department of the Army 1776 Niagara Street Buffalo, New York 14207

Dear Colonel Clark:

This is in reply to your letter of July 3, 1986 concerning deauthorization of the Caledonia Project to which we had sent an interim reply dated July 9, 1986. This is to advise you that we have coordinated the matter with the Village of Caledonia and concur with your recommendation to deauthorize the project because of the lack of economic justification.

Sincerely,

James F. Kelley, Director Flood Protection Bureau

RLK/dl

cc: E. Seiffer A. Buddle

Honorable Robert Bostwick

16 AO

d1/2222

DEPARTMENT OF THE ARMY BUFFALO DISTRICT, CORPS OF ENGINEERS 1776 NIAGARA STREET BUFFALO, NEW YORK 14207-3199

NCBPO

3 JUL 1286

SUBJECT: Review for Deauthorization for the Caledonia Flood Control Project,

Caledonia, New York

Mr. James Kelley Director Flood Protection Bureau New York State Department of Environmental Conservation 50 Wolf Road Albany, New York 12233-0001 OFC. MGMT. DAS

Dear Mr. Kelley:

This is in response to a 25 June 1986 telephone request from Mr. Richard Konsella of your office and a 30 June 1986 letter from Mr. Eric A. Seiffer, Director, Region 8, DEC, for direct coordination on the subject project and file information on our 1976 review.

The Buffalo District is currently conducting this deauthorization review, pursuant to the Water Resources Development Act of 1974 (Public Law 93-251, as amended), approved 7 March 1974. This Act requires that Congress annually be provided a list of uncompleted Corps of Engineers projects which no longer are considered appropriate for continued authorization. Your present views regarding the appropriateness of deauthorization action on the project are requested.

The Spring Creek project, at Caledonia, NY, was authorized by the Flood Control Act of 1950 (House Document 232, 81st Congress, 1st Session), and provides for a diversion channel with a capacity of 400 cubic feet per second, to start at Spring Creek, just south of the New York Central Railroad, extending west, about 1,600 feet along the south side of the railroad, thence south about 900 feet to the end, at the Erie Railroad fill, passing through a new bridge at Main Street, and the filling of a low area west of Spring Road.

No work has been done on the Corps project, and it has been classified as deferred for restudy since 1954, due to the lack of local cooperation and opposition by the New York State Fish Hatchery at Caledonia, NY.

An initial deauthorization review was conducted in 1975 and completed in January 1976. The report on this initial review recommended that the project we deauthorized because of lack of economic justification. This recommendation was reconsidered at the request of local and congressional interests. Details of this review are attached as Enclosure 1. Reviews conducted in 1977 and 1983 reconfirmed the 1975 review findings and recommendations. The project, however, continue to be classified

NCBPO

SUBJECT: Review for Deauthorization of the Caledonia Flood Control Project, Caledonia, NY

continued to be classified as deferred for restudy. The estimated construction costs for the project, at October 1985 price levels are \$240,000 Federal and \$205,000 non-Federal, yielding estimated annual costs and benefits of \$61,000 and \$12,000, respectively. (Benefit-to-Cost-Ratio: 0.20 to 1). Improvement works performed in 1979, by local interest groups to reduce local flooding problems, consist of a ditch and a 4-foot diameter tile pipe running from the south side of Route 5 to the old New York Central Railroad and Mill Street. These improvements have further increased the lack of economic justification for the project. Since this benefit-to-cost ratio is substantially below the 1.0 benefit-to-cost ratio needed to economically justify Federal participation in the project, I will recommend that the project be deauthorized unless input you provide in writing, by 25 July 1986, causes me to decide otherwise.

The final decision on the deauthorization recommendation of the Office, Chief of Engineers rests with Congress. Any project submitted on the Chief of Engineers recommended list, may be removed by a resolution adopted by either of the Committees on Public Works, within a 90-day Congressional review period.

My point of contact pertaining to this matter is Ms. Mary Jo Braun of my Program Development Office who can be contacted at commercial number (716) 876-5454, extension 2222 or by writing to:

District Commander
U.S. Army Engineer District, Buffalo
1776 Niagara Street
Buffalo, New York 14207-3199
ATTN: Ms. Mary Jo Braun

Thank you for your views and assistance in this matter.

Buffalo District - Leadership in Engineering.

Sincerely,

Bruce W. Haigh, LTC

DANIEL R. CLARK Colonel, Corps of Engineers District Commander

1 Enclosure As stated

CF:
Mr. Eric A. Seiffer MCBPD-PF
Regional Director NCBPD
New York State Department NCBPA
of Environmental Conservation NCBDE
6274 East Avon-Lima Road NCBPO
Avon, New York 14414

NCBPO

SUBJECT: Review for Deauthorization of the Caledonia Flood Control Project, Caledonia, NY

Mr. Carroll Bickford Town Supervisor Town of Caledonia 370 Leicester Street Caledonia, NY 14423



Dear Mr. Bickford:

The Buffalo District is currently conducting a review of the subject project, for deauthorization, pursuant to the Water Resources Development Act of 1974 (Public Law 93-251, as amended), approved 7 March 1974. This act requires that Congress annually be provided a list of uncompleted Corps of Engineers projects which are no longer considered appropriate for continued authorization. Your present views regarding the appropriateness of deauthorization action on the project were requested by letter dated 24 April, 1986. I have not received a written response from you. I will recommend to the Chief of Engineers, that the project be deauthorized, unless input you provide, in writing, by 15 July 1986, causes me to decide otherwise.

The final decision on the deauthorization recommendation of the Chief of Engineers rests with Congress. Any project submitted on the Chief of Engineers recommended list may be removed by a resolution adopted by either of the Committees on Public works, within a 90-day Congressional review period.

My point of contact pertaining to this matter is Hs. Hary Jo Braun of my Program Development Office, who can be contacted by calling commercial number (716) 876-5454, extension 2222, or by writing to:

District Commander
U.S. Army Engineer District, Buffalo
1776 Niagara Street
Buffalo, NY 14207
ATTN: Ms. Mary Jo Braun

NOBBO

SUBJECT: Review for Deauthorization of the Caledonia Flood Control Project, Caledonia, NY

The Buffalo District -- Leadership in Engineering.

Sincerely,

DANIEL R. CLARK Colonel, Corps of Engineers District Commander

Copy Furnished:
NCSPO
NCBDB
NCSPA
NCBPD-PF

BOARD OF LEGISLATORS

COUNTY OFFICE BUILDING BELMONT, NEW YORK 14813

John W. Hasper, Chairman Linda J. Canfield, Clerk Telephone 716 268-9222

POSSOCIAL PROPERTO SILLIFF STATE STATES TO SELLE SANDAN

John E. Margeson, Administrative Assistant Telephone 716 268-9217

May 27, 1986

John Zorich, Chief Planner U.S. Army Corps of Engineers Buffalo District 1776 Niagara St. Buffalo, New York 14207

Dear Mr. Zorich:

On behalf of the entire Allegany County Planning Board, I wish to thank both you and Mr. Cadet for your presentation explaining the current study of the potential Genesee River dam project at Stannards.

The study has provoked a great deal of interest in our County and your program answered many questions.

Again, thank you for your time, effort and expertise. Wishing you success with your study.

Delores Cross, Chairman

Planning Board

DC:11c

New York State Department of Environmental Conservation 50 Wolf Road, Albany, New York 12233-0001



Henry G. Williams Commissioner

April 24, 1986

Colonel Danie R. Clark District Commander U.S. Corps of Engineers Department of the Army Buffalo District 1776 Niagara Street Buffalo, New York 14207

Dear Colonel Clark:

Please be advised of our continued interest in the Genesee River Basin Study and desire that expeditious action be taken to complete pre-authorization planning on this project as soon as possible.

Sincerely,

James F. Kelley
Director, Flood
Protection Bureau

RLK/d1

cc: Eric Seiffer John Spagnoli Ed Karath

GREAT WESTERN POWER & LIGHT, INC. P.O. Box N

P.O. Box N Manti, Utah 84642 Telephone: (801) 835-0202



January 7, 1986

Environmental Resource Planner U.S. Army Corps of Engineer Post Office Building, Room 341 350 South Main Street Salt Lake City, Utah 84101

To whom it may concern:

In accordance with the Federal Energy Regulatory Commission (FERC) proceedures we request your comments for the filing of an application for licensing of a major hydroelectric water project, Great Western Power & light Inc. acting as the agent for Livingston County Associates request your input concerning the following proposal:

Description of Existing and Proposed Facility

Mount Morris Dam is located on the Genesee River approximately 67 river miles above the mouth of the Genesee River in Livingston County, New York. The project was authorized by the Flood Control Act of 1944, and construction was initiated in March 1948 and was substantially completed in December of 1951. The Chief of Engineers in April 1944 commented that "...the proposed Mount Morris Reservoir should be the initial step in any comprehensive plan for the development of the water resources of the Genesee River Basin. Provision should be made or increasing the storage capacity of the reservoir if found desirable when construction is undertaken. The increased capacity would afford greater security against flooding and the enlarged reservoir could be better utilized in the further development of the river's resources..."

Thus, the dam when constructed had flood control as its prime objective, but maintained flexibility for other uses including hydropower, by the inclusion of two intake openings in the left abutment suitable for installation of two 18 feet diameter penstocks.

Mt. Morris is a concrete gravity overflow dam, with an overall length of 1,028 feet, a top width of 20 feet and a bottom width of 212.8 feet.

The top of the non-overflow section is at elevation 790, while the overflow section is at elevation 760. The maximum height of the structure above stream bed is 215 feet. A control tower is located in the right abutment.

(continued on next page)

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The spillway is an uncontrolled ogee section, 550 feet long, located in the center of the dam. With a head of 28 feet the spillway design discharge is 320,000 cfs.

The outlet consists of nine 5' x 7' rectangular conduits located in the base of the spillway section. Each conduit is controlled by a hydraulic vertical slide gate, with a second gate for emergency operation. The inlet invert elevation of each conduit is at 585.0 ft., while the outlet invert is at 560.7 ft.

As noted above, the construction of the dam also included provision of future hydropower development at the site by inclusion of two intakes in the left abutment, suitable for installation of two 18 ft. diameter penstocks. Each penstock opening with centerline elevation at 644.5 ft. is plugged with concrete pending future power installation.

No powerhouse or other power generating facilities are at the dam. It should be noted, however, that approximately 500 feet downstream of the dam toe, [left abutment] a relatively flat area has been created essentially from spoil material from the dam construction. This area may be suitable for location of a powerhouse and support facilities.

A 240 foot long, 464 foot wide stilling basin is located at the toe of the overflow section and serves both the spillway discharge and outlet conduits. The basin is set at elevation 560.0 feet and the training walls are at elevation 610.0 feet.

The proposed concept for power generation uses two of the low level outlets near the left abutment, combined in a single conduit, aligned to run at the base of the stepped training wall to the powerhouse located on relatively flat topography, just downstream of the stilling basin and sill. This configuration will have minimal impact upon the stilling basin and will result with the powerhouse above tailwater levels.

Livingston County Associates plans to utilize the existing 18 foot pipe provisions already located on the dam for hydroelectric facilities. We plan to have a total installed capacity of 5000 kW. We plan to operate this facility as a run-of-the-river hydro unit and do not plan to alter or change any of the flows released from the Mt. Morris Dam. We will be working closely with the Army Corps of Engineers, Buffalo District.

A copy of the preliminary permit No.8140 approved by FERC on this project is available upon written request.

We would appreciate receiving, at your earliest convenience, the results of your research and any comments, studies or recommendations you may have.

Thank you for you help in this matter.

Jordan R. Walker